

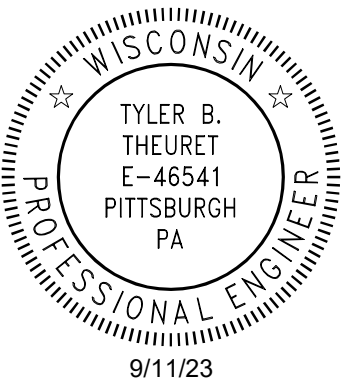
UNIVERSAL STEEL BUILDINGS

MIKE ARMBRUSTER

WAUKESHA, WI 53188

FO# 27630

Building 1 of 1



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GENERAL		MATERIALS		ASTM DESIGNATION	MINIMUM YIELD	MATERIALS	ASTM DESIGNATION	MINIMUM YIELD
All materials included in the Metal Building System are in accordance with the manufacturer's standard materials and details unless otherwise specified on the order documents. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 2.1)		Hot-Rolled Mill Sections		A 36, A 572, A 992	Fy = 36 ksi and/or 50 ksi	Roof and Wall Sheeting	A 792, Gr. 50 Class 1 A 792, Gr. 80	Fy = 50 ksi Fy = 80 ksi
		Structural Steel Plates		A 572, A 1011	Fy = 55 ksi	Mild Steel Bolts	A 307	Fy = 36 ksi
		Structural Steel Bars		A 572 or A 529	Fy = 55 ksi	High Strength Bolts	F3125: A 325-N A 490-N	Fy = 92 or 81 ksi N/A
		Cold Formed Light Gauge Shapes		A 653 Gr. 55	Fy = 55 ksi	Anchor Rods (If supplied)	A 36	Fy = 36 ksi
		Cable Bracing		A 475, EHS	N/A	Pipe and Hollow Structural Sections	A 500 Gr. B	Fy = 42 ksi, 46 ksi
Rod Bracing		A 36	Fy = 36 ksi					
		CORRECTION OF ERRORS AND REPAIRS						
		The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping, and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 7.14; CISC Code of Standard Practice, December 2015, Clause 7.15; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.10).						
ADJACENT EXISTING BUILDINGS		DRAWING DISCREPANCIES						
The manufacturer does not investigate the influence of the Metal Building System on adjacent existing buildings or structures. The end customer assures that such buildings and structures are adequate to resist snow loads or other conditions as a result of the presence of the Metal Building System. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.5)		In case of discrepancies between the manufacturers steel plans and plans for other trades, the manufacturers steel plans govern. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 3.3; CISC Code of Standard Practice, December 2015, Clause 3.4; MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.1).						
SHOP-PRIMED STEEL		DELIVERIES						
All structural members of the Metal Building System not fabricated of corrosion resistant material or protected by corrosion resistant coating are painted with one coat of shop primer. All surfaces to receive shop primer are cleaned of loose rust, loose mill scale and other foreign matter by using, as a minimum the hand tool cleaning method SSPC-SP2 (Steel Manual, Structures Painting Council) prior to painting. The coat of shop primer is intended to protect the steel framing for only a short period of exposure to ordinary atmospheric conditions. Shop-primed steel should be placed on blocking to prevent contact with the ground, and so positioned as to minimize water holding pockets, dust, mud an other contamination of the primer film. Repairs of damage to primed surfaces and or removal of foreign material due to improper field storage or site conditions are not the responsibility of the manufacturer. (CISC Code of Standard Practice, December 2015, Clauses 6.8 & 6.9; (MBMA 2018 Metal Building Systems Manual, Part IV, Section 4.2.4).		Delivery of any material by the manufacturers carrier, a common carrier, or to purchasers/ customers own leased, chartered, or authorized conveyance shall constitute delivery to builder, and thereafter, such material shall be at builders risk. If builder chooses to use its own, or private carrier, it shall be solely responsible for compliance with all applicable government regulations. All charges shall be borne by the builder. The manufacturers responsibility for damage or loss ceases upon delivery of shipment to carrier. The manufacturer will endeavor to deliver on the required date. The manufacturers truck is not considered as being late if deliveries are between 8am - 12pm (morning) and 12pm - 5pm (afternoon). However, the manufacturer cannot be held responsible for circumstances beyond our control. For deliveries via the manufacturers truck, the manufacturer will only honor claims that were approved by the customer service department at the time of delivery. For deliveries via contract carriers, it is the responsibility of the customer to file claims with the carrier. The manufacturer cannot assume any liability for the claim.						
ERECTION-GENERAL		SHORTAGES						
The erector, by entering into contract to erect the building, holds itself out as skilled in the erection of Metal Building Systems and is responsible for complying with all applicable local, federal, and state construction and safety regulations including OSHA regulations as well as any applicable requirements of local, national, or international union rules or practices. (CISC Code of Standard Practice, December 2015, Clause 7.3; (MBMA 2018 Metal Building System Manual, Part IV, Section 6.9).		The purchaser /customer should make an inspection upon arrival of all building components. The purchaser/customer must note on the freight bill any missing item(s) and notify the manufacturers customer service department immediately; otherwise, the manufacturer cannot be held responsible for any shortages. If any item is damaged, note on the bill of lading and file a claim with the freight agent. Concealed shortages must be reported to the manufacturers customer service department within the following time frames (date from receipt of first delivery), based on the project shipment size, i.e., number of truck loads used in delivery. 1 to 3 loads....2 weeks 4 loads and over....3 weeks The manufacturers responsibility for shortages expires at the end of these time periods.						
		FABRICATION ERRORS						
The erector shall erect the Metal Building System in accordance with the erection drawings, the Erection and Detail Manual (2019), and / or the Seam-Lok Technical - Erection manual (2019) as furnished by the manufacturer. The aforementioned erection information is intended to illustrate the layout of the framing members, provide the associated connection details, and suggests sequence of erection. It is not intended to specify any particular method of erection to be followed by the erector. The erector remains solely responsible for the safety and appropriateness of all techniques and methods utilized by its crews in the erection of the Metal Building System. The erector is responsible for supplying any safety devices such as scaffolds, runways, nets, et, which may be required to safely erect the Metal Building System. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.9) The manufacturer expressly disclaims any responsibility for injury to persons in the course of erection or for damages to the product itself. Field erection of a Pre-Engineered Metal Building, as in all construction projects, involves hazards to persons within the area of the construction and risk of damage to the property itself. Only experienced persons who are skilled and qualified in the erection of Metal Building Systems should be permitted to field-erect a building due to the hazards of this construction activity. The manufacturer is not responsible for the erection of the Metal Building System, the supply of any tools or equipment, or any other field work. The manufacturer provides no field supervision for the erection of the structure nor does the manufacturer perform any intermediate or final inspections of the Metal Building System during or after erection.		The purchaser/customer is responsible for contacting the customer service department to advise the manufacturer of fabrication problems and corresponding cost estimates. The manufacturer will be responsible for providing the builder with verbal approval to proceed with appropriate field corrections. This will be done in a timely manner. IF THE BUILDER PROCEEDS WITH CORRECTIVE WORK WITHOUT THE MANUFACTURERS APPROVAL, HE DOES SO AT HIS OWN RISK. The manufacturer shall not be responsible for any claims where the purchaser/customer has not documented the problem, its correction, and reasonable costs for repair, and submitted this documentation for payment within 30 days of the occurrence.						
		INVOICE PAYMENT						
The erector shall furnish temporary guys and bracing where needed for squaring, plumbing, and securing the structural framing against loads, such as wind loads acting on the exposed framing as well as loads due to erection equipment and erection operation, but not including loads resulting from the performance of work by others. Bracing furnished by the manufacturer for the Metal Building System cannot be assumed to be adequate during erection. Temporary supports such as temporary guys, braces, false work, cribbing, or other elements required for the erection operation will be determined, erected, and installed by the erector. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 7.10.3; CISC Code of Standard Practices, December 2015, Clause 1.5; MBMA 2018 Metal Buildings System Manual, Part IV, Section 6.2.1.5).		By acceptance of the materials of services set forth in the invoice, the purchaser/customer agrees to pay the invoice amount within the time period specified on the invoice. AT NO TIME IS IT ACCEPTABLE TO DEDUCT A BACK CHARGE OR SHORTAGE FROM AN INVOICE.						
ERECTION TOLERANCES		SAFETY PROCEDURES						
U.S. ; Erection tolerances are those set forth in AISC code of standard practice except individual members are considered, plumb, level and aligned if the deviation does not exceed 1:500. (AISC Code of Standard Practice for Steel Buildings and Bridges June 15, 2016 Section 7.13.1; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.8) Canada; Erection tolerances are those set forth in CISC Code of Standard Practice except individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. (CISC Handbook of Steel Construction, Eleventh Edition, Third Revised Printing, Part 1, Clause 29.3; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.8)		The manufacturer is committed to manufacturing a quality product that can be erected safely. Although good job site practices and a commitment to safety by the erector are beyond the control of the manufacturer, the manufacturer highly recommends the erector provide good, safe working conditions on the job site. The erector should follow all local, state, and federal health and safety regulations at all times. Accident prevention practices should be implemented and each employee should know emergency procedures. The manufacturer also recommends daily meetings to discuss erection safety procedures. For additional information concerning federal health and safety regulations, contact the occupational safety and health administration (osha).						
BOLT TIGHTENING		U.S. Department of Labor Occupational Safety and Health Administration 200 Constitution Avenue, N.W. Washington, DC 20210 www.osha.gov						
The proper tightening and inspection of all fasteners is the responsibility of the erector (Reference RCSC for structural joints using high strength bolts; June 11, 2020). All high strength (ASTM F3125, A325, A490) bolts and nuts must be tightened by the "turn-of-the-nut" method unless otherwise specified by the end customer in the contract documents. Inspection of high strength bolt and nut installation by other than the erector must also be specified in the contract documents and the erector is responsible for ensuring that the installation procedures are compatible prior to the start of erection (CISC Handbook of Steel Construction, Eleventh Edition, Third Revised Printing, Part 1, Clause 23.7.2), (MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.9).		The manufacturer shall not be responsible for personal injury or property damage as a result of failure to follow all applicable safety regulations and material handling and installation recommendations.						

OLYMPIA

STEEL BUILDINGS

Olympia Steel Buildings®
400 Island Avenue • McKees Rocks, PA 15136

888-449-7756

MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/ 1/23

REVISION: 0

ENG: TBT

DWN: BJC

APPD: TBT

F.O.27630

CITY: WAUKESHA

ST / PV: WI

REVISION HISTORY

DESCRIPTION

DATE

REV.

DRAWING STATUS

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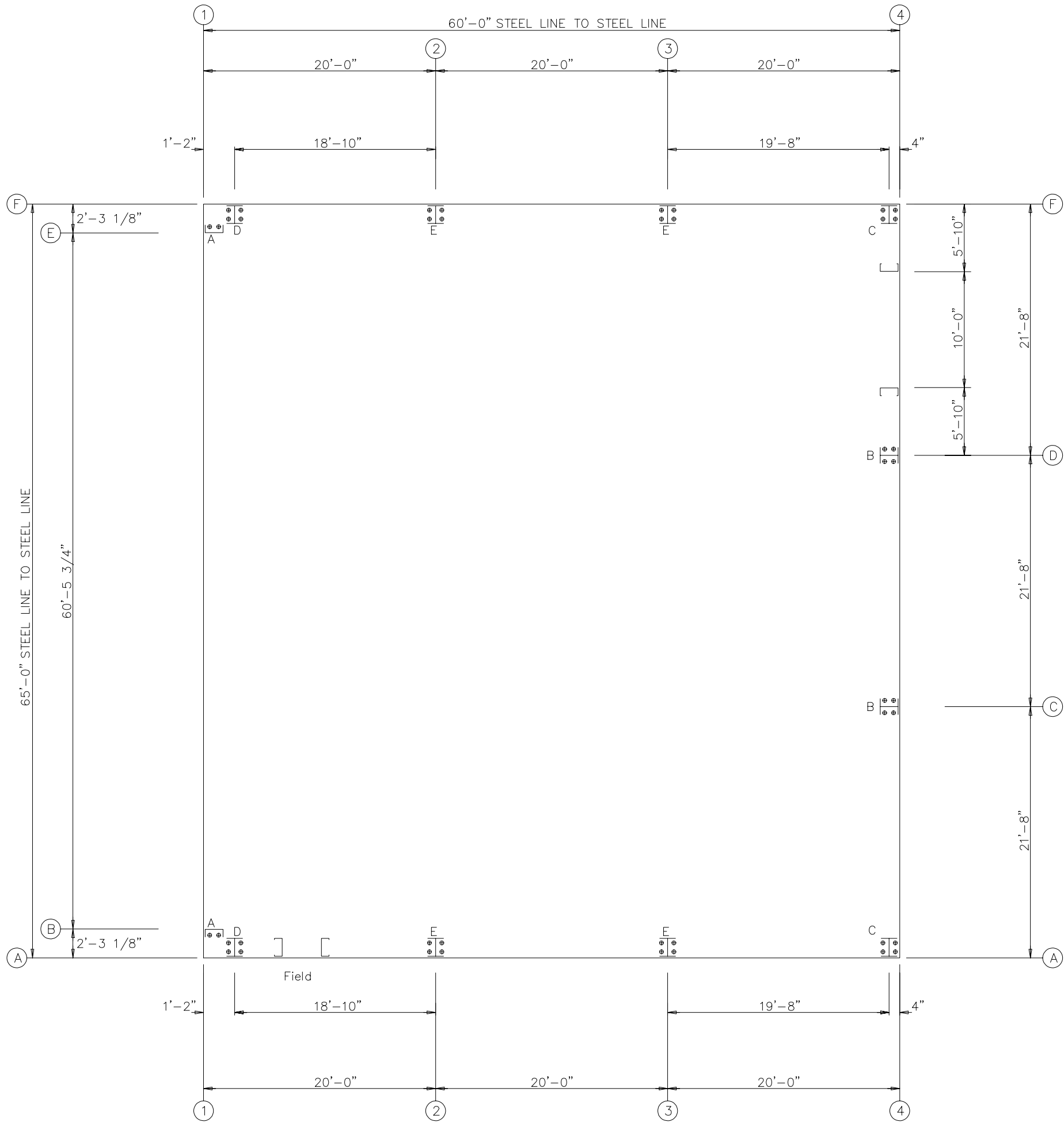
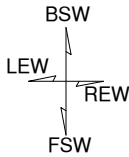
WISCONSIN

TYLER B. THEURET
E-46541
PITTSBURGH PA

PROFESSIONAL ENGINEER

9/11/23

PAGE 1 OF 17



ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)
Finished Floor @ 100'-0"

DRAWING IS NOT TO SCALE



MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

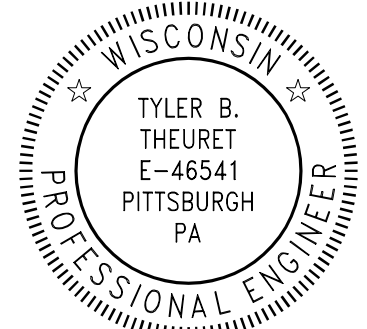
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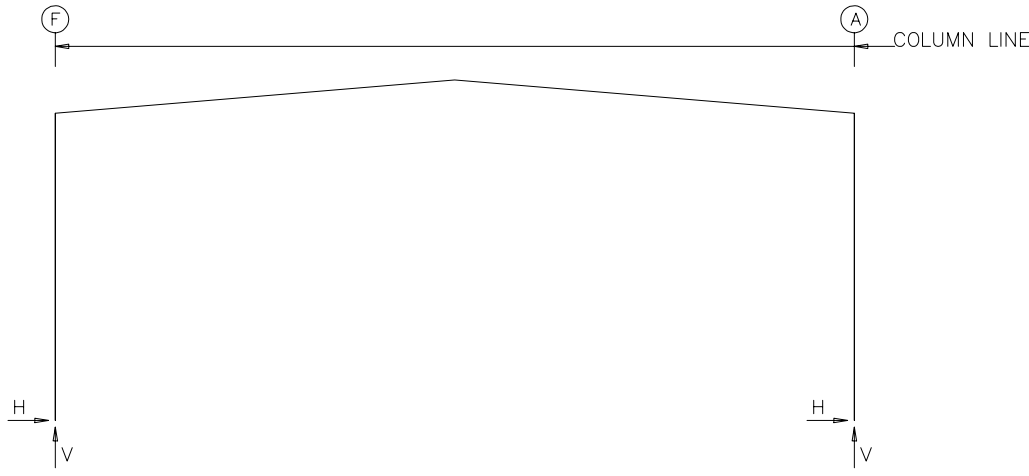
MIKE ARMBRUSTER

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9/11/23

FRAME LINES: 1 2 3



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Bolt Dia	Base_Plate (in) Width	Base_Plate (in) Length	Base_Plate (in) Thick	AFF/BFF (in)
1	F	4	0.750	8.000	11.75	0.500	0.0
1	A	4	0.750	8.000	11.75	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Bolt Dia	Base_Plate (in) Width	Base_Plate (in) Length	Base_Plate (in) Thick	AFF/BFF (in)
2*	F	4	0.750	8.000	11.56	0.500	0.0
2*	A	4	0.750	8.000	11.56	0.500	0.0

2* Frame lines: 2 3

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	-----Dead-----		-----Collateral-----		-----Live-----		-----Snow-----		-----Wind_Left1-----		-----Wind_Right1-----	
1	F	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	A	2.3	6.2	0.1	0.3	2.7	6.9	2.8	7.2	-4.8	-8.3	0.2	-4.7
		-2.3	6.2	-0.1	0.3	-2.7	6.9	-2.8	7.2	-0.2	-4.7	4.8	-8.3
Frame Line	Column Line	--Wind_Left2--		--Wind_Right2--		--Wind_Long1--		--Wind_Long2--		--Seismic_Left		Seismic_Right	
1	F	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	A	-4.7	-5.8	0.3	-2.2	-0.3	-5.4	-0.4	-4.4	-0.2	-0.2	0.2	0.2
		-0.3	-2.2	4.7	-5.8	0.4	-4.4	0.3	-5.4	-0.2	0.2	0.2	-0.2
Frame Line	Column Line	F1UNB_SL_L-		F1UNB_SL_R-									
1	F	Horiz	Vert	Horiz	Vert								
1	A	2.4	7.2	2.4	4.1								
		-2.4	4.1	-2.4	7.2								
Frame Line	Column Line	-----Dead-----		-----Collateral-----		-----Live-----		-----Snow-----		-----Wind_Left1-----		-----Wind_Right1-----	
2*	F	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	0.9	2.7	0.3	0.7	5.3	13.0	5.5	13.6	-7.6	-12.7	0.9	-7.2
		-0.9	2.7	-0.3	0.7	-5.3	13.0	-5.5	13.6	-0.9	-7.2	7.6	-12.7
Frame Line	Column Line	--Wind_Left2--		--Wind_Right2--		--Wind_Long1--		--Wind_Long2--		--Seismic_Left		Seismic_Right	
2*	F	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	-7.7	-8.0	0.8	-2.5	-0.6	-17.9	-1.0	-16.1	-0.2	-0.1	0.2	0.1
		-0.8	-2.5	7.7	-8.0	1.0	-16.1	0.6	-17.9	-0.2	0.1	0.2	-0.1
Frame Line	Column Line	--Seismic_Long		F2UNB_SL_L-		F2UNB_SL_R-							
2*	F	Horiz	Vert	Horiz	Vert	Horiz	Vert						
2*	A	0.0	-0.7	4.6	13.6	4.6	7.8						
		0.0	-0.7	-4.6	7.8	-4.6	13.6						
2*	Frame lines:		2	3									



MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/ 1/23 REVISION: 0

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F.O. 27630

MIKE ARMBRUSTER

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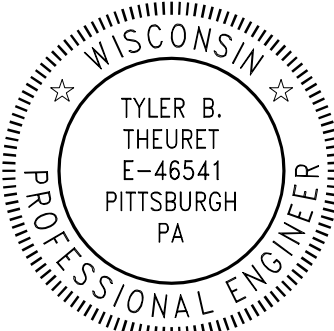
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DESCRIPTION

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ENDWALL COLUMN:				BASIC COLUMN REACTIONS (k)														
Frm Line	Col Line	---Dead---		Wind Press	Wind Suct	Seis Long												
1	E	Horz	Vert	Horz	Horz	Vert												
1	E	-0.7	1.0	-5.9	6.5	0.0												
1	B	-0.7	1.0	-5.9	6.5	0.0												
														Wind Press				
Frm Line	Col Line	Dead	Collat	Live	Snow	Wind_Left1		Wind_Right1		Wind_Left2		Wind_Right2		Horz				
4	A	0.6	0.1	1.8	1.9	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz				
4	A	0.6	0.1	1.8	1.9	0.0	-2.1	0.0	-1.4	0.0	-1.4	0.0	-0.6	-2.1				
4	C	1.1	0.2	4.8	5.0	-2.3	-8.3	0.0	-0.6	-2.3	-6.7	0.0	1.0	-4.7				
4	D	1.1	0.2	4.8	5.0	0.0	-0.6	2.3	-8.3	0.0	1.0	2.3	-6.7	-4.7				
4	F	0.6	0.1	1.8	1.9	0.0	-1.4	0.0	-2.1	0.0	-0.6	0.0	-1.4	-2.1				
Frm Line	Col Line	Wind Suct	Wind_Long1		Wind_Long2		Seis_Left		Seis_Right		Seis Long	E2UNB_SL_L-						
4	A	Horz	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Vert	Horz	Vert					
4	A	2.4	0.0	-2.4	0.0	-1.4	0.0	0.0	0.0	0.0	0.0	0.0	1.9					
4	C	5.2	0.0	-5.1	-0.3	-3.5	-0.2	-0.3	0.0	0.3	0.0	0.0	6.5					
4	D	5.2	0.3	-3.5	0.0	-5.1	0.0	0.3	0.2	-0.3	0.0	0.0	1.9					
4	F	2.4	0.0	-1.4	0.0	-2.4	0.0	0.0	0.0	0.0	0.0	0.0	0.5					
Frm Line	Col Line	E2UNB_SL_R-																
4	A	Horz	Vert															
4	A	0.0	0.5															
4	C	0.0	1.9															
4	D	0.0	6.5															
4	F	0.0	1.9															

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
⊕ 20	Endwall	3/4"	
⊕ 24	Frame	3/4"	

BUILDING BRACING REACTIONS

Reactions in plane of wall										
—Wall—		Col	± Reactions(k)				Panel Shear (lb/ft)			
Loc	Line	Line	Horz	Vert	Horz	Vert	Wind	Seis	Note	
L_EW	1								(h)	
F_SW	A	2,3	6.9	*	0.6	*				
R_EW	4	C,D	Bracing, see EW reactions							
B_SW	F	3,2	6.9	*	0.6	*				

(h)Rigid frame at endwall

*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

ANCHOR BOLTS & BASE PLATES									
Frm Line	Col Line	Anc._Qty	Bolt Dia	Base_Plate Width	(in) Length	Thick	AFF/BFF (in)		
1	E	2	0.750	3.250	5.750	0.125	0.0		
1	B	2	0.750	3.250	5.750	0.125	0.0		
4	A	4	0.750	6.000	9.875	0.375	0.0		
4	C	4	0.750	6.000	9.875	0.375	0.0		
4	D	4	0.750	6.000	9.875	0.375	0.0		
4	F	4	0.750	6.000	9.875	0.375	0.0		

DESIGN INFORMATION

- All loading conditions are examined and only the maximum / minimum H or V and the corresponding H or V are reported.
- Positive reactions are shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:

DESIGN CRITERIA				SEISMIC CRITERIA				DEFLECTION LIMITS			
Width (ft)				= 65				ENDWALL COLUMN			
Length (ft)				= 60				L / 120			
Eave Height (ft)				= 25				ENDWALL RAFTER (Live)			
Roof Slope (rise/12)				= 1.0:12				L / 180			
Building Code				= IBC 15				ENDWALL RAFTER (Wind)			
Local Code (State/Prov)				= WBC 18				L / 180			
Dead Load (psf)				= 2.250				WALL GIRTS			
Collateral Load (psf)				= 1.00				L / 90			
Roof Live Load (psf)				= 20.00				PURLIN (LIVE)			
Frame Live Load (psf)				= 20.00				L / 180			
				---				PURLIN (WIND)			
Snow:				---				L / 180			
Ground Snow Load (psf)				= 30.00				WALL PANEL			
Snow Importance				= 1.00				L / 90			
Thermal Coefficient				= 1.00				ROOF PANEL (Live)			
Snow Exposure Factor				= 1.0000				L / 180			
Slippery Roof				= N				ROOF PANEL (Wind)			
Roof Snow Load, Pf (psf)				= 21				L / 120			
Min Roof Snow Load, Pf (psf)				= 21.0000				H / 60			
								Main Frame (Horiz)			
Wind:								L / 180			
Ultimate Wind Speed (mph)				= 115 mph				H / 60			
Risk Category				= II - Normal				Main Frame (Vert)			
Importance - Wind				= 1.00				L / 180			
Wind Exposure				= B				WIND BRACING			
Enclosure Classification = C - Enclosed								H / 60			
								Main Frame (Crane)			
								H / 100			
								Main Frame (Seismic)			
								H / 50			
								SEISMIC BRACING			
								H / 50			
								PARTITION COLUMN			
								L / 120			
								PARTITION GIRT			
								L / 120			
								PARTITION PANEL			
								L / 120			

Equivalent Lateral Brace Force Procedure.

Steel systems not specifically detailed for seismic resistance.



MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

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DWN: BJC

APPD: TBT

MIKE ARMBRUSTER

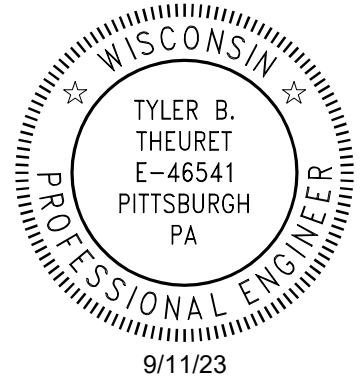
DRAWING STATUS

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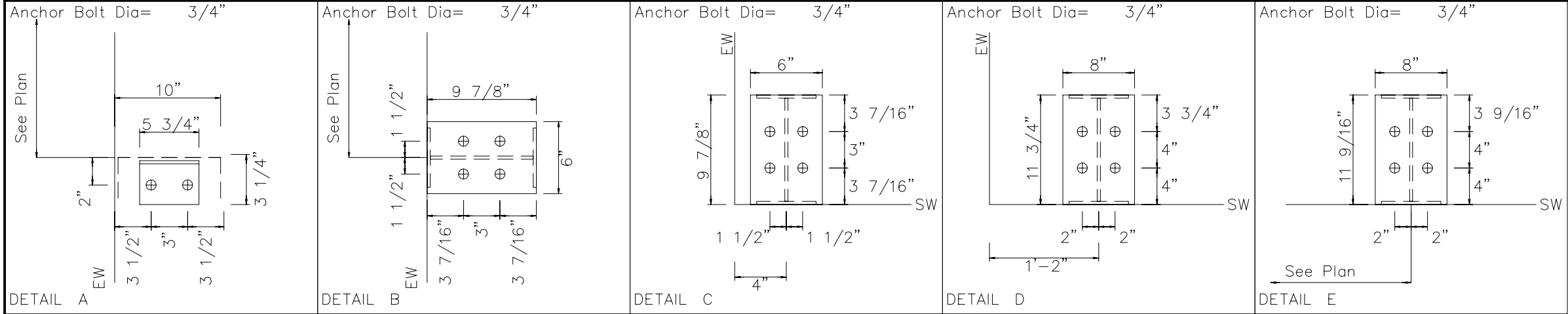
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FINAL DRAWINGS.



9/11/23



Olympia Steel Buildings
4000 Laurel Road • Mukwonago, WI 53150
888-449-7756

MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/1/23 REVISION: 0

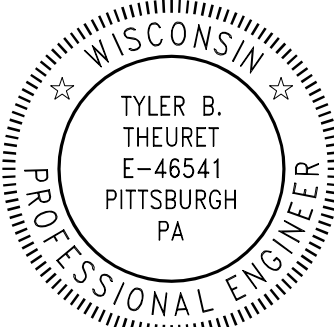
ENG: TBT DWN: BJC APPD: TBT

F.O. 27630

MIKE ARMBRUSTER

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CITY: WAUKESHA		ST / PV: WI
REVISION HISTORY		
REV.	DESCRIPTION	DATE



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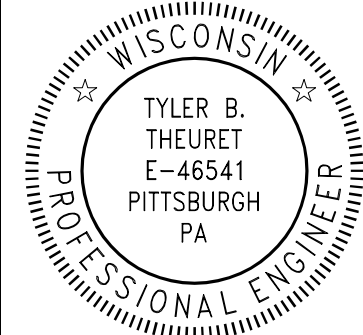
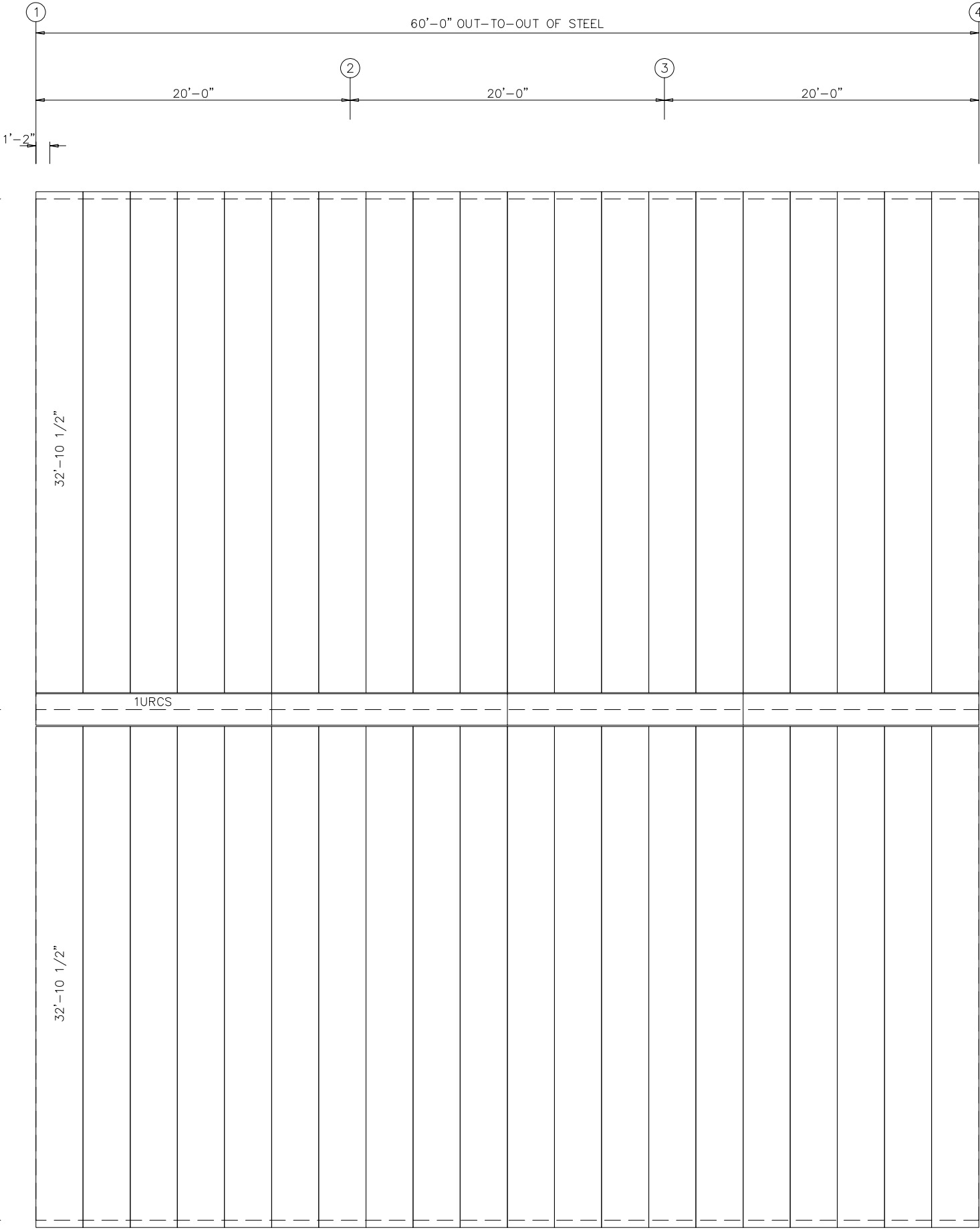
GENERAL NOTES:

Panel "Start" and "End" dimensions must be followed for the proper installation of the gable trim(s) provided.

ROOF SHEETING PLAN

PANELS: 26 Ga. R – Evergreen

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MIKE ARMBRUSTER

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CITY: WAUKESHA		ST / PV: WI
REVISION HISTORY		
REV.	DESCRIPTION	DATE

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Olympia Steel Buildings
400 Industrial Avenue • Waukesha, WI 53186
888-449-7756

MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

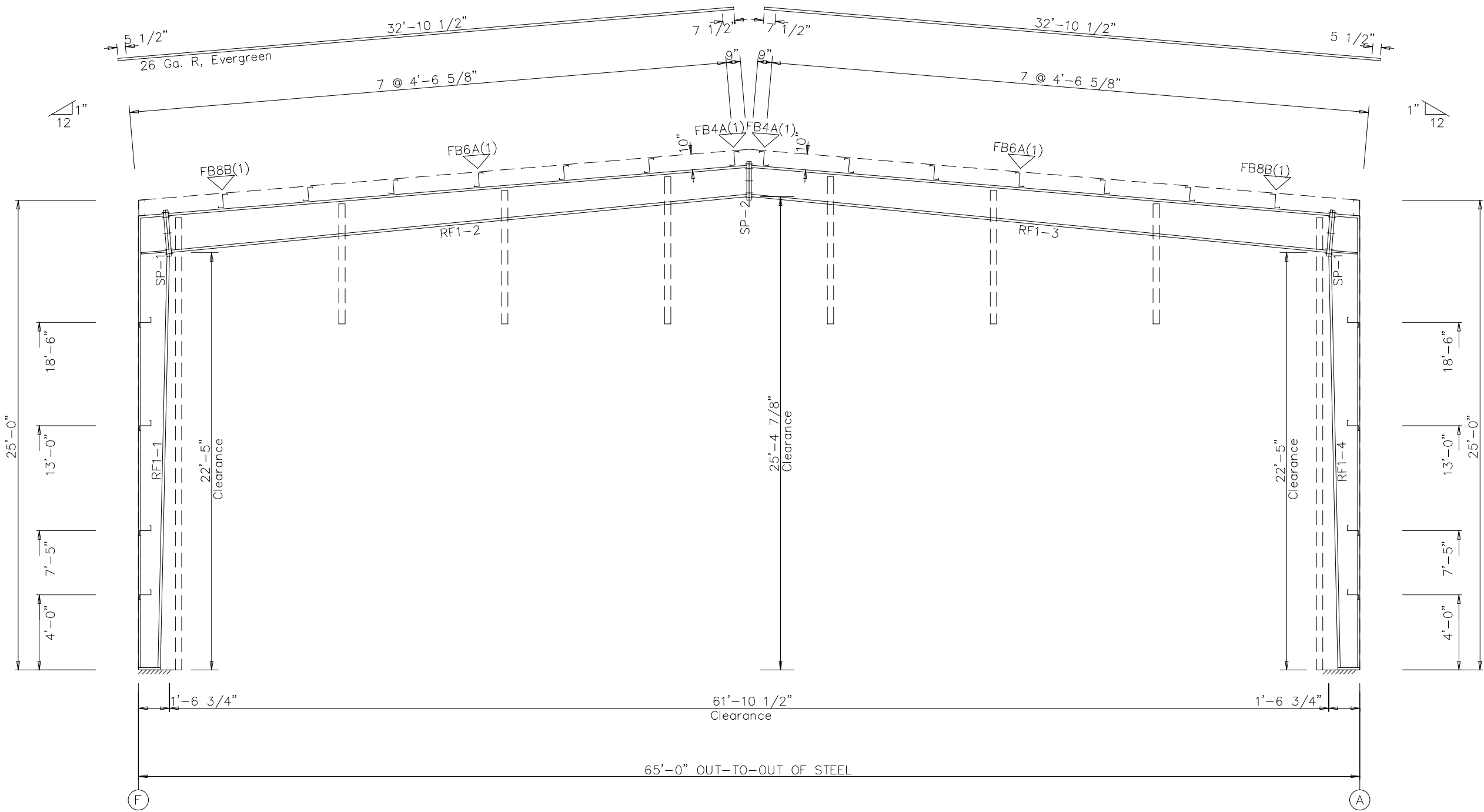
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ENG: TBT DWN: BJC APPD: TBT

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	2	A325	0.750	2.25
SP-2	4	4	2	A325	0.750	2.00

▽FLANGE BRACES: Both Sides(U.N.)
FBxxB(1)
B - L20X1/4
A - L15X1/8

MEMBER TABLE								
Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W	x Thk x Length	W	x Thk x Length
RF1-1	11.0	15.7	0.188	180.0	8	x 1/4" x 289.2	8	x 1/2" x 265.3
	15.7	18.0	0.219	110.6	6	x 1/4" x 16.2		
RF1-2	22.0	18.4	0.188	270.0	6	x 1/4" x 373.4	6	x 1/4" x 372.0
	18.4	17.0	0.188	103.4				
RF1-3	17.0	18.4	0.188	103.4	6	x 1/4" x 373.4	6	x 1/4" x 372.0
	18.4	22.0	0.188	270.0				
RF1-4	18.0	15.7	0.219	110.6	6	x 1/4" x 16.2	8	x 1/2" x 265.3
	15.7	11.0	0.188	180.0	8	x 1/4" x 289.2		



GENERAL NOTES:

- See Detail Sheets for Connection Information.
- See Shipping List for Flange Brace Lengths.

DRAWING IS NOT TO SCALE



MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/ 1/23

REVISION: 0

ENG: TBT

DWN: BJC

APPD: TBT

F.O. 27630

MIKE ARMBRUSTER

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FOR CONSTRUCTION:
FINAL DRAWINGS.

CITY: WAUKESHA

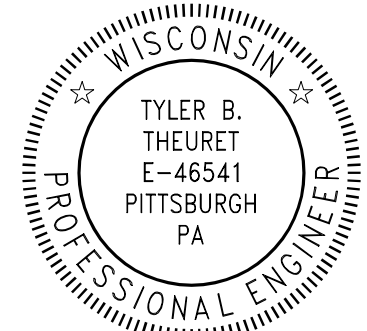
ST / PV: WI

REVISION HISTORY

DESCRIPTION

DATE

REV.

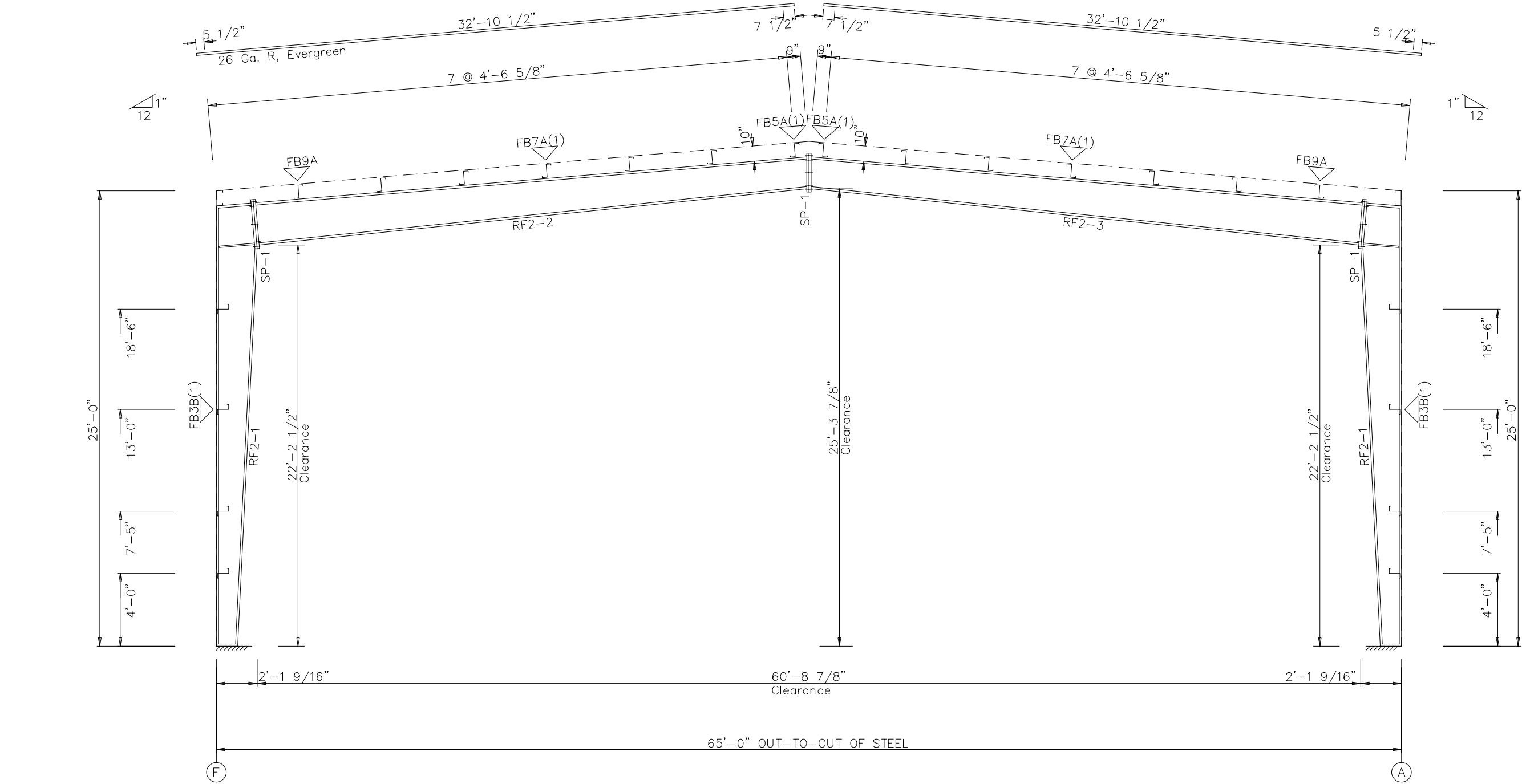


9/11/23

SPLICE BOLT TABLE						
Mark	Qty					
	Top	Bot	Int	Type	Dia	Length
SP-1	4	4	2	A325	0.750	2.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxB(1)
B - L20X1/4
A - L15X1/8

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	
	Start/End	Thick	Length		W x Thk x Length	Inside Flange W x Thk x Length
RF2-1	11.0/20.6	0.149	180.0		6 x 1/4" x 289.2	6 x 5/16" x 263.2
RF2-2	20.6/25.0	0.219	111.1		6 x 1/4" x 22.9	
	25.0/22.6	0.188	126.5		6 x 1/4" x 246.5	6 x 5/16" x 126.5
RF2-3	22.6/18.0	0.149	240.0		6 x 5/16" x 120.0	6 x 1/4" x 238.5
	18.0/22.6	0.149	240.0		6 x 5/16" x 120.0	6 x 1/4" x 238.5
	22.6/25.0	0.188	126.5		6 x 1/4" x 246.5	6 x 5/16" x 126.5



GENERAL NOTES:

- 1. See Detail Sheets for Connection Information.
- 2. See Shipping List for Flange Brace Lengths.

RIGID FRAME ELEVATION: FRAME LINE 2 3



MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/ 1/23

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ENG: TBT

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APPD: TBT

F.O. 27630

MIKE ARMBRUSTER

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CITY: WAUKESHA

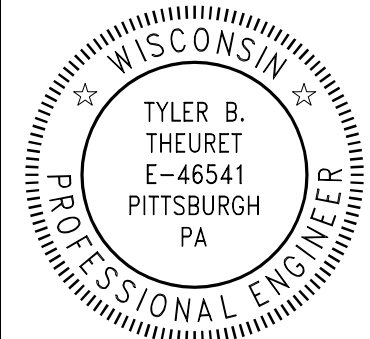
ST / PV: WI

REVISION HISTORY

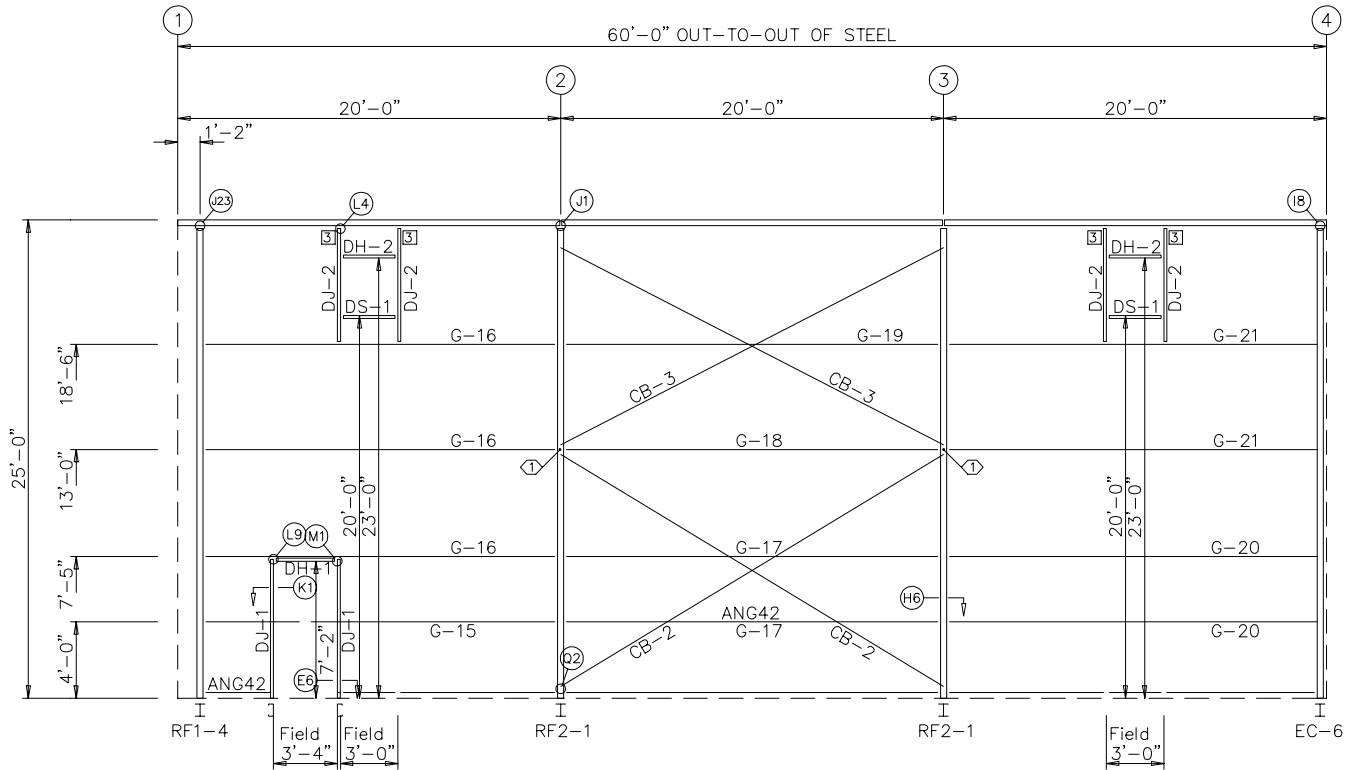
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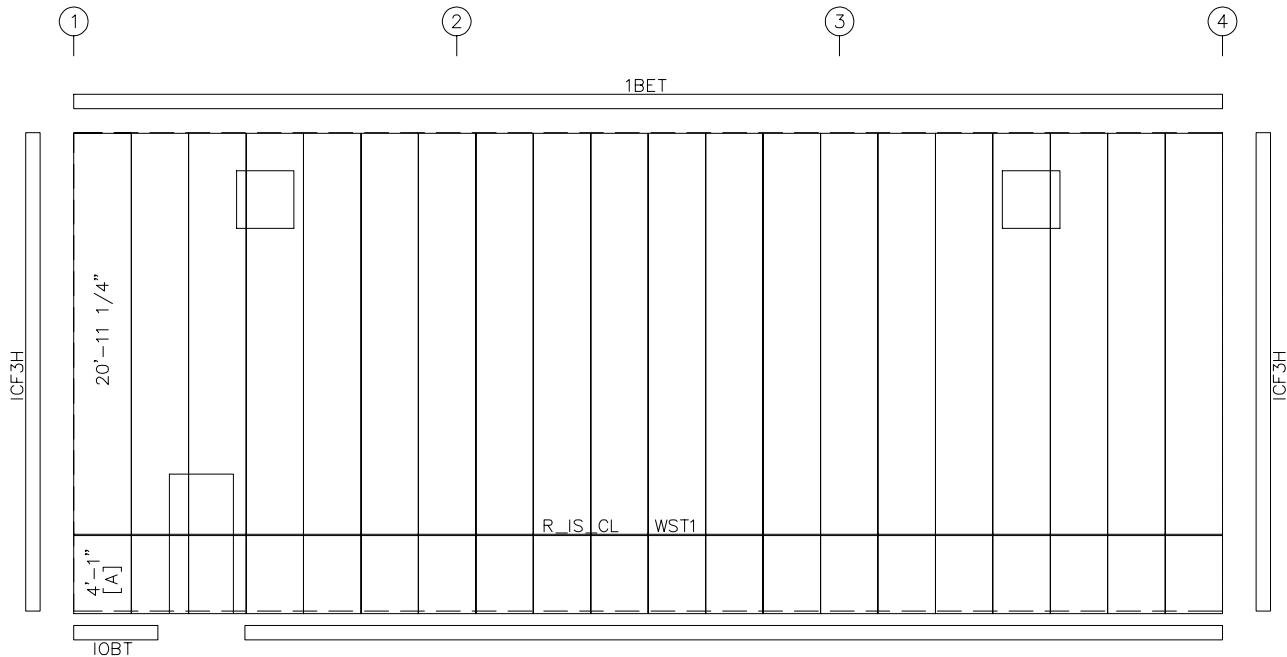
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SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. R - Light Stone
[A] PANELS: 26 Ga. R - Evergreen

MEMBER TABLE			
FRAME LINE A			
QUAN	MARK	PART	LENGTH
2	DJ-1	08X35C16	7'-4 3/4"
4	DJ-2	08X35C16	5'-6 3/4"
1	DH-1	08X30C16	3'-4"
2	DH-2	08X35C16	3'-0"
2	DS-1	08X35C16	3'-0"
1	G-15	08X25Z16	18'-2 1/2"
3	G-16	08X25Z14	18'-2 1/2"
2	G-17	08X25Z16	19'-5 1/2"
1	G-18	08X25Z12	19'-5 1/2"
1	G-19	08X25Z14	19'-5 1/2"
2	G-20	08X25Z16	19'-0 1/2"
2	G-21	08X25Z14	19'-0 1/2"
2	CB-2	CABLE375	21'-1 7/8"
2	CB-3	CABLE375	20'-2 7/8"

CONNECTION PLATES		
FRAME LINE A		
ID	QUAN	MARK / PART
3	4	JC

SPECIAL BOLTS					
FRAME LINE A					
○ ID	QUAN	TYPE	DIA	LENGTH	WASH
1	2	A325	1/2"	1 1/4"	0

DRAWING IS NOT TO SCALE

TRIM COLORS			
EAVE TRIM	= Evergreen	CORNER TRIM	= Evergreen
BASE TRIM	= Evergreen	GUTTER	= N/A
DOOR TRIM	= Evergreen	DOWNSPOUTS	= N/A
RAKE TRIM	= Evergreen		
* LINER TRIM	= Liner panel color		
* SOFFIT TRIM	= Soffit panel color		
* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.			



MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/ 1/23 REVISION: 0

ENG: TBT DWN: BJC APPD: TBT

MIKE ARMBRUSTER

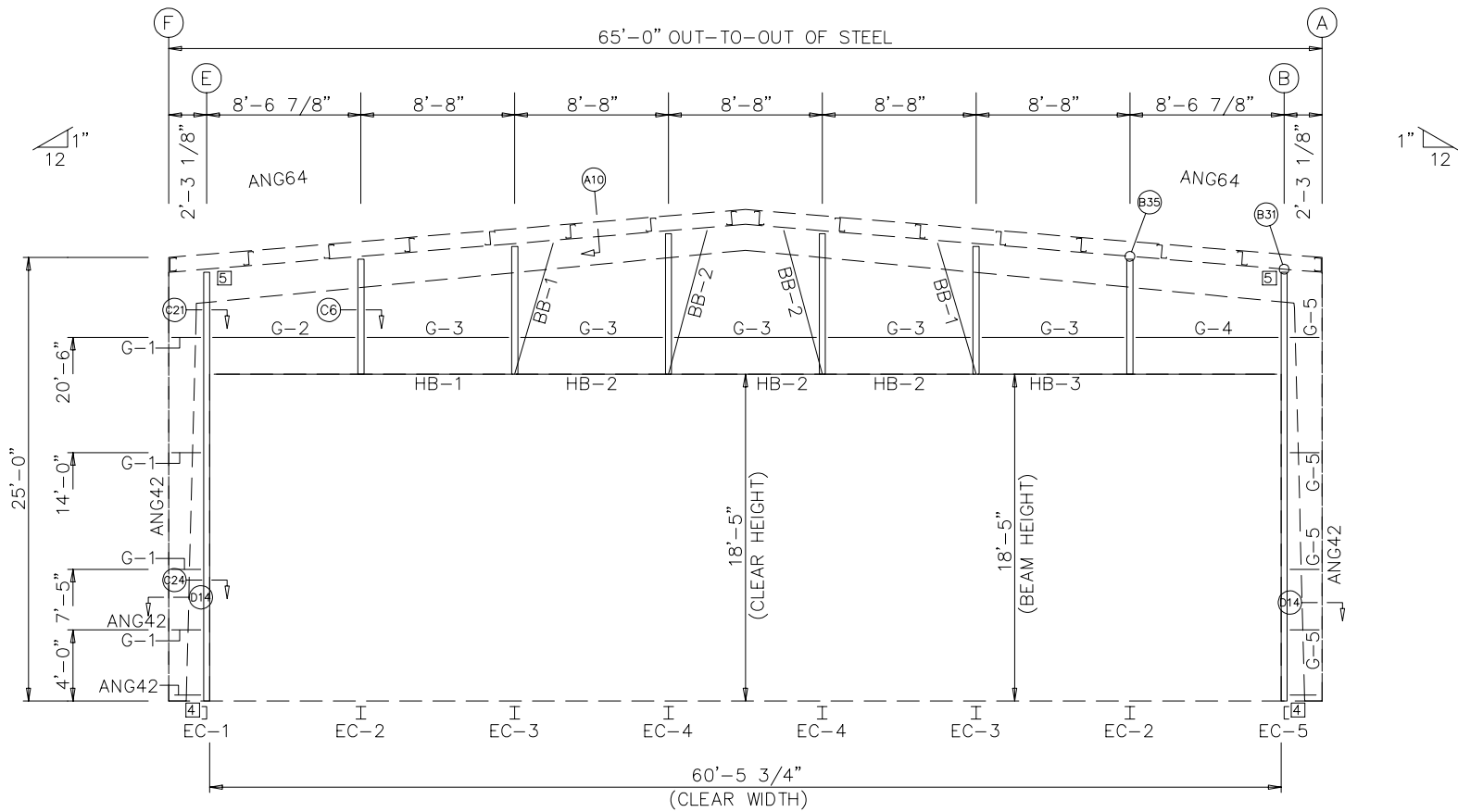
DRAWING STATUS		REVISION HISTORY	
ST / PV: WI	CITY: WAUKESHA	DESCRIPTION	DATE
REV.			
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☐		☒	
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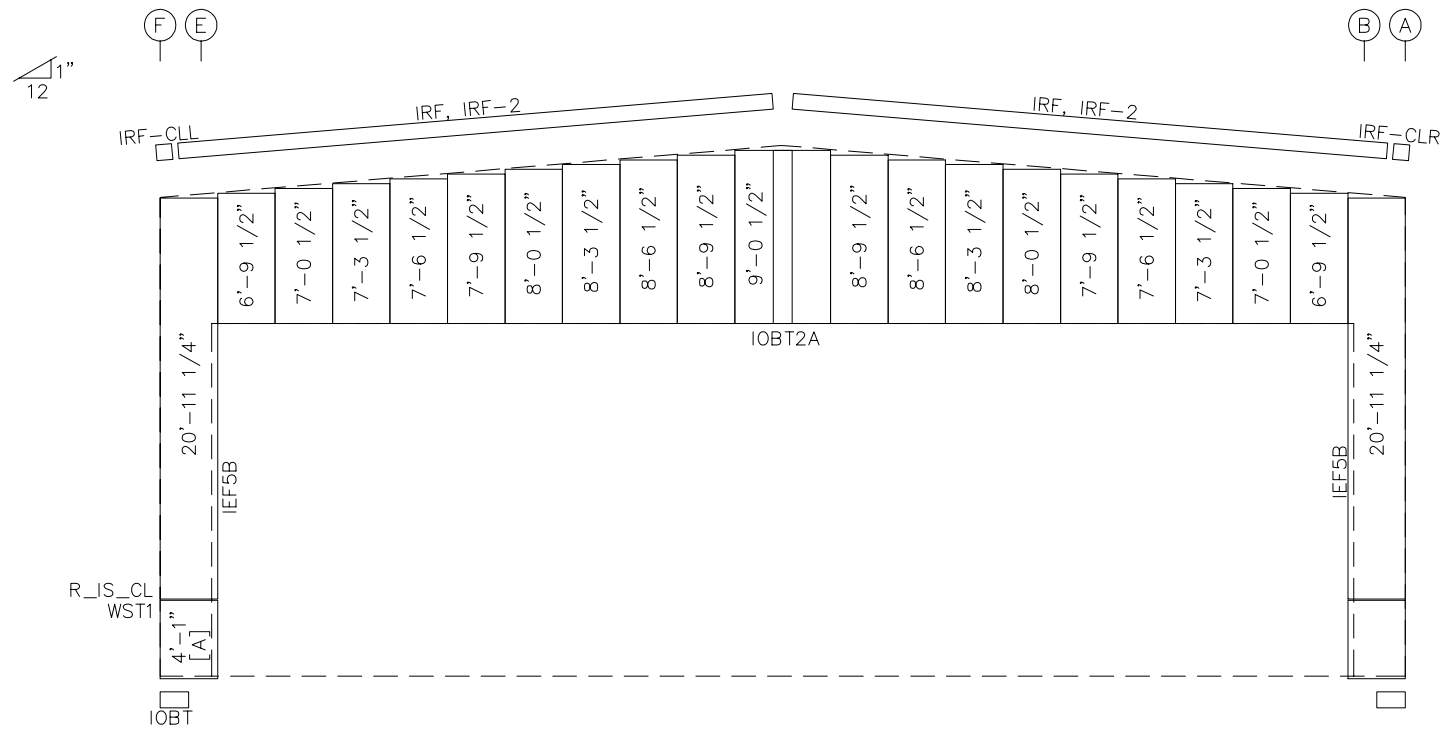
9/11/23

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

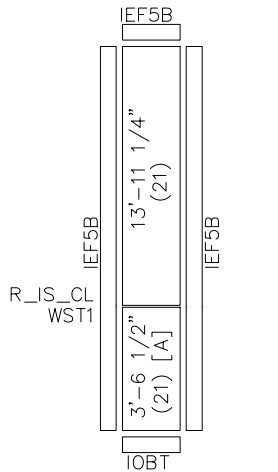


ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. R - Light Stone
[A] PANELS: 26 Ga. R - Evergreen



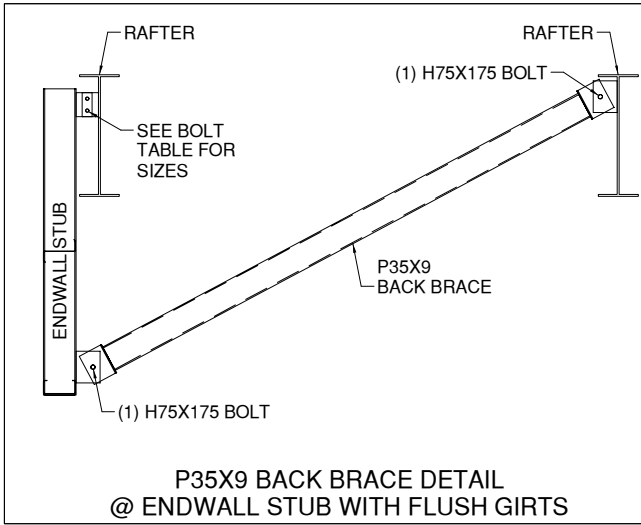
DOOR SHEETING

PANELS: 26 Ga. R
Light Stone
[A] PANELS: 26 Ga. R
Evergreen

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
W10X22/Raf	2	A325	3/4"	1 3/4"
EC-1,5/ECXF	2	A325	3/4"	2 1/4"
ECXF/Raf	2	A325	3/4"	2"
HB/EC-1,5	2	A325	3/4"	2"
HB/EC-3,4	2	A325	3/4"	1 3/4"
HB/EC-2	2	A325	1/2"	1 1/4"

MEMBER TABLE			
FRAME LINE 1			
QUAN	MARK	PART	LENGTH
1	HB-1	C10X20	16'-11 5/8"
3	HB-2	C10X20	8'-1 1/2"
1	HB-3	C10X20	16'-11 5/8"
2	BB-1	P35X9	20'-0 3/8"
2	BB-2	P35X9	20'-3 3/8"
1	EC-1	C10X25	23'-10 1/16"
2	EC-2	W10X22	5'-10 5/8"
2	EC-3	W10X22	6'-10 7/16"
2	EC-4	W10X22	7'-7 1/8"
1	EC-5	C10X25	23'-10 1/16"
4	G-1	08X25Z16	1'-11 5/8"
1	G-2	08X25Z16	8'-3 1/8"
5	G-3	08X25Z16	8'-1 1/4"
1	G-4	08X25Z16	8'-3 1/8"
4	G-5	08X25Z16	1'-11 5/8"

CONNECTION PLATES		
FRAME LINE 1		
ID	QUAN	MARK/PART
4	2	SP-CFBC
5	2	ECXF-ALT



P35X9 BACK BRACE DETAIL
@ ENDWALL STUB WITH FLUSH GIRTS

DRAWING IS NOT TO SCALE

TRIM COLORS			
EAVE TRIM	= Evergreen	CORNER TRIM	= Evergreen
BASE TRIM	= Evergreen	GUTTER	= N/A
DOOR TRIM	= Evergreen	DOWNSPOUTS	= N/A
RAKE TRIM	= Evergreen		
* LINER TRIM	= Liner panel color		
* SOFFIT TRIM	= Soffit panel color		
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MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/ 1/23 REVISION: 0

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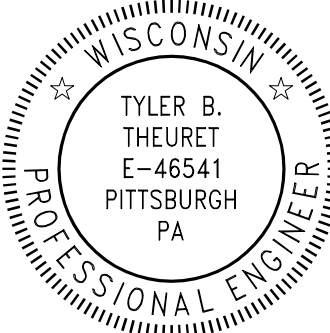
MIKE ARMBRUSTER

DRAWING STATUS

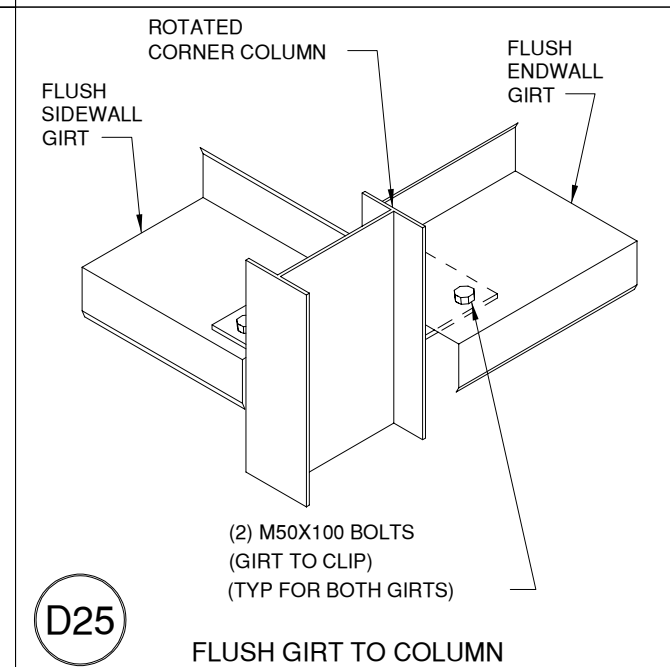
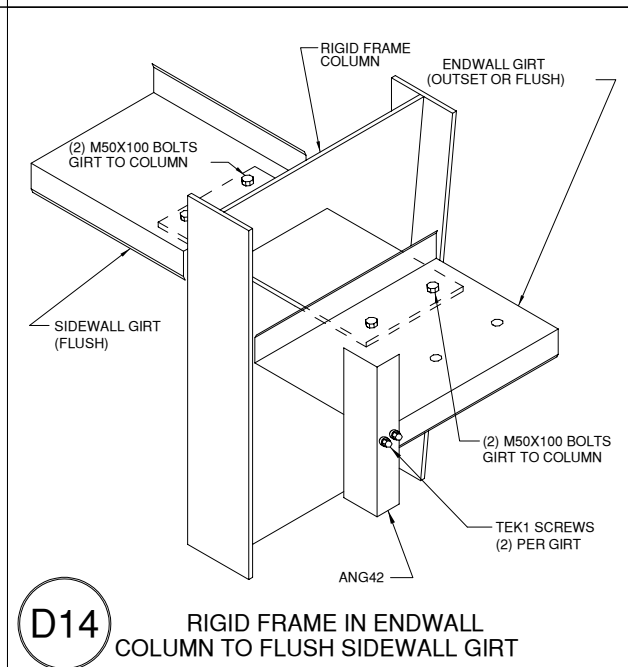
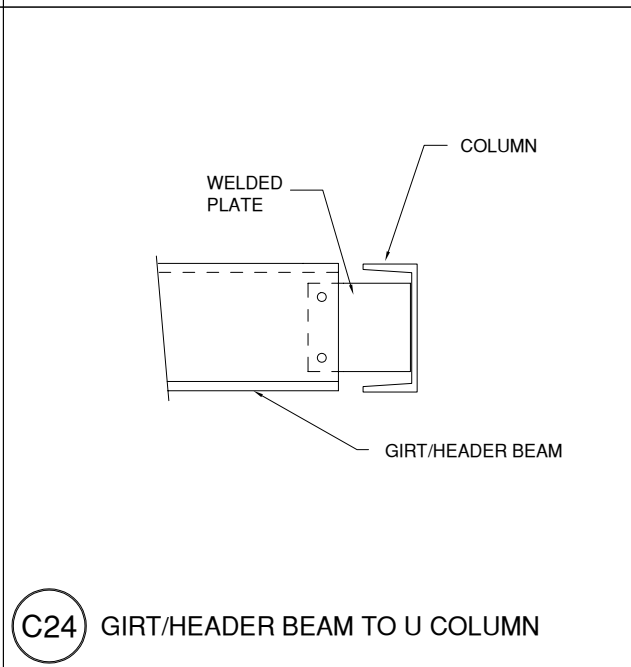
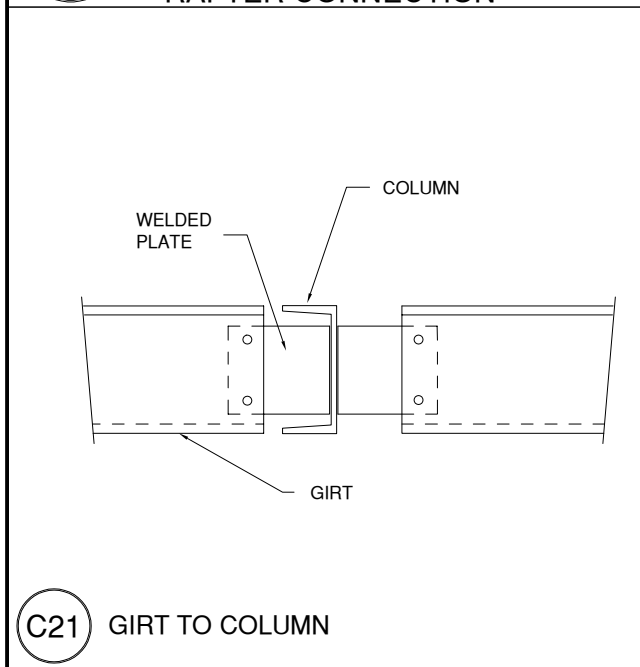
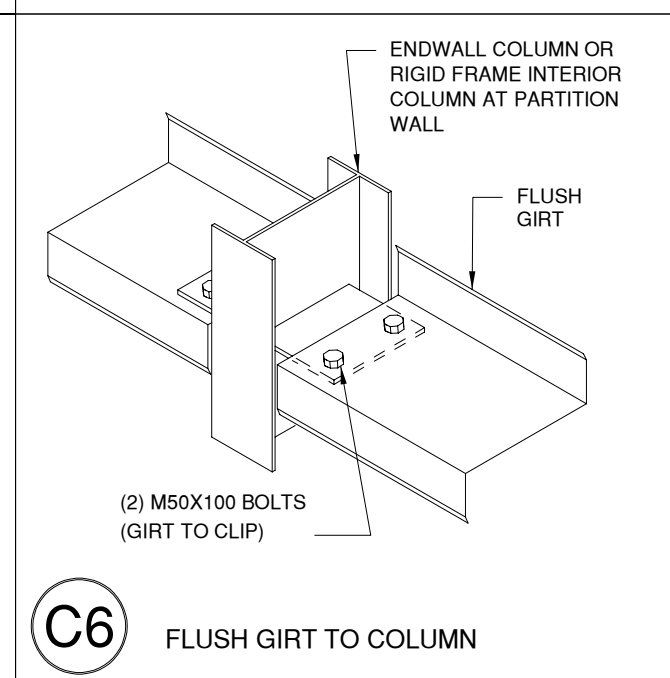
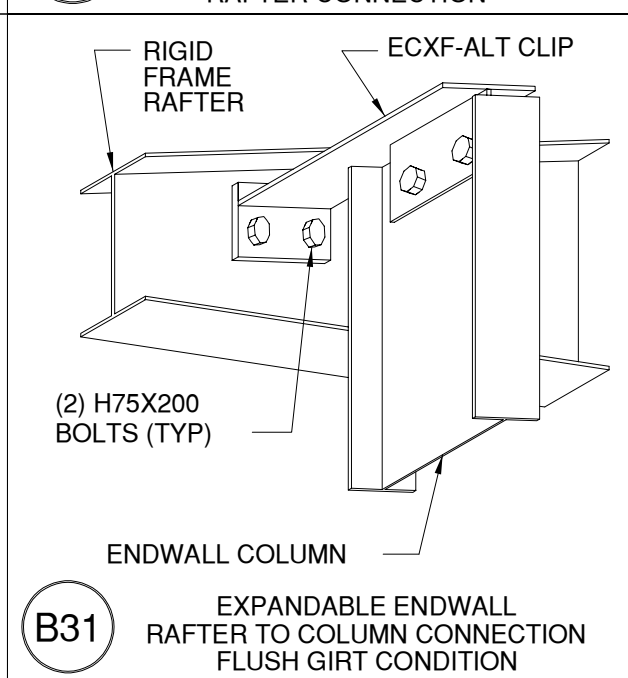
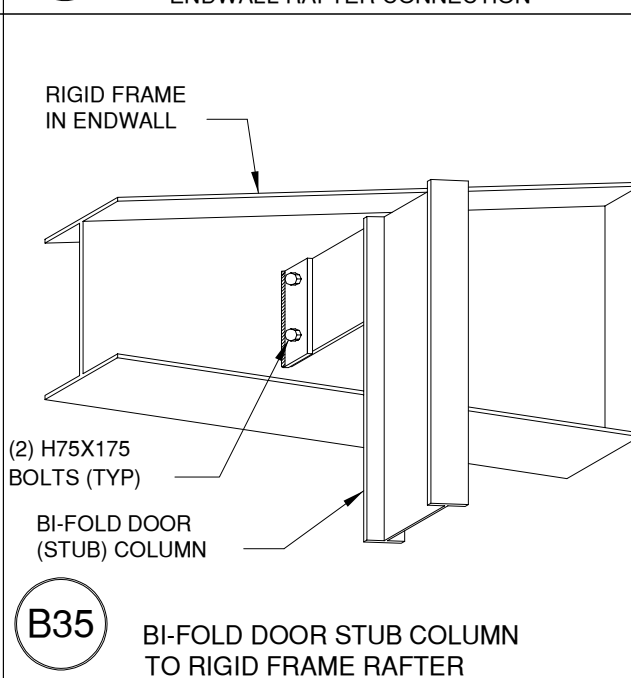
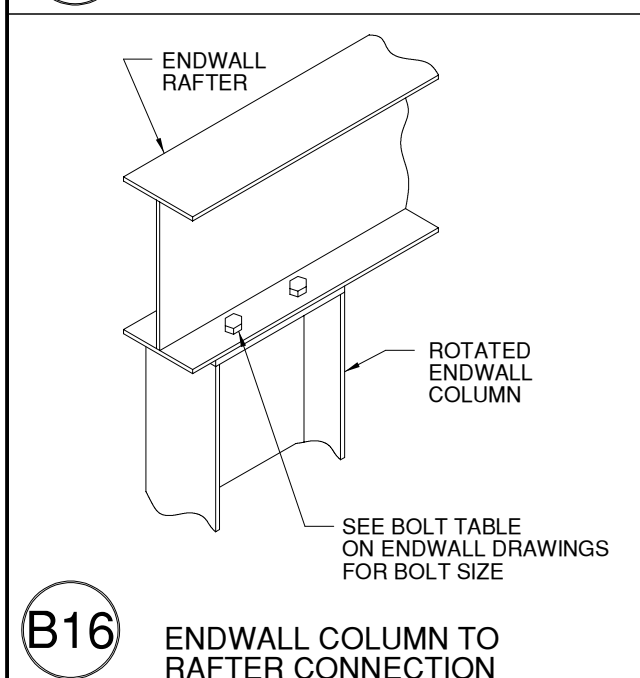
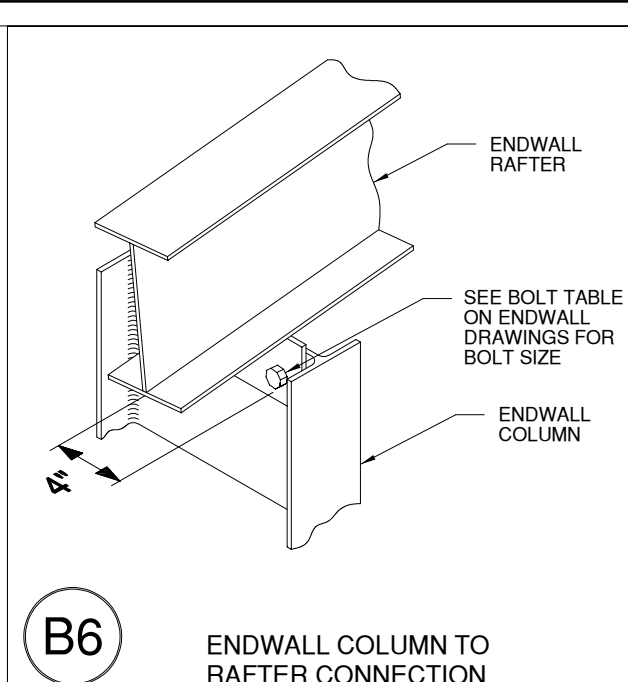
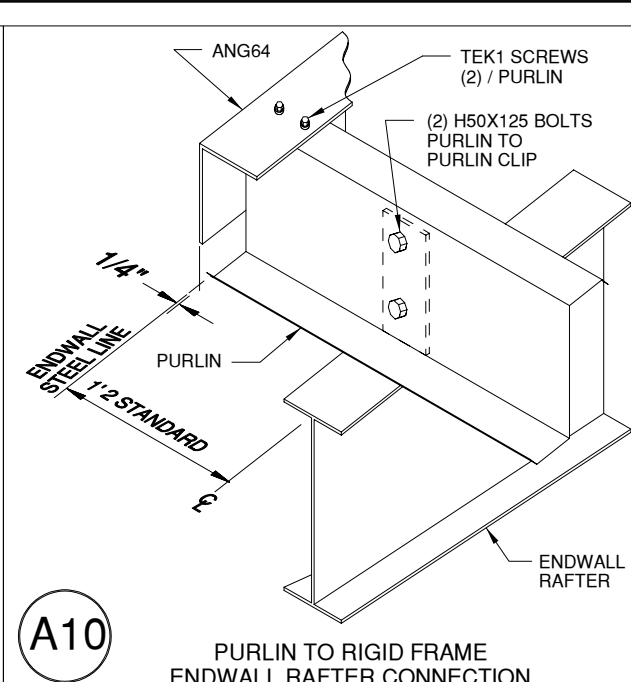
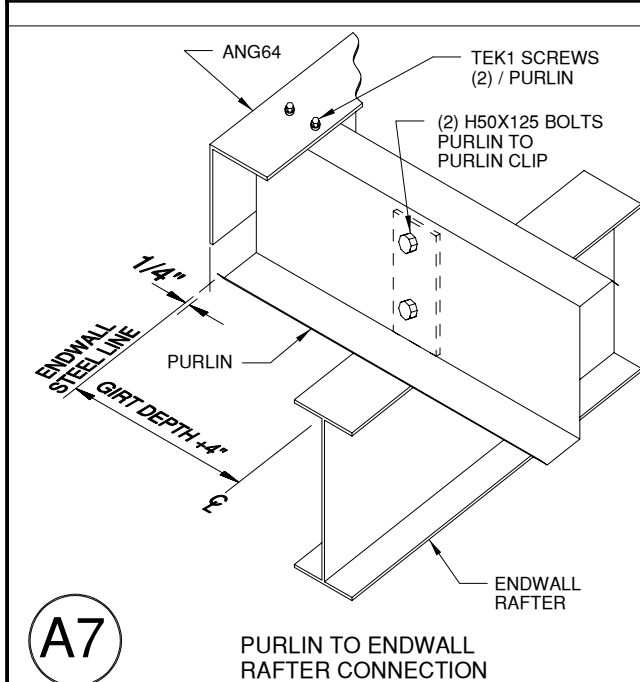
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MIKE ARMBRUSTER
65'-0" x 60'-0" x 25'-

DATE: 9/ 1/23	REVISION: 0
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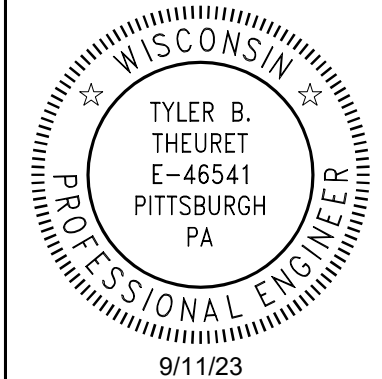
MIKE ARMBRUSTER

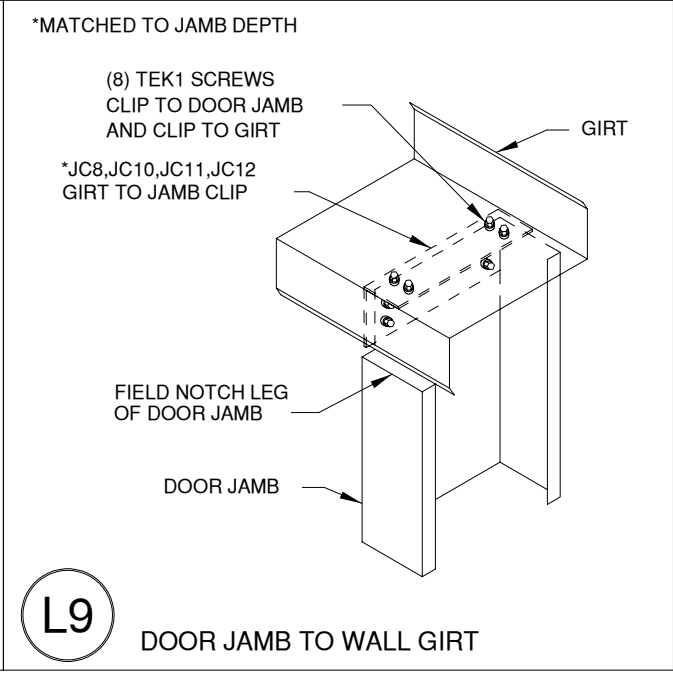
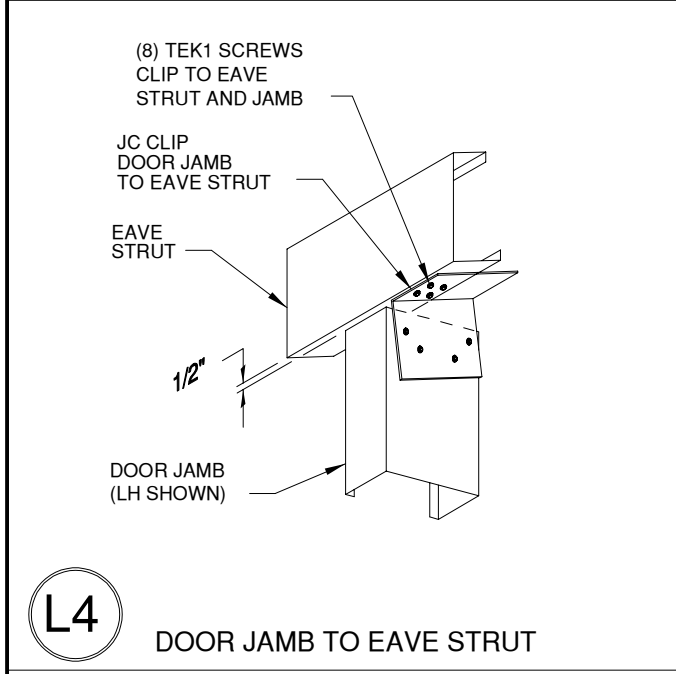
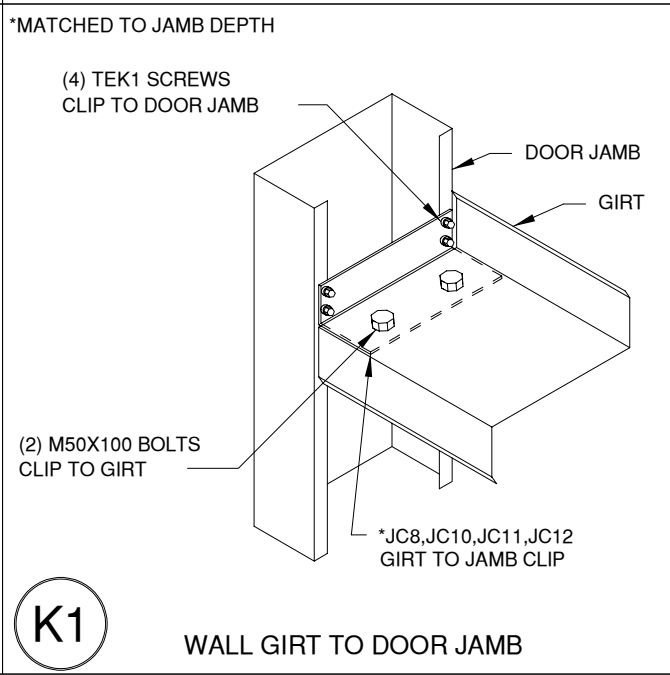
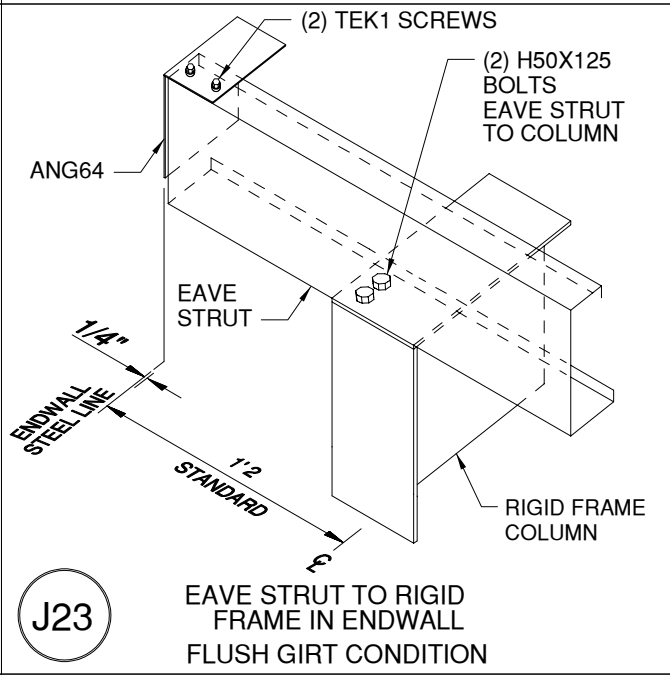
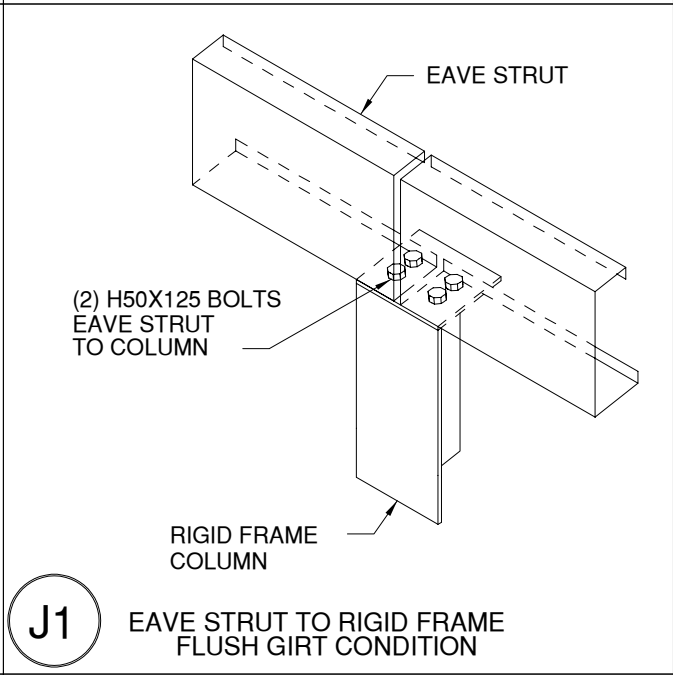
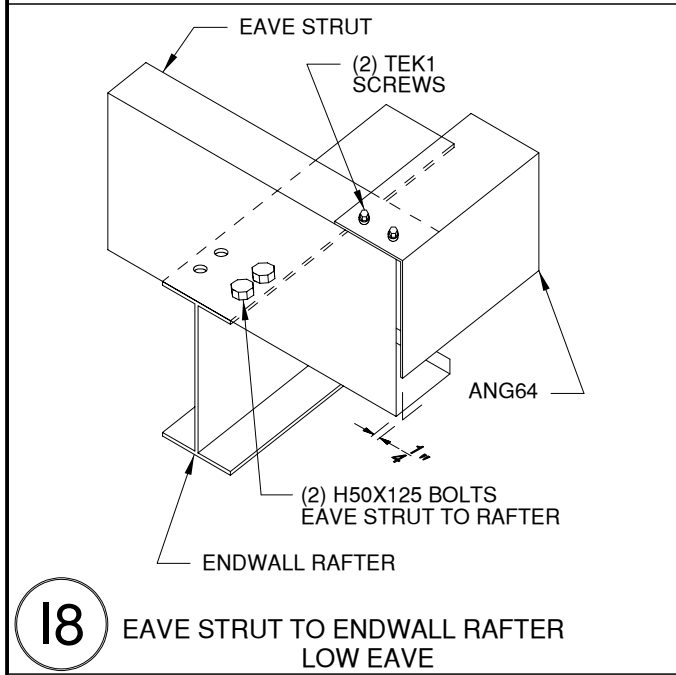
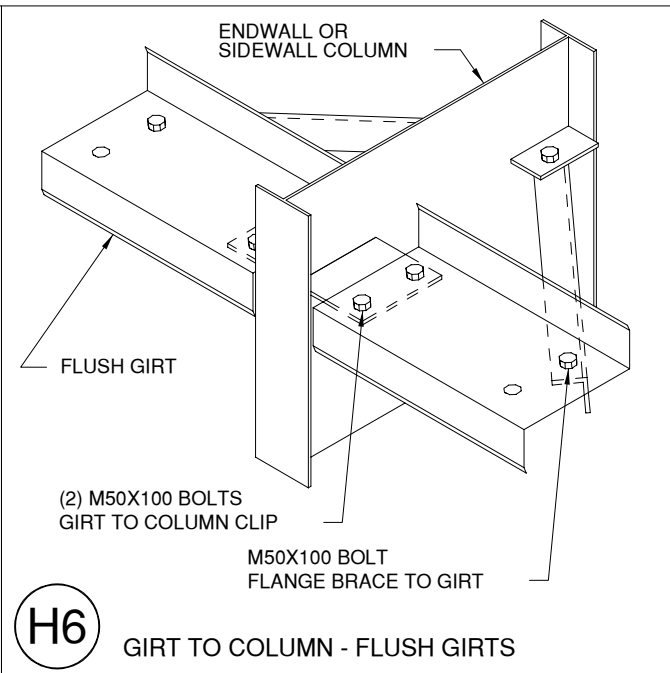
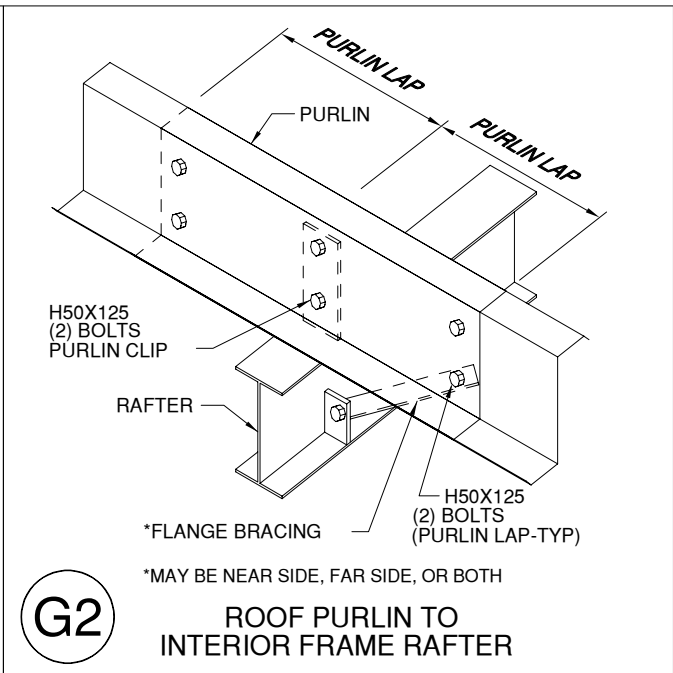
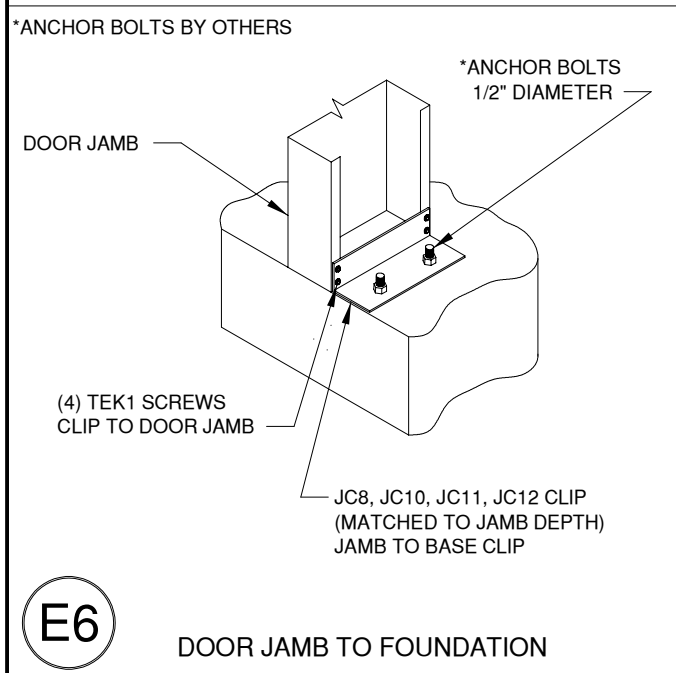
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**FOR CONSTRUCTION:
FINAL DRAWINGS**





Olympia Steel Buildings
4000 Valley Road • Mechanicsville, VA 23103
888-449-7756

MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/ 1/23 REVISION: 0

ENG: TBT DWN: BJC APPD: TBT

F.O. 27630

MIKE ARMBRUSTER

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FINAL DRAWINGS.

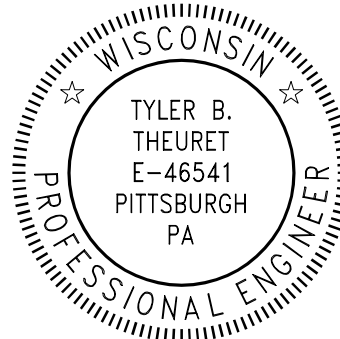
CITY: WAUKESHA ST / PV: WI

REVISION HISTORY

DATE

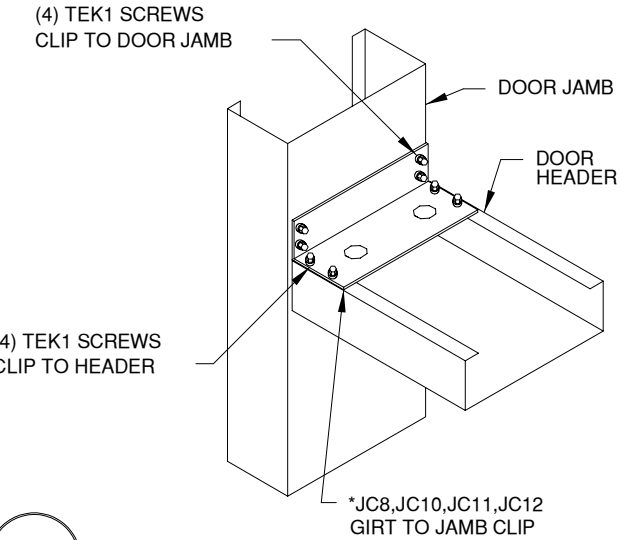
DESCRIPTION

REV.

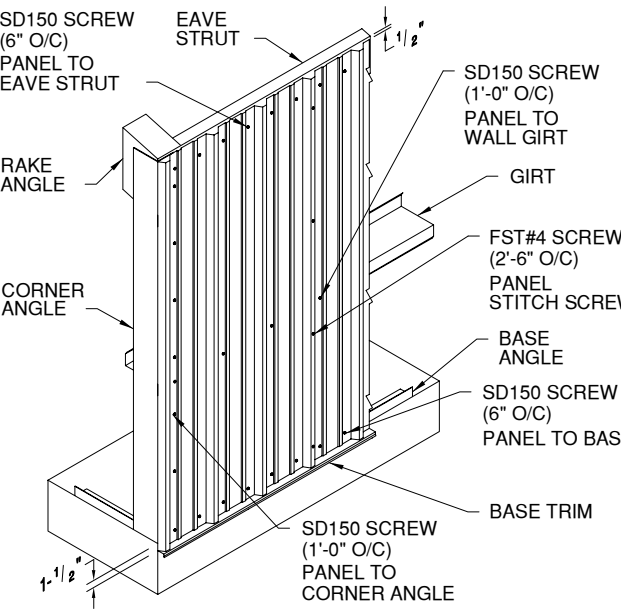


9/11/23

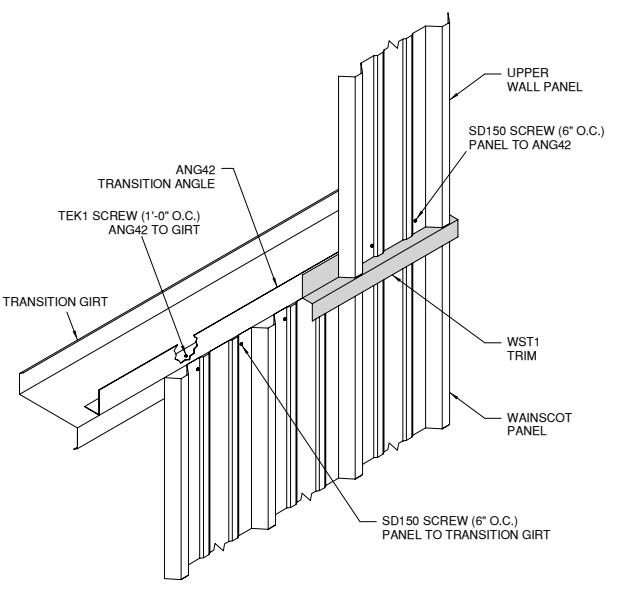
*MATCHED TO JAMB DEPTH



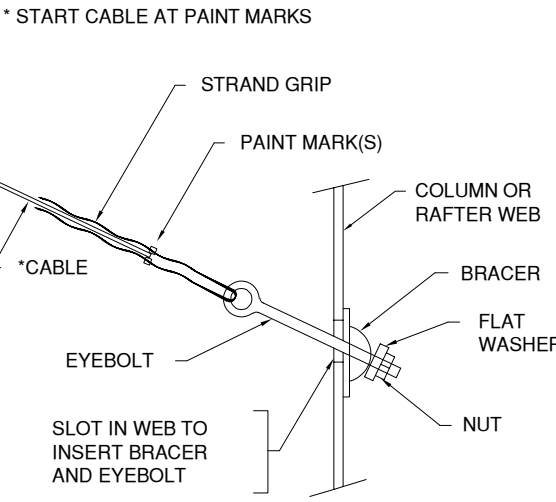
M1 DOOR HEADER TO DOOR JAMB



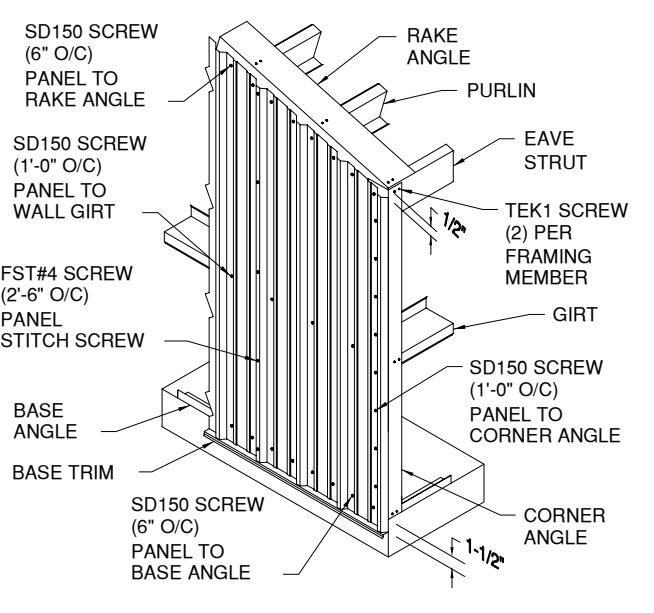
"R" PANEL SIDEWALL



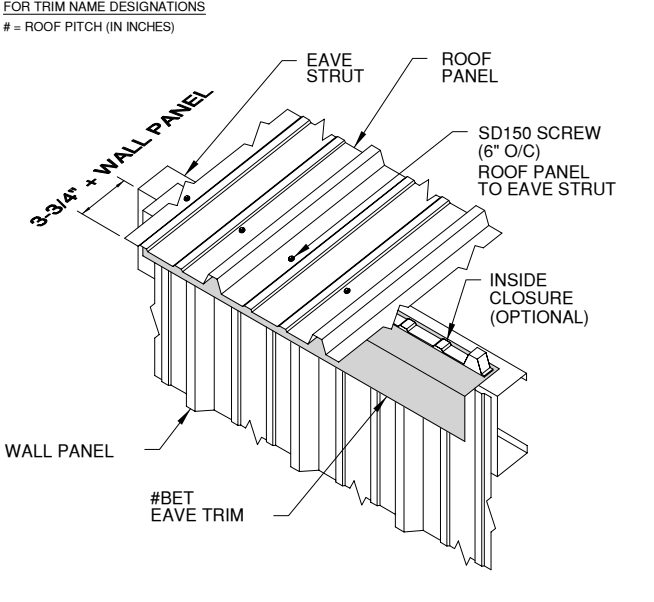
WAINSCOT WITH TRIM CONDITION



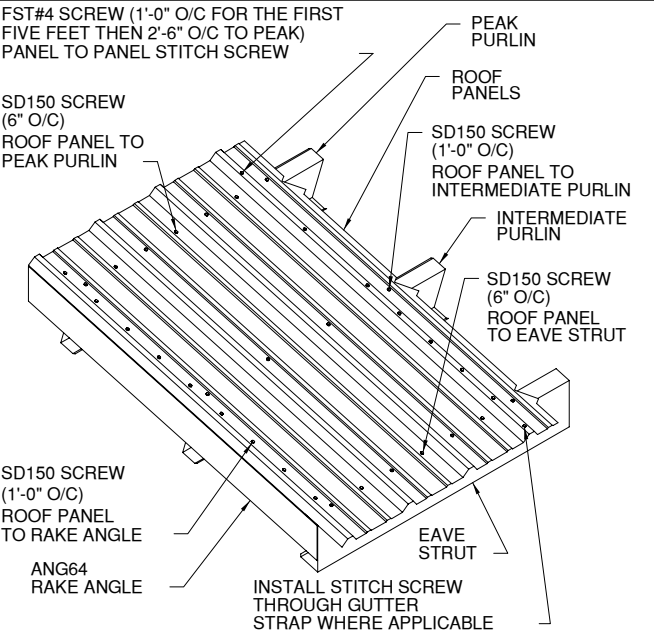
Q2 CABLE BRACE / EYEBOLT



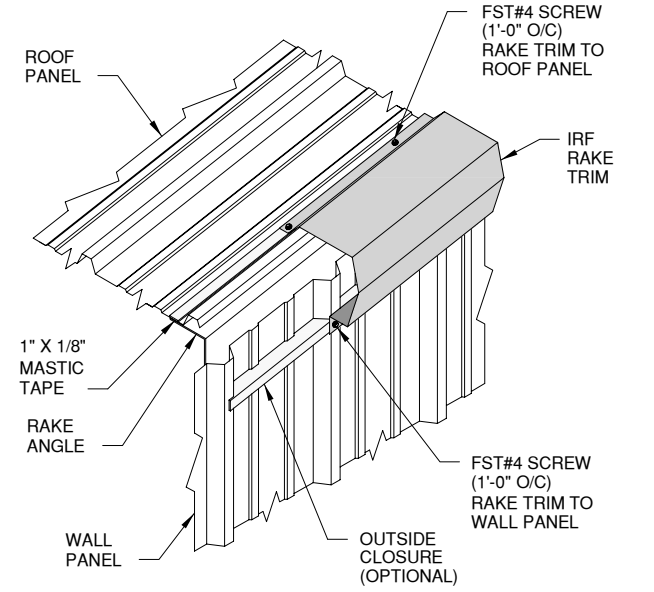
"R" PANEL ENDWALL



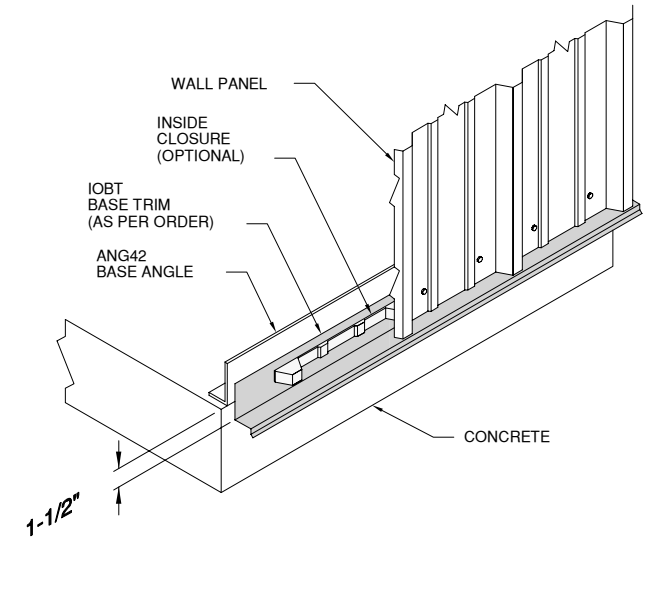
STANDARD EAVE CONDITION



ROOF FASTENER APPLICATION



STANDARD RAKE CONDITION



STANDARD BASE TRIM



MIKE ARMBRUSTER

65'-0" x 60'-0" x 25'-0"

DATE: 9/ 1/23 REVISION: 0

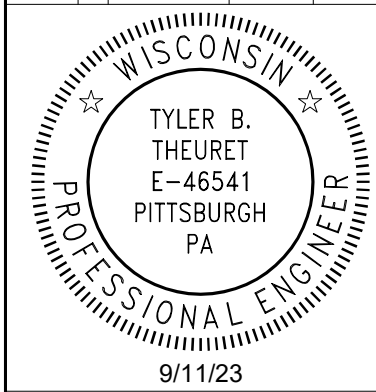
ENG: TBT DWN: BJC APPD: TBT

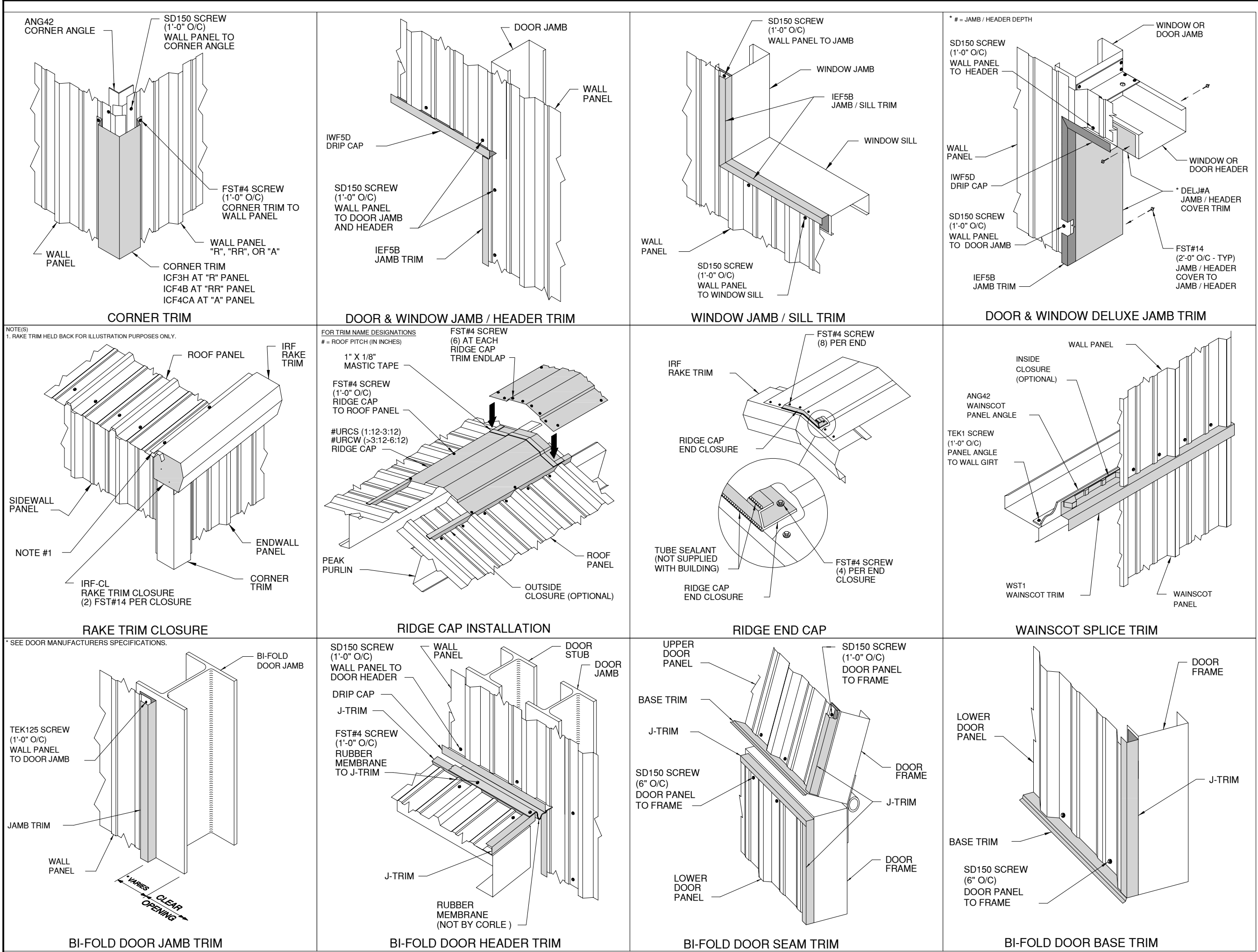
F.O. 27630

REVISION HISTORY		
REV.	DESCRIPTION	DATE

MIKE ARMBRUSTER

DRAWING STATUS	CITY: WAUKESHA ST / PV: WI
☐ FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.	☐ FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.
☐ FOR CONSTRUCTION: FINAL DRAWINGS.	





MIKE ARMBRUSTER
65'-0" x 60'-0" x 25'-0"
DATE: 9/1/23
ENG: TBT
DWN: BJC
APPD: TBT
REVISION: 0

F.O. 27630

CITY: WAUKESHA			ST / PV: WI	REVISION HISTORY		DATE
DRAWING STATUS			DESCRIPTION		REV.	
MIKE ARMBRUSTER			FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.			
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			FOR CONSTRUCTION: FINAL DRAWINGS.			

