## **PROJECT DIRECTORY:**

#### OWNER:

MetalTek International 905 E. St. Paul Avenue Waukesha, WI 53188 TEL: (262) 544-7700

#### **ARCHITECT:**

Schroeder & Holt Architects 311 E. Chicago, Suite 310 Milwaukee, WI 53202 TEL: (414) 727-2321 email stevee@sha-a2k.com ATTN: Steven M. Esser

GENERAL CONTRACTOR/DEVELOPER: Pinnacle Construction of WI 6495 s 27th St. Franklin, WI 53132 TEL: (414) 435-0250 email www.pinnaclecons.com ATTN: Mike Michalski

STRUCTURAL: Core 4 Engineers 12308 North Corporate Parkway, Suite 300 Mequon, WI 53092 TEL: (262) 236-9372 ATTN: Matthew Christianson

## SHEET INDEX:

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A1.1	ARCHITECTURAL SECTIONS AND DETAILS						
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S1.0	GENERAL STRUCTURAL NOTES						
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# MetalTek X-Ray Building Addition

# East St. Paul Avenue

Waukesha, WI

ST. PAUL AVE.



## **PROJECT INFORMATION:**

7,074sf 5,025sf 2,749sf

Governing Code: Construction type: Occupancy classification: Number of stories allowed: Number of stories: Sprinkler system: Allowable area per floor: Allowable area increase - frontage: Total Allowable Building area:

Wisconsin adopted 2009 IBC IIB - metal unprotected NONE 15,500sf 11,625sf 27,125sf

Fire extinguishers:

ARCHITECTURAL

STRUCTURAL

## VICINITY MAP



20 Image: A set of the  $\infty$ July • • Date Set Progress

ddition Building ay MetalTel

PER IFC

**chitects** Holt er ed Schroe





MetalTek 905 E. St. Paul Ave Waukesha, WI

FORMATION ON THIS DRAWING TAKES PRECEDEN OVER THE SPECIFICATIONS MANUAL IF THE DOCUMENTS HAVE CONFLICTING INFORMATION. DATE BY DESCRIPTION PROJECT #: XXXX DRWN BY: DATE: September 18th, 2013 Floor Plans

SHEET:

A1.0









#### 

- METAL FLASHING

#### - CLOSURE STRIP

SCALE: |-|/2" = |'-0"

#### DOOR & FRAME GENERAL NOTES

- I. HOLLOW METAL FRAMES SHALL BE WELDED AND GROUND
- FRAMES ONLY, WITH 3 CUSHIONS ON THE LATCH SIDE. 2. HOLLOW METAL FRAMES AND SIDELIGHT FRAMES SHALL BE
- PAINTED, UNLESS NOTED OTHERWISE.
   TYPICAL HOLLOW METAL FRAME TO BE DOUBLE RABBETED WITH 1/2" RETURN ON EACH SIDE.
- 4. GLASS IN SIDELIGHTS AND DOORS SHALL BE 1/4" TEMPERED AT INTERIOR APPLICATIONS, AND 1" INSULATING, TEMPERED AT
- EXTERIOR APPLICATIONS, UNLESS NOTED OTHERWISE. DOORS AND FRAMES AT RATED ASSEMBLIES SHALL MEET ALL CODES AND "UL" REQUIREMENTS AND BE PROPERLY LABELED
- FOR THE REQUIRED RATING.
- 6. ALL DOORS TO BE 1 3/4" THICK UNLESS NOTED OTHERWISE. 7. WOOD DOORS SHALL BE SOLID CORE WITH STAINED VENEER
- UNLESS OTHERWISE NOTED. 8. BRING ANY UNUSUAL DIMENSIONS, HEIGHTS, HARDWARE AND/OR CONDITIONS TO ARCHITECTS ATTENTION PRIOR TO
- CONSTRUCTION. 9. PROVIDE WEATHER-STRIPPING, THRESHOLD AND SWEEP AT ALL
- EXTERIOR DOORS. IO. DOOR HARDWARE SHALL BE COMPLIANT W/ CURRENT
- ACCESSIBILITY CODES, INCLUDING LEVER HANDLE LATCH AND LOCK SETS, AND DELAY ADJUSTABLE CLOSERS AS REQUIRED.
- I. HOLLOW METAL DOOR FRAMES AND SIDELIGHT FRAMES SHALL
   BE PAINTED, UNLESS NOTED OTHERWISE.
- I 2. CENTER MULLIONS AT DOUBLE DOOR (IF APPLICABLE) ARE TO BE REMOVABLE.
- 13. TYPICAL HOLLOW METAL FRAME TO BE 2' WIDE AND DOUBLE RABBITED WITH 1/2" RETURN ON EACH SIDE.
- 14. SEE OVERALL FLOOR PLANS FOR INTEGRAL SIDELIGHTS \$/ OR

SWEEP

TRANSOM WINDOW LOCATIONS. 15. 1/2" MAX. THRESHOLD FOR ALL DOORS

#### HARDWARE SETS





DOOF	R SCHEDULE													
DOOR	ROOM NAME	DO SI		DOOR SIZE		DOOR INFORMATION		FRAME INFORMATION				DETAILO		NOTES
NO.		WIDTH	HEIGHT	THICKNESS	MATERIAL	TYPE	FINISH	MATERIAL	TYPE	FINISH	HARDWARE	DLIAILS	(MIN.)	NUILO
BASE	BASEMENT													
101.1	X-RAY ROOM	4'-0"	10-0"	2	H.M.	С	PRIME ≰ PAINT	-	-	-	÷	-	-	-
101.2	X-RAY ROOM	3'-0"	7'-0"	3/4"	H.M.	В	PRIME ≰ PAINT	Н.М.	I	PRIME ∉ PAINT	ļ	-	-	-
101.3	X-RAY ROOM	3'-0"	7'-0"	3/4"	H.M.	В	PRIME ≰ PAINT	Н.М.	I	PRIME ∉ PAINT	ļ	-	-	-
101.4	X-RAY ROOM	3'-0"	7'-0"	3/4"	H.M.	A	PRIME ≰ PAINT	Н.М.	-	PRIME ∉ PAINT	2		÷	-

#### DOOR TYPES



Α





В HOLLOW METAL HOLLOW METAL I ⅔4" INSULATED, HOLLOW METAL DOOR  $| \frac{3}{4}|$  Hollow metal door

T.O.FTG ELEV. = 95'-6"

SCALE: 1/4" = 1'-0"



6" VINYL FACED BATT INSULATION -----

CONCRETE WALL

T.O.W ELEV. = 114'-0"

## CONCRETE DOOR & TRACK (BY OTHERS) -T.O. OHD FRAME ELEV. = 109'-8"

overhead door -----



GRAVEL FILL \_\_\_\_\_ CONCRETE FOOTING \_\_\_\_





## FRAME TYPES





C OVERHEAD DOOR CLOPAY 2" INSULATED STEEL GARAGE DOOR - MODEL 520S OR EQUAL



MetalTek 905 E. St. Paul Ave Waukesha, WI

INFORMATION ON THIS DRAWING TAKES PRECEDENCE OVER THE SPECIFICATIONS MANUAL IF THE DOCUMENTS HAVE CONFLICTING INFORMATION.							
NO.         DATE           △         -           △         -           △         -           △         -           △         -           △         -           △         -           △         -           △         -           △         -           △         -	BY 	DESCRIPTION					
PROJECT #: XXXX							
DATE:		September 18th, 201					
Floor Plans							

SHEET:

SCALE: 1/4" = 1'-0"

A1.1

DE	ESIGN CRITERIA					
1.	BUILDING CODE - INTERNA	TIONAL BUILDIN	G CODE	(IBC) <b>200</b>	)9 / ASC	E7-05
2.	ROOF LIVE LOADS			20	PSF	
3.	FLOOR LIVE LOADS					
	STORAGE (HEAVY)			250	PSF	
4.	SNOW LOADS					
	GROUND SNOW EXPOSURE FACTOR TEMPERATURE FACTOR IMPORTANCE FACTOF FLAT ROOF SNOW SLOPED ROOF SNOW DRIFTING SNOW, PER	Ce Ct Is Pf Ps UIRED, S	Pg 1.0 1.0 1.0 21.0 21.0 SEE PLAI	30 PSF PSF NS AND	PSF /OR CALCULATIONS	
5.	WIND LOADS					
	BASIC WIND SPEED OCCUPANCY CATEGO IMPORTANCE FACTOR EXPOSURE CATEGOR INTERNAL PRESSURE	DRY R Y COEFFICIENT	V Iw Gcpi	90 II 1.0 C +/- 0.18	MPH	
	COMPONENTS & CLAI DESIGNED FOR THE V TRIBUTARY AREA OF AREAS MAY BE USED	DDING NOT DESI VIND PRESSURE 10 SQUARE FEE <sup>-</sup> BASED ON DELE	GNED B` LISTED Г. WIND GATED	Y THE EN BELOW. PRESSU DESIGN	IGINEEI PRESS JRES FC CALCUI	R OF RECORD SHALL BE SURES ARE BASED ON A DR LARGER TRIBUTARY LATIONS.
	WALLS:					
	MAIN AREA.	POSITIVE NEGATIVE		18.0 19.5	PSF PSF	
	CORNERS :	POSITVE NEGATIVE		18.0 24.0	PSF PSF	
	ROOF:					
		POSITIVE NEGATIVE		10.0 21.3	PSF PSF	
	CORNERS:	POSITIVE NEGATIVE		10.0 24.7	PSF PSF	
		POSITIVE NEGATIVE		10.0 33.0	PSF PSF	
6.	SEISMIC LOADS:					
	OCCUPANCY CATEGO IMPORTANCE FACTOR SITE CLASS	DRY R	le	ll 1.0 D		
	MAPPED SPECTRAL R Ss S1	ESPONSE ACCE	LERATIC	ONS 0.111 0.045		
	SPECTRAL RESPONS	E COEFFICIENTS	i	0.118		

SD1 0.072 SEISMIC DESIGN CATEGORY STRUCTURAL STEEL SYSTEM NOT SEISMIC FORCE RESISTING SYSTEM SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE RESPONSE MODIFICATION FACTOR R 3.0 RESPONSE COEFFICIENT 0.039 Cs ANALYSIS PROCEDURE PER PEMB MFG

#### **GENERAL REQUIREMENTS**

- THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO 1. NOT INCLUDE THE METHOD OF CONSTRUCTION. CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO: BRACING, SHORINGFOR LOADS DUETO CONSTRUCTION EQUIPMENT, TEMPORARY STRUCTURES, AND PARTIALLY COMPLETED WORK. OBSERVATION VISITS TO THE SITE BY STRUCTURAENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS
- THE ARCHITECT AND/OR ENGINEER OF RECORD SHALL NOT HAVE CONTROL OVER OR BE IN CHARGE OF. AND SHALL NOT BE RESPONSIBLE IN ANY WAY FOR CONSTRUCTION MEANS, METHODS TECHNIQUES, SEQUENCES, OR PROCEDURES, OR FOR SAFETY OR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH ANY CONSTRUCTION ACTIVITIES, SINCE THESE ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.
- SUBMITTALS PREPARED BY SUBCONTRACTORS SHALL BE REVIEWED B&ONTRACTOR PRIOR TO SUBMITTING TO ARCHITECT/ENGINEER.
- CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS AT THE JOB SITE. ANY DISCREPANCIES BETWEEN THE CONDITIONS FOUND ANO THOSE INDICATED IN THE CONTRACT DOCUMENTS SHALL BE BROUGHT TO THE ATTENTION OF ARCHITECT PRIOR TO PROCEEDING WITH THE WORK.
- SEE DOCUMENTS FROM OTHER DISCIPLINES FOR FLOOR, WALL, ANO ROOF OPENINGS, TRENCHES, PITS, PIPE SLEEVES, EQUIPMENT PADS, METAL PAN STAIRS, MISCELLANEOUS IRON, ETC.
- DO NOT PLACE PIPES, DUCTS, CHASES, ETC. IN STRUCTURAL BEAM ANO COLUMN MEMBERS. DO NOT CUT ANY STRUCTURAL MEMBER FOR PIPES, DUCTS, ETC., UNLESS NOTED OTHERWISE. NOTIFY STRUCTURAL ENGINEER WHEN DOCUMENTS BY OTHER DISCIPLINES SHOW OPENINGS, POCKETS, ETC. NOT INDICATED IN THE STRUCTURAL DRAWINGS BUT ARE LOCATED IN THE STRUCTURAL MEMBERS. CONTRACTOR SHALL OBTAIN PRIOR APPROVAL FROM STRUCTURAL ENGINEER FOR INSTALLATION OF SUCH PIPES, DUCTS, CHASES, ETC.
- DETAILS LABELED "TYPICAL" ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE LOCATIONS SPECIFICALLY INDICATED. WHERE A DETAIL IS NOT INDICATED, THE DETAIL SHALL BE THE SAME AS FOR OTHER SIMILAR CONDITIONS
- CONTRACTOR DESIGNED ELEMENTS SHALL BE DESIGNED BY LICENSED PROFESSIONAL 8 ENGINEERS REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, DESIGN LOAD DATA, SUPPORT REACTIONS, ANO CERTIFICATION THAT ELEMENTS WERE DESIGNED FOR LOADS SPECIFIED IN THE CONTRACT DOCUMENTS OR IN THE BUILDING CODE. ALL DOCUMENTS NOTED SHALL BE SEALED BY THE LICENSED ENGINEER. IF CRITERIA INDICATED ARE NOT SUFFICIENT. SUBMIT A WRITTEN REQUEST FOR ADDITIONAL INFORMATION TO THE ARCHITECT. THE FOLLOWING ELEMENTS ANO THEIR CONNECTIONS SHALL BE CONTRACTOR DESIGNED:
  - PREFABRICATED METAL BUILDING

#### SPREAD FOUNDATIONS

- FOUNDATIONS ARE DESIGNED TO BE SUPPORTED ON APPROVED EXISTING SUBGRADE OR APPROVED COMPACTED STRUCTURAL FILL HAVING A PRESUMED MINIMUM ALLOWABLE BEARING CAPACITY OF 2,000 PSF.
- THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE 2. CONDITIONS DESCRIBED IN THE DRAWINGS, SPECIFICATIONS, TEST BORINGS OR GEOTECHNICAL REPORTS. THIS DATA IS INCLUDED TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION, AND TO REPRESENT CONDITIONS ONLY AT SPECIFIC LOCATIONS AT THE PARTICULAR TIME THE OBSERVATIONS WERE MADE.
- ALL EXTERIOR FOUNDATIONS SHALL BEAR ON APPROVED SUBGRADE AT MINIMUM DEPTH OF 4'-0 BELOW ADJACENT FINISH EXTERIOR GRADE.
- FOOTING ELEVATIONS SHOWN ON THE DRAWINGS REPRESENT ESTIMATED DEPTHS AND ARE NOT 4 TO BE CONSTRUED AS LIMITING THE AMOUNT OF EXCAVATION REQUIRED TO REACH SUITABLE BEARING MATERIAL.
- THE CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS IN ALL EXCAVATIONS AS REQUIRED TO PREVENT HORIZONTAL MOVEMENT OR VERTICAL SETTLEMENT OF SURROUNDING SOIL AND/OR PROPERTY WHICH WILL ENDANGER LIVES OR PROPERTY.
- THE CONTRACTOR SHALL PROVIDE CONTROL OF SURFACE AND SUBSURFACE WATER PROMPTLY TO ENSURE THAT ALL FOUNDATION WORK IS PERFORMED IN A DRY CONDITION.
- FOUNDATIONS SHALL NOT BE PLACED ON FROZEN SUBGRADE 7
- THE CONTRACTOR SHALL PROTECT IN-PLACE FOUNDATIONS AND SLABS-ON-GRADE FROM FROST PENETRATION UNTIL THE PROJECT IS COMPLETE.
- FOUNDATION WALLS SHALL BE BRACED DURING BACKFILLING AND COMPACTION OPERATIONS. BRACING SHALL BE LEFT IN PLACE UNTIL PERMANENT STRUCTURAL SUPPORT SYSTEM IS INSTALLED AND APPROVED BY THE ENGINEER.
- WHERE FOUNDATION WALLS HAVE FILL ON BOTH SIDES, BACKFILLING SHALL BE DONE 10 SIMULTANEOUSLY ON BOTH SIDES OF THE WALL

#### CONCRETE

- 1. CONCRETE WORK SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING STANDARDS:
  - A. ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE".
  - B. ACI MCP "MANUAL OF CONCRETE PRACTICE".
  - C. ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
  - D. ACI 318.1 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL PLAIN CONCRETE".
- 2. CONCRETE SHALL HAVE A MINIMUM 28 DAY ULTIMATE COMPRESSIVE STRENGTH AS FOLLOWS:

OOTINGS	3.000	PSI	
LAB ON GRADE	4,000	PSI	
ALLS/GRADE BEAMS	4,000	PSI	

- 3. CONCRETE MIX DESIGN (INCLUDING AGGREGATE SIZE, WATER CEMENT RATIO, AIR ENTRAINMENT, ADMIXTURES, SLUMP AND HISTORY OF BREAK TESTS ) SHALL BE SUBMITTED TO THE EOR FOR APPROVAL PRIOR TO THE COMMENCEMENT OF ANY WORK.
  - MAXIMUM WATER/CEMENT RATIO PERMITTED IS:
  - 0.50 FOR SLABS ON GRADE 0.54 FOR BELOW GRADE CONCRETE 0.45 FOR EXPOSED CONCRETE
- 4. CONCRETE WHICH WILL BE EXPOSED TO THE WEATHER SHALL HAVE AIR-ENTRAINING ADMIXTURE AS REQUIRED TO PROVIDE 4 - 6 % AIR ENTRAINMENT.
- 5. MAXIMUM AGGREGATE SIZE SHALL BE AS FOLLOWS: 3/4" FOR SLABS ON GRADE, WALLS, BEAMS & COLUMNS **1" FOR FOOTINGS** 3/8" FOR TOPPING SLABS
- 5. CONCRETE SHALL BE EVALUATED ACCORDING TO METHOD 1 OR METHOD 2 AS DESCRIBED IN ACI 301. THE RESULTS OF THESE ANALYSES SHALL BE SUBMITTED TO THE EOR FOR APPROVAL PRIOR TO ANY WORK
- 6. AT THE OWNERS OPTION, THE CONTRACTOR SHALL MAKE PROVISIONS TO ALLOW AN INDEPENDENT TESTING AGENCY, HIRED BY THE OWNER, TO CAST 4 TEST CYLINDERS FOR EACH 50 CUBIC YARDS OF CONCRETE PLACED, OR FOR ANY DAY'S OPERATION. THE TESTING AGENCY SHALL BE RESPONSIBLE FOR CASTING AND CURING SPECIMENS IN COMPLIANCE TO ASTM C31 AND CASTING TESTING SPECIMENS IN COMPLIANCE TO ASTM C39.
- 7. CONSTRUCTION JOINTS SHOWN ON THE CONTRACT DRAWINGS SHALL NOT BE ALTERED WITHOUT WRITTEN APPROVAL OF THE ARCHITECT.
- 8. DRAWINGS SHOWING THE LOCATION OF CONSTRUCTION JOINTS, CONTROL JOINTS, AND PLACING SEQUENCE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO THE PREPARATION OF REINFORCING SHOP DRAWINGS.
- 9. GROUT USED TO SET PLATES SHALL BE NON-SHRINK AND NON-METALLIC. 10. THE CONTRACTOR SHALL USE SMOOTH FORMS FOR EXPOSED CONCRETE SURFACES. BOARD FORMS MAY BE USED FOR UNEXPOSED CONCRETE SURFACES. EARTH FORMS
- ARE FORBIDDEN. 11. PROVIDE A MINIMUM OF 6" OF COMPACTED GRANULAR FILL UNDER ALL SLABS ON GRADE.
- 12. FLOOR FLATNESS AND LEVELNESS OF SLAB ON GRADE CONCRETE SHALL HAVE A MINIMUM F-NUMBER OF Ff25/FL20 AS RECOGNIZED BY THE MOST CURRENT VERSION OF ASTM E 1155 AND ACI 302.1. SEE SPECIFICATION FOR FURTHER REQUIREMENTS.

#### CONCRETE REINFORCING

- 1. DETAILING, FABRICATION AND ERECTION OF REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING:
  - A. ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT".
  - B. ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
  - C. MSP2 "CRSI MANUAL OF STANDARD PRACTICE".
  - D. AWS D1.4 "STRUCTURAL WELDING CODE REINFORCING STEEL".
  - E. WRI "WELDED WIRE FABRIC MANUAL OF STANDARD PRACTICE".
- 2. STEEL REINFORCING BARS SHALL CONFORM TO ASTM A615 GRADE 60, DEFORMED. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
- 3. THE REINFORCEMENT FABRICATOR SHALL PROVIDE AND SCHEDULE ON SHOP DRAWINGS ALL REQUIRED REINFORCING STEEL AND NECESSARY ACCESSORIES TO HOLD REINFORCEMENT SECURELY IN PLACE AT THE CORRECT LOCATIONS.
- 4. UNLESS SHOWN ON THE DRAWINGS OTHERWISE, THE REQUIRED CLEARANCE FOR **REINFORCEMENT SHALL BE PER ACI 318:** 
  - A. CONCRETE PLACED DIRECTLY ON EARTH
  - B. CONCRETE EXPOSED TO EARTH OR WEATHER #6 & LARGER
  - C. CONC. NOT EXPOSED TO EARTH OR WEATHER #14 & LARGER
- 5. THE CONTRACTOR SHALL REFER TO TYPICAL DETAILS SHOWN ON THE CONTRACT DRAWINGS FOR ADDITIONAL REINFORCING REQUIREMENTS.
- 6. WHERE REINFORCEMENT IS REQUIRED IN SECTIONS, REINFORCEMENT IS CONSIDERED TYPICAL WHERE EVER THE SECTION APPLIES.
- 7. WELDED WIRE FABRIC SHALL HAVE A MINIMUM OF 6" LAP AND BE TIED TOGETHER.
- 8. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF COMPLETION OF REINFORCEMENT INSTALLATION AND ALLOW AT LEAST 24 HOURS BEFORE SCHEDULED CONCRETE PLACEMENT FOR THE ARCHITECT TO INSPECT REINFORCEMENT.

- 3" #5 & SMALLER 1 1/2" 1 1/2" #11 & SMALLER

#### STRUCTURAL STEEL

- 1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING LATEST EDITION:
  - A. AISC "SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STEEL FOR BUILDINGS".
  - B. AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
  - C. AWS D1.1 "STRUCUTRAL WELDING CODE -STEEL".
- D. AISC "STRUCTURAL STEEL DETAILING MANUAL".
- 2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING

HOT ROLLED W AND WT SHAPES	<b>ASTM 4992</b>	$(E_{V} = 50 \text{ KSL})$
ANGLES CHANNELS AND PLATES	ASTM A36	(Fy = 36  KSL)
		(Ty = 30  KSI)
S, M, & NP SNAPES	ASTIVI ASO	(Fy - 30 KSI)
STEEL PIPE	ASTM A53 GR B	(Fy = 35 KSI)
RECTANGULAR HSS	ASTM A500 GR B	(Fy = 46 KSI)
ROUND HSS	ASTM A500 GR B	(Fy = 42 KSI)
HIGH STRENGTH BOLTS	ASTM A325	
HEAVY HEX NUTS	ASTM A563	
HARDENED STEEL WASHERS	ASTM F436	
ANCHOR RODS	ASTM F1554 GR 36	(Fy = 36 KSI)
THREADED RODS	ASTM A36	(Fy = 36 KSI)
HEADED STUD ANCHORS	ASTM A108	

- 3. PROVIDE 2 MIL THICKNESS RED OR GREY OXIDE PRIMER ON ALL STEEL SURFACES UNLESS NOTED.
- 4. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED PER ASTM
- A123 AND FASTENERS HOT DIPPED GALVANIZED PER ASTM A153. 5. ANCHOR RODS SHALL BE PRESET WITH TEMPLATES.

POSITION INWHICH THE WELD IS TO BE MADE.

WRITTEN APPROVAL FROM THE EOR.

- 6. LEVELING PLATES AND BEARING PLATES SHALL BE SET IN A FULL BED OF NON-SHRINK GROUT.
- 7. CONNECTIONS MAY BE BOLTED OR WELDED AT THE FABRICATORS OPTION. BOLTED CONNECTIONS SHALL BE AS FOLLOWS:
  - A. MINIMUM BOLT DIAMETER OF 3/4" UNLESS NOTED.
  - B. HIGH STRENGTH BOLTS IN SINGLE OR DOUBLE SHEAR UNLESS NOTED.
  - C. SIMPLE SHEAR CONNECTIONS SHALL BE CAPABLE OF END ROTATION PER AISC REQUIREMENTS FOR UNRESTRAINED MEMBERS.
- 8. THE MINIMUM FILLET WELD SIZE SHALL NOT BE LESS THAN 3/16" UNLESS NOTED.
- 9. ALL WELDS SHALL USE WELD METAL CONFORMING TO E70XX AND CONFORMING TO AWS WELDING PROCEDURES AND STANDARDS.
- 10. ALL WELDS SHALL BE MADE BY AWS CERTIFIED WELDERS CERTIFIED IN THE
- 11. THE ERECTION OF ANY STRUCTURAL STEEL MEMBERS SHALL NOT COMMENCE UNTIL ALL SUPPORTING CONCRETE/MASONRY ELEMENTS HAVE ATTAINED AT LEAST 75% OF THEIR INTENDED MINIMUM COMPRESSIVE STRENGTH.
- 12. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SUPPORTS AS REQUIRED FOR THE SAFE ERECTION OF ALL STEEL. TEMPORARY BRACING SHALL REMAIN IN PLACE UNTIL PERMANENT BRACING HAS BEEN INSTALLED AND FLOOR SLAB CONCRETE HAS ATTAINED 75% OF ITS REQUIRED STRENGTH.
- 13. STRUCTURAL STEEL SHALL BE TRUE AND PLUMB BEFORE FINAL BOLTING OR WELDING OF CONNECTIONS.
- 14. THE CONTRACTOR SHALL NOT MODIFY OR CUT ANY STRUCTURAL STEEL WITHOUT
- 15. THE CONTRACTOR SHALL FIELD TOUCH UP ALL ABRASIONS, BURNS, AND SIMILAR DEFECTS IN PAINT OF STRUCTURAL STEEL.

#### METAL BUILDING SYSTEM

- 1. METAL BUILDING WORK SHALL CONFORM TO THE FOLLOWING LATEST EDITION:
  - A. AISC "SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STEEL FOR BUILDINGS".
  - B. AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
  - C. AWS D1.1 "STRUCTURAL WELDING CODE -STEEL".
  - D. AISC "STRUCTURAL STEEL DETAILING MANUAL"
  - E. MBMA "METAL BUILDING SYSTEMS MANUAL".
  - F. DESIGN LOADS AND CODE AS NOTED ON THESE DRAWINGS.
- 2. THE METAL BUILDING SHALL BE DESIGNED AND FABRICATED BY A MBMA MEMBER MANUFACTURER.
- 3. THE METAL BUILDING MANUFACTURER (MBM) SHALL PROVIDE STAMPED DRAWINGS AND CALCULATIONS BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE JURISDICTION IN WHICH THE BUILDING IS LOCATED.
- 4. THE MBM SHALL CONFORM TO THE FOLLOWING DEFLECTION CRITERIA: AISC - "DESIGN GUIDE #3 SERVICEABILITY DESIGN CONSIDERATIONS FOR STEEL BUILDING" UNLESS NOTED ON THE DRAWINGS.
- 5. ANCHOR RODS SHALL BE PRESET WITH TEMPLATES.
- 6. LEVELING PLATES AND BEARING PLATES SHALL BE SET IN A FULL BED OF NON-SHRINK GROUT.
- 7. THE MBM SHALL BE RESPONSIBLE FOR ALL CONNECTIONS, STIFFENERS, AND ETC. REQUIRED TO SAFELY ERECT THE BUILDING. THE MBM IS REPONSIBLE FOR ANY REQUIRED HOLES SHOWN PASSING THROUGH THE MBM STEEL ON THE DRAWINGS.
- 8. THE MBM SHALL PROVIDE FOUNDATION REACTIONS, COLUMN LOCATIONS AND BASE PLATE SIZES TO THE ENGINEER IN A TIMELY MANNER. CHANGES TO, OR OMMISSIONS OF REACTIONS, ETC, BY THE MBM THAT REQUIRE REDESIGN OF THE FOUNDATIONS WILL REQUIRE ADDITIONAL ENGINEERING FEES.
- 9. ALL WELDS SHALL USE WELD METAL CONFORMING TO E70XX AND CONFORMING TO AWS WELDING PROCEDURES AND STANDARDS.
- 10. ALL WELDS SHALL BE MADE BY AWS CERTIFIED WELDERS CERTIFIED IN THE POSITION IN WHICH THE WELD IS TO BE MADE.
- 11. THE ERECTION OF ANY STRUCTURAL STEEL MEMBERS SHALL NOT COMMENCE UNTIL ALL SUPPORTING CONCRETE/MASONRY ELEMENTS HAVE ATTAINED AT LEAST 75% OF THEIR INTENDED MINIMUM COMPRESSIVE STRENGTH.
- 12. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SUPPORTS AS REQUIRED FOR THE SAFE ERECTION OF ALL STEEL. TEMPORARY BRACING SHALL REMAIN IN PLACE UNTIL PERMANENT BRACING HAS BEEN INSTALLED AND FLOOR SLAB CONCRETE HAS ATTAINED 75% OF ITS REQUIRED STRENGTH.
- 13. STRUCTURAL STEEL SHALL BE TRUE AND PLUMB BEFORE FINAL BOLTING OR WELDING OF CONNECTIONS.
- 14. THE CONTRACTOR SHALL NOT MODIFY OR CUT ANY STRUCTURAL STEEL WITHOUT WRITTEN APPROVAL FROM THE EOR AND MBM.
- 15. THE CONTRACTOR SHALL FIELD TOUCH UP ALL ABRASIONS, BURNS, AND SIMILAR DEFECTS IN PAINT OF STRUCTURAL STEEL.

REINFORCEMENT DEVELOPMENT AND SPLICE LENGTH SCHEDULE										
Fy=60 KSI f'c=3000 PSI										
BAR SIZE		#3	#4	#5	#6	#7	#8	#9	#10	#11
CLASS A	TOP BARS	22	29	36	43	63	72	81	91	101
SPLICE LENGTH	OTHERS	17	22	28	33	48	55	62	70	78
CLASS B	TOP BARS	28	37	47	56	81	93	105	118	131
SPLICE LENGTH	OTHERS	22	29	36	43	63	72	81	91	101
REINFORCEMENT	DEVELOPMEN	t ani	D SF	PLIC	EL	EN	GTH	I SC	HED	ULE
	Fy=60 K	SI f'c=	-400	0 P	SI					
BAR SIZE		#3	#4	#5	#6	#7	#8	#9	#10	#11
CLASS A	TOP BARS	19	25	31	37	54	62	70	79	87
SPLICE LENGTH	OTHERS	15	19	24	29	42	48	54	61	67
CLASS B	TOP BARS	24	32	40	48	70	80	91	102	113
SPLICE LENGTH	OTHERS	19	25	31	37	54	62	70	79	87
<ul> <li>NOTES: (APPLY TO BOTH 3000 PSI &amp; 4000 PSI CONCRETE)</li> <li>1. ALL SPLICE LENGTHS SHALL BE CLASS B UNLESS NOTED OTHERWISE.</li> <li>2. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE.</li> <li>3. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPLICE LENGTHS ARE CALCULATED PER ACI 318-05, SECTIONS 12.2.5, RESPECTIVELY. TABULATED VALUES FOR BEAMS AND COLUMNS ARE BASED ON TRANSVERSE REINFORCING AND CONCRETE COVER MEETING MIN. CODE REQUIREMENTS. LENGTHS ARE IN INCHES.</li> <li>4. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.</li> </ul>										
5. SPLICE AND D ARE BASED OI BEAMS O	EVELOPMENT I N CASE 1 PER ( IR COLUMNS:	LENG CRSI COVI AND 2.0 B	(199 ER <i>F</i> C.C AR I	5 IN 96): AT L . SF DIA	IH EAS PAC	IS S ST 1 ING	I.0 E	EDU BAR LEA	DIA. ST	
ALL OTHE	ERS:	COVI AND	ER A C.C	AT L . SF	EAS PAC	ST 1 ING	1.0 E AT	3AR LEA	DIA. AST	

3.0 BAR DIA.

	SHA Schroeder & Holt Architects
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	COREAEngineering 2308 N. Corporate Pkwy. Suite 300 Mequon, WI 53092 262.236.9372 Collaborative   Communicative
	METAL TEK X-RAY LONG VAULT 985 E. ST. PAUL AVE. WAUKESHA, WI 53188
	INFORMATION ON THIS DRAWING TAKES PRECEDENCE OVER THE SPECIFICATIONS MANUAL IF THE DOCUMENTS HAVE CONFLICTING INFORMATION.         NO.       DATE       BY       DESCRIPTION         .       .       .       .         . </th
	PROJECT #: 14046 DRWN BY: DATE: 7/18/14 GENERAL STRUCTURAL
	NOTES SHEET:
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STRUCTURAL DRAWING INDEX							
SHEET NAME							
GENERAL STRUCTURAL NOTES							
FOUNDATION PLAN							
ROOF FRAMING PLAN							
STRUCTURAL BUILDING SECTIONS							
STRUCTURAL DETAILS							





7/18/14

FOUN SHEET:

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BID

FOUNDATION PLAN

S2.0

	WALL SCHEDULE
THICKNESS	REMARKS
3' - 4"	CONCRETE WALL W/ #5 @ 12" OC VERT & #5 @ 8" OC HOR EACH FACE
1' - 4"	CONCRETE WALL W/ #5 @ 18" OC VERT & HOR EACH FACE

#### WALL FOOTING SCHEDULE

DIMEN	SIONS					
		LONGIT	UDINAL	TRANS		
WIDTH	DEPTH	QUANTITY	SIZE	SIZE	SPACING	NOTES
4' - 0"	1' - 0"	5	4	4	4' - 0"	
5' - 4"	1' - 0"	5	5	0	0' - 0"	

#### FOUNDATION PLAN NOTES:

- FLOOR SLAB: 8" CONCRETE SLAB ON GRADE WITH #3 @ 18" OC EACH WAY, MID-DEPTH, OR FORTA FERRO FIBER REINFORCING (5 LB/CY), ON 6" COMPACTED GRAVEL. TOP OF SLAB ELEVATION = 100' 0" UNO.
- 2. TOP OF EXTERIOR FOOTING ELEVATION = 95' 6" UNO, MATCH EXISTING.
- 3. TOP OF WALL ELEVATION = 114'-0" UNO.
- 4. SEE SHEET S1.0 FOR GENERAL NOTES AND SCHEDULES.
- 5. FOR FOUNDATION WALL CONSTRUCTION JOINTS AND TYPICAL REINFORCING DETAILS, SEE SHEET S5.0.









		STEEL CO
MARK	SIZE	BOTTO
SC1	W10X39	E
SC2	PEMB COL	PER
-	-	•

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BID & PERMIT SET	INFORMATION ON THIS DRAWING TAKES PRECEDENCE OVER THE SPECIFICATIONS MANUAL IF THE DOCUMENTS HAVE CONFLICTING INFORMATION.



![](_page_6_Figure_1.jpeg)

![](_page_6_Picture_2.jpeg)

![](_page_7_Figure_0.jpeg)

![](_page_7_Figure_1.jpeg)

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S5.0

SEE PLAN -

S5.0

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S5.0

![](_page_7_Figure_2.jpeg)