

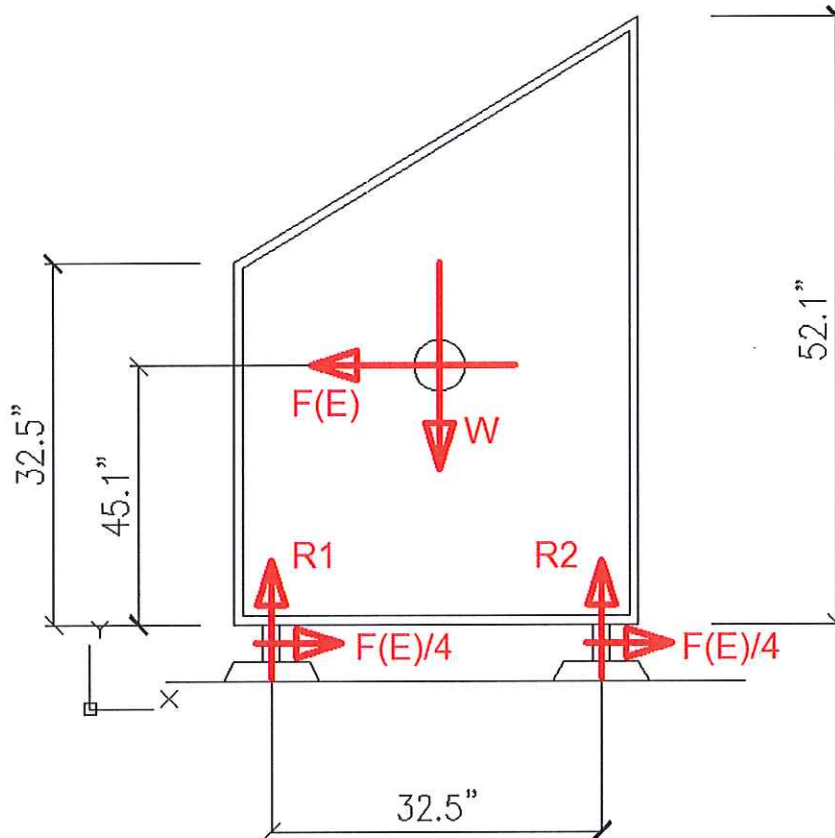
# Calculation of the Anchorages for the Cabinet

- Estimated weights:
  - Cabinet: 500 lb
  - Contents: 500 lb
  - **TOTAL: 1,000 lb**



1/19/17

- Considered earthquake:
  - The maximum base acceleration of the Response Spectra previously considered was used:
  - Design base acceleration:  $S_{DS} = 0.764 g$



- $W = 1,000 \text{ lb}$
- $F_E = 1,000 \text{ lb} \times 0.764 = 764 \text{ lb}$
- $M_E = 764 \text{ lb} \times 45.1" = 2,870 \text{ lb.ft}$
- $R_1 = \frac{1,000}{4} + \frac{2,870 \times 12}{2 \times 32.5} = 780 \text{ lb}$
- $R_2 = \frac{1,000}{4} - \frac{2,870 \times 12}{2 \times 32.5} = -280 \text{ lb}$

A majoration factor of 1.5x was used for design.

### **Case 1:**

$$\begin{cases} N_1 = R_1 = -780 \text{ lb} = -3.5 \text{ kN} \\ T_1 = \frac{F_E}{4} = \frac{764}{4} = 191 \text{ lb} = 0.85 \text{ kN} \end{cases}$$

### **Case 2:**

$$\begin{cases} N_2 = 280 \text{ lb} = 1.25 \text{ kN} \\ T_2 = \frac{F_E}{4} = \frac{764}{4} = 191 \text{ lb} = 0.85 \text{ kN} \end{cases}$$

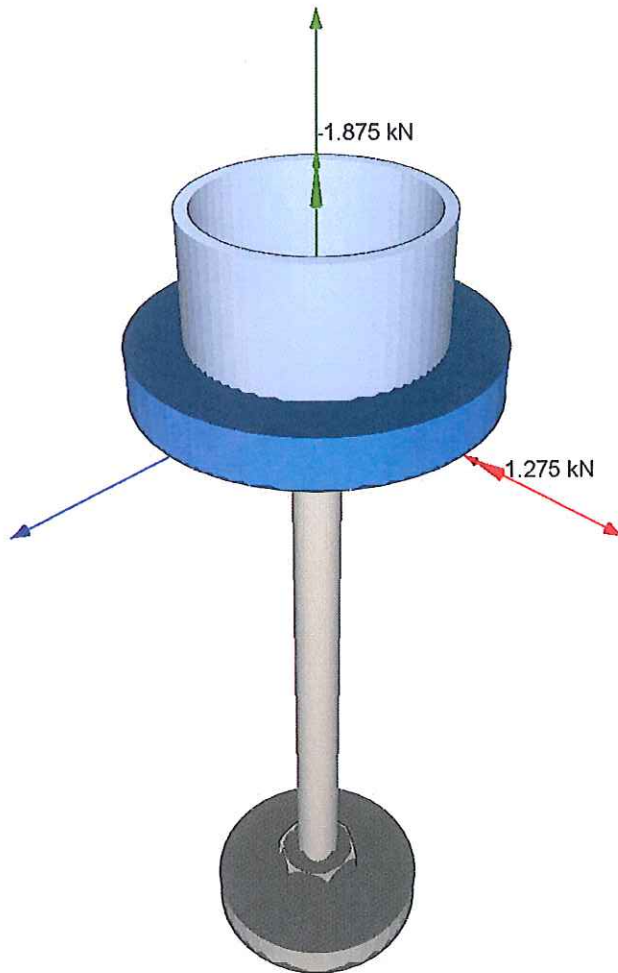
### **Anchor Bolt Characteristics:**

- $f'_c = 2 \text{ ksi} = 14 \text{ MPa}$
- $f_y = 40 \text{ ksi} = 276 \text{ MPa}$
- Diam. =  $3/8" = 10 \text{ mm}$
- Embedment Length:  $L = 6" = 150 \text{ mm}$

Following are the analysis program (PROKON - BasePlate) results.

**Holding-down Bolt Design - SANS 10162 - 2005****Calculation Sheet for Load Case : 2**

P : -1.875 kN  
V<sub>x</sub> : 1.275 kN

**Material Strength Properties**

f<sub>cu</sub> : 14 MPa  
f<sub>u</sub> Bolt : 400 MPa  
f<sub>y</sub> Base Plate : 260 MPa

**Column Section**

O1 48.4x2.5

**Bolt Resistance Forces**

Bolt Diameter : 10 mm

Bolt Netto Cross Section

$$\begin{aligned}A_n &= \frac{0.75 \cdot \pi \cdot d^2}{4} \\ &= \frac{0.75 \times \pi \times 10^2}{4} \\ &= 58.905 \text{ mm}^2\end{aligned}$$

Tension Resistance

25.2.2.1

$$\begin{aligned}T_r &= \frac{0.67 \cdot A_n \cdot f_u}{1000} \\ &= \frac{0.67 \times 58.905 \times 400}{1000} \\ &= 15.787 \text{ kN}\end{aligned}$$

Shear Resistance

25.2.3.3

$$\begin{aligned}V_r &= \frac{0.6 \cdot 0.67 \cdot 0.7 \cdot A_n \cdot f_u}{1000} \\ &= \frac{0.6 \times 0.67 \times 0.7 \times 58.905 \times 400}{1000} \\ &= 6.630 \text{ kN}\end{aligned}$$

Compression Resistance

$$\begin{aligned}C_r &= \frac{0.75 \cdot 0.67 \cdot A_n \cdot f_u}{1000} \\ &= \frac{0.75 \times 0.67 \times 58.905 \times 400}{1000} \\ &= 11.840 \text{ kN}\end{aligned}$$

### Factors Affecting Bolt Pull-Out Area

cBolt = Anchorage length of bolt  
cBolt = 150 mm

End Type Used is a Circular Plate  
Circular Plate diameter is 50 mm  
Circular Plate thickness is 8 mm

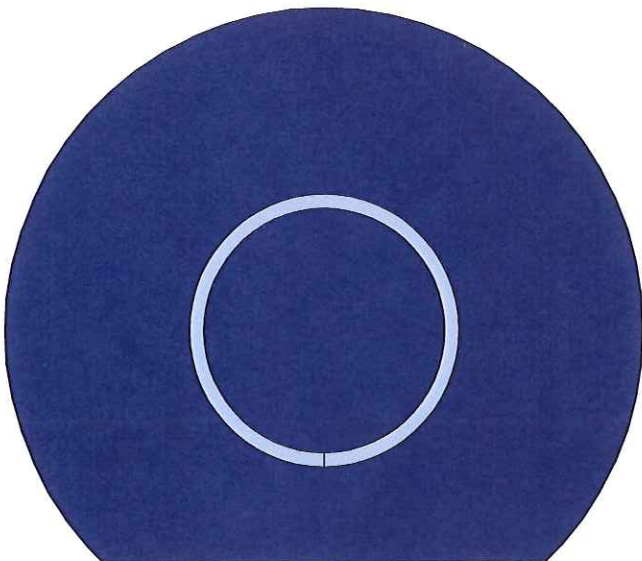
### Find Effective Compression Area

Calculate  $Z_{pl}$

$$\begin{aligned}Z_{pl} &= \frac{b \cdot t_p^2}{4} \\ &= \frac{1 \times 10^2}{4} \\ &= 25.000\end{aligned}$$

Effective Distance from Edge of Section

$$\begin{aligned}cM_{\alpha} &= \sqrt{\frac{Z_{pl} \cdot 2 \cdot 0.9 \cdot \frac{f_y}{1.15}}{b \cdot \frac{f_{ct}}{1.5}}} \\ &= \sqrt{\frac{25 \times 2 \times 0.9 \times \frac{260}{1.15}}{1 \times \frac{14}{1.5}}} \\ &= 33.016 \text{ mm}\end{aligned}$$

**Find Equilibrium**



Software Consultants (Pty) Ltd  
Internet: <http://www.prokon.com>  
E-Mail : [mail@prokon.com](mailto:mail@prokon.com)

Job Number		Sheet
Job Title		
Client		
Calcs by	Checked by	Date

**The indicated plane represents strain resulting in force equilibrium**

**The actual number of Grid Point used for calculation is 1068**

### **Moment balancing**

Sum Of Moments around X-axis = 0.000

Sum Of Moments around Y-axis = 0.000

### **Axial Force balancing**

Sum Of Forces in Z-direction = 0.010

**The Shear Resisitance in the Bolts Resists the Following Forces:**

Forces in X-direction

Forces in Y-direction

Moments around Z-axis

**Thus:**

Bolt 1 Shear Force is 1.205 kN

The maximum Shear Force being 1.205 kN

### **Calculating Factors of Safety in Critical Bolt**

#### **Tension in Bolts**

$$\begin{aligned} FOS &= \frac{T_r}{Tension} \\ &= \frac{15.708}{1.865} \\ &= 8.423 \end{aligned}$$

#### **Shear in Bolts**

$$\begin{aligned} FOS &= \frac{V_r}{S_{hear}} \\ &= \frac{6.5973}{1.2052} \\ &= 5.474 \end{aligned}$$

#### **Bolt BasePlate interaction**



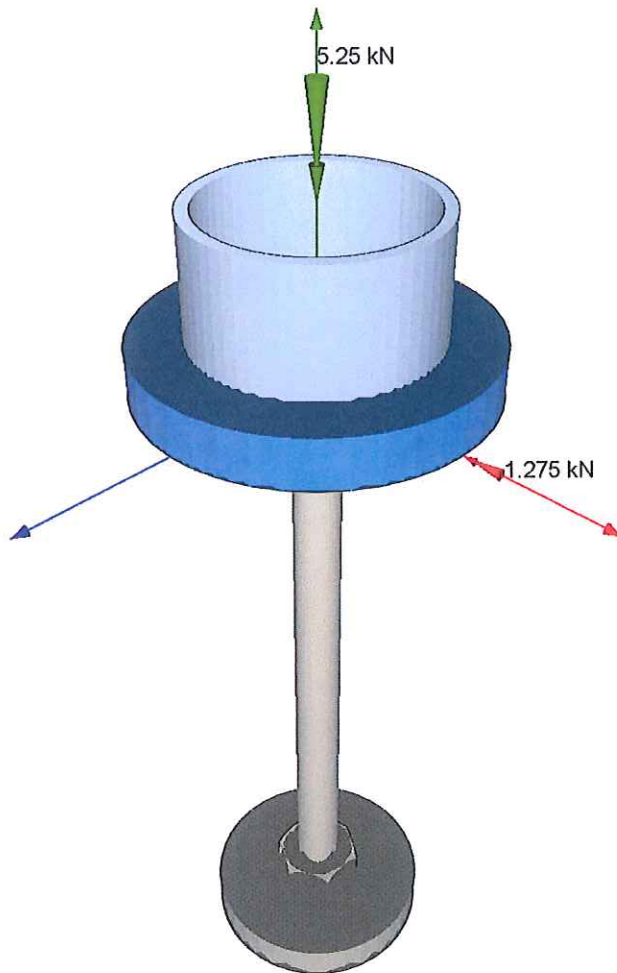
Software Consultants (Pty) Ltd  
Internet: <http://www.prokon.com>  
E-Mail : [mail@prokon.com](mailto:mail@prokon.com)

Job Number		Sheet
Job Title		
Client		
Calcs by	Checked by	Date

$$FOS = \frac{Resistance}{Force}$$
$$= \frac{29.361}{1.865}$$
$$= 15.743$$

**Holding-down Bolt Design - SANS 10162 - 2005****Calculation Sheet for Load Case : 1**

P : 5.25 kN  
V<sub>x</sub> : 1.275 kN

**Material Strength Properties**

f<sub>cu</sub> : 14 MPa  
f<sub>u</sub> Bolt : 400 MPa  
f<sub>y</sub> Base Plate : 260 MPa

**Column Section**

O1 48.4x2.5

**Bolt Resistance Forces**

Bolt Diameter : 10 mm

Bolt Netto Cross Section



Job Number		Sheet
Job Title		
Client		
Calcs by	Checked by	Date

$$\begin{aligned}A_n &= \frac{0.75 \cdot \pi \cdot d^2}{4} \\ &= \frac{0.75 \times \pi \times 10^2}{4} \\ &= 58.905 \text{ mm}^2\end{aligned}$$

Tension Resistance

25.2.2.1

$$\begin{aligned}T_r &= \frac{0.67 \cdot A_n \cdot f_u}{1000} \\ &= \frac{0.67 \times 58.905 \times 400}{1000} \\ &= 15.787 \text{ kN}\end{aligned}$$

Shear Resistance

25.2.3.3

$$\begin{aligned}V_r &= \frac{0.6 \cdot 0.67 \cdot 0.7 \cdot A_n \cdot f_u}{1000} \\ &= \frac{0.6 \times 0.67 \times 0.7 \times 58.905 \times 400}{1000} \\ &= 6.630 \text{ kN}\end{aligned}$$

Compression Resistance

$$\begin{aligned}C_r &= \frac{0.75 \cdot 0.67 \cdot A_n \cdot f_u}{1000} \\ &= \frac{0.75 \times 0.67 \times 58.905 \times 400}{1000} \\ &= 11.840 \text{ kN}\end{aligned}$$

### Factors Affecting Bolt Pull-Out Area

cBolt = Anchorage length of bolt  
cBolt = 150 mm

End Type Used is a Circular Plate  
Circular Plate diameter is 50 mm  
Circular Plate thickness is 8 mm

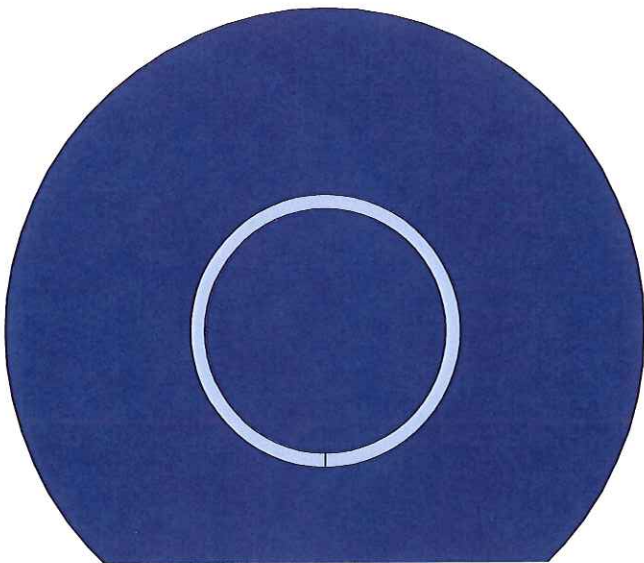
### Find Effective Compression Area

Calculate  $Z_{pl}$

$$\begin{aligned}Z_{pl} &= \frac{b \cdot t_p^2}{4} \\ &= \frac{1 \times 10^2}{4} \\ &= 25.000\end{aligned}$$

Effective Distance from Edge of Section

$$\begin{aligned}cM_{\alpha} &= \sqrt{\frac{Z_{pl} \cdot 2 \cdot 0.9 \cdot \frac{f_y}{1.15}}{b \cdot \frac{f_{ct}}{1.5}}} \\ &= \sqrt{\frac{25 \times 2 \times 0.9 \times \frac{260}{1.15}}{1 \times \frac{14}{1.5}}} \\ &= 33.016 \text{ mm}\end{aligned}$$

**Find Equilibrium**



Software Consultants (Pty) Ltd  
 Internet: <http://www.prokon.com>  
 E-Mail : [mail@prokon.com](mailto:mail@prokon.com)

Job Number		Sheet
Job Title		
Client		
Calcs by	Checked by	Date

**The indicated plane represents strain resulting in force equilibrium**

**The actual number of Grid Point used for calculation is 1068**

**Moment balancing**

Sum Of Moments around X-axis = 0.000  
 Sum Of Moments around Y-axis = 0.000

**Axial Force balancing**

Sum Of Forces in Z-direction = 0.009

**The Shear Resistance in the Bolts Resists the Following Forces:**

Forces in X-direction  
 Forces in Y-direction  
 Moments around Z-axis

**Thus:**

Bolt 1 Shear Force is 1.205 kN

The maximum Shear Force being 1.205 kN

**Calculating Factors of Safety in Critical Bolt**

**Compression in Bolts**

$$\begin{aligned}
 FOS &= \frac{C_r}{\text{Compression}} \\
 &= \frac{15.708}{.84741} \\
 &= 18.536
 \end{aligned}$$

**Shear in Bolts**

$$\begin{aligned}
 FOS &= \frac{V_r}{S_{\text{shear}}} \\
 &= \frac{6.5973}{1.2052} \\
 &= 5.474
 \end{aligned}$$

**Concrete**



Software Consultants (Pty) Ltd  
Internet: <http://www.prokon.com>  
E-Mail : [mail@prokon.com](mailto:mail@prokon.com)

Job Number		Sheet
Job Title		
Client		
Calcs by	Checked by	Date

$$FOS = \frac{StrainMax}{Strain}$$
$$= \frac{.0035}{.00007}$$
$$= 50.000$$

### Bolt BasePlate interaction

$$FOS = \frac{Resistance}{Force}$$
$$= \frac{29.361}{.84741}$$
$$= 34.648$$

