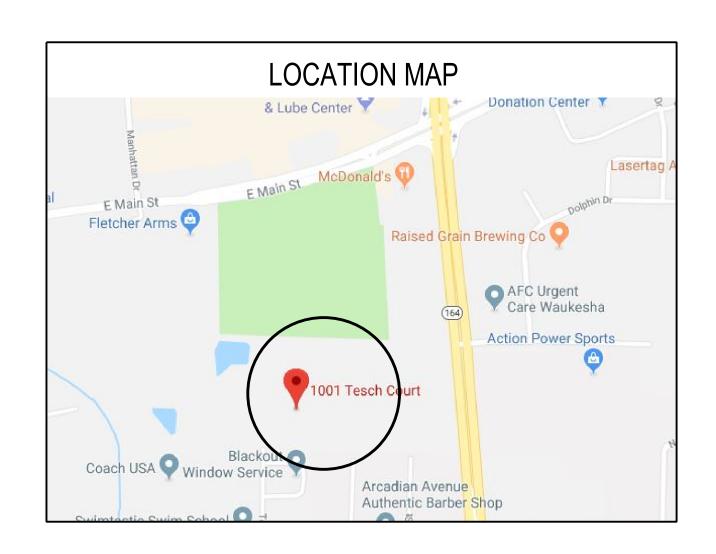
ALLIED RESOURCE ADDITION 12 GAUGE CONSTRUCTION WAUKESHA, WI

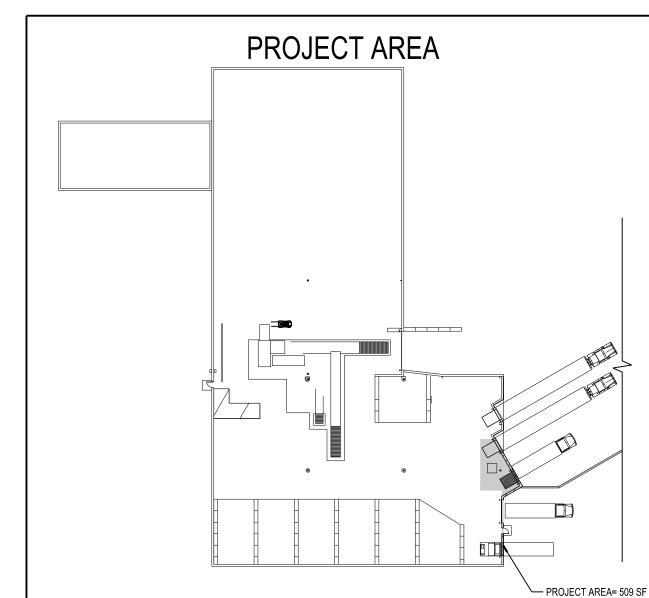
Angus Young

Architecture | Engineering Interiors | Landscape

——— Balance in Creativity ———

Janesville: 555 South River Street - Janesville, WI 53548 | Ph. 608.756.2326 Madison: 16 North Carroll Street - Madison, WI 53703 | Ph. 608.284.8225 www.angusyoung.com





SHEET INDEX GENERAL G001 COVER SHEET/ CODE INFORMATION ARCHITECTURAL AD101 PROPOSED DEMOLITION PLAN A101 PROPOSED FIRST FLOOR PLAN AND ROOF PLAN A401 EXTERIOR ELEVATIONS STRUCTURAL S101 FOUNDATION AND SLAB DEMO AND PROPOSED PLAN S102 ROOF FRAMING PLAN AND ELEVATIONS S301 GENERAL NOTES

REGULATORY DATA
PROJECT ADDRESS
1001 TESCH COURT
WAUKESHA, WI 53186
BUILDING CODE
2015 INTERNATIONAL BUILDING CODE 2009 ANSI 2015 IECC
USE AND OCCUPANCY CLASSIFICATION
F1- UNCHANGED FROM EXISTING
GENERAL BUILDING HEIGHT AND AREA
PROJECT AREA = 509 SF
EXISITING BUILDING AREA: 30,000 SF BUILDING HEIGHT: 27'-6"
ALLOWABLE AREA: 48,000 SF
ALLOWABLE HEIGHT: 75'-0"
TYPE OF CONSTRUCTION
SPRINKLERED
CONSTRUCTION TYPE - 3B

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22 18 14 10 06 02 21 17 13 09 05 01 DETAILS IN THIS SET ARE PLACED ON THE SHEETS AND NUMBERED WITH RESPECT TO THE GRID ABOVE. CONSEQUENTLY, DETAILS ON A GIVEN SHEET MAY OR MAY NOT BE	23	19	15	11	07	03	00	
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ALLIED RESOUR	RCE ADDITION
12 GAUGE COI	NSTRUCTION
PROJECT NUMBER 66660	REVIEWED BY JJS
APPROVED BY JJS	DRAWN BY AYA
ISSUANCES	REVISIONS
CITY OF WAUKESHA PLAN COMMISSION: 02/	27/2019
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G0	01

SCALE: PLOTTED BY:

ORIGINAL SIZE = 24" x 36"

12. ALL EXISTING FURNITURE AND EQUIPMENT REMOVAL TO BE COORDINATED WITH THE OWNER.

13. DASHED LINES DENOTE WALLS, DOORS, TO BE REMOVED.

7. ALL DEMOLITION WHICH DAMAGES ADJACENT SURFACES IS TO BE REPAIRED TO MATCH THE EXISTING SURFACE DAMAGED (MATERIALS & FINISHES) AND ALL REPAIR WORK IS TO BE COORDINATED WITH NEW CONSTRUCTION.

8. PATCH WALLS & ROOF TO MATCH EXISTING CONSTRUCTION BEHIND REMOVAL OF WALL LOUVERS, EXHAUST FANS, INTAKE HOODS & CABINET HEATERS. VERIFY SEQUENCE OF REMOVAL W/ CONSTRUCTION MANAGER. SEE MECHANICAL AND ELECTRICAL DEMO SHEETS FOR WALL, ROOF & FLOOR OPENINGS TO BE PATCHED.

10. REMOVE EXISTING WALL INCLUDING DOORS, WINDOWS, BORROWED LITES, AND ANY EQUIPMENT OR FUNISHINGS ATTACHED TO WALL OR PORTION OF EXISTING WALL AS SHOWN ON FLOOR PLAN (MIN. 4" BELOW FLOOR SLAB) AND AS REQUIRED FOR NEW CONSTRUCTION. FLOOR SURFACE TO BE PATCHED AS REQUIRED TO RECEIVE NEW FLOOR MATERIAL. WALL SURFACE TO BE PATCHED AS REQUIRED TO RECEIVE NEW WALL FINISH. SUPPORT UNBRACED SECTIONS OF WALL OR ROOF AS REQUIRED.

9. ALL TRADES ARE TO COORDINATE THE REMOVAL OF EXISTING LOOSE EQUIPMENT WITH GENERAL CONTRACTOR AND/OR OWNER. ADDITIONAL EQUIPMENT FOUND THAT IS NOT NOTED ON DEMOLITION PLAN SHALL BE REMOVED AS PART OF GENERAL DEMOLITION AFTER VERIFICATION WITH GENERAL CONTRACTOR/OWNER.

DEMOLISION PLAN

DEMOLITION PLAN KEY NOTES 01 REMOVE AND RELOCATE EXISTING DOOR FRAME, DOOR, AND HARDWARE (SEE A101 FOR FINAL LOCATION) 02 REMOVE AND RELOCATE EXISTING OVERHEAD DOOR (SEE A101 FOR FINAL LOCATION) 03 REMOVE EXISTING EXTERIOR METAL STAIR 04 REMOVE EXISTING SLAB FOR NEW LEVELER AND FOUNDATION (SEE S101) 05 REMOVE AND REUSE EXISTING LANDSCAPING BLOCKS BY OTHERS. 06 EXISTING COMPACTOR TO BE MOVED SEE A101 FOR FINAL LOCATION.

WALL TO BE REMOVED

DEMOLITION PLAN GENERAL NOTES

1.DEMOLITION CONTRACTOR IS TO STOP WORK IMMEDIATELY IN AREA IF ASBESTOS IS ENCOUNTERED. NOTIFY CONSTRUCTION MANAGER OF SUSPECT AREA SO PROPER ABATEMENT CAN BE DONE. (UNDER A SEPARATE ASBESTOS ABATEMENT CONTRACT AS NEGOTIATED BY OWNER.)

3. DEMOLITION CONTRACTOR IS TO PROVIDE TEMPORARY SHORING AND BRACING FOR EXIST. ROOF/FLOOR STRUCTURE AS REQUIRED UNTIL PERMANENT WALLS & LINTELS ARE INSTALLED. REFER TO STRUCT. & ARCH. DWGS. FOR BEARING CONDITIONS.

5.ALL TRADES ARE TO COORDINATE ANY DEMOLITION, CAPPING OR ABANDONMENT OF EXISTING

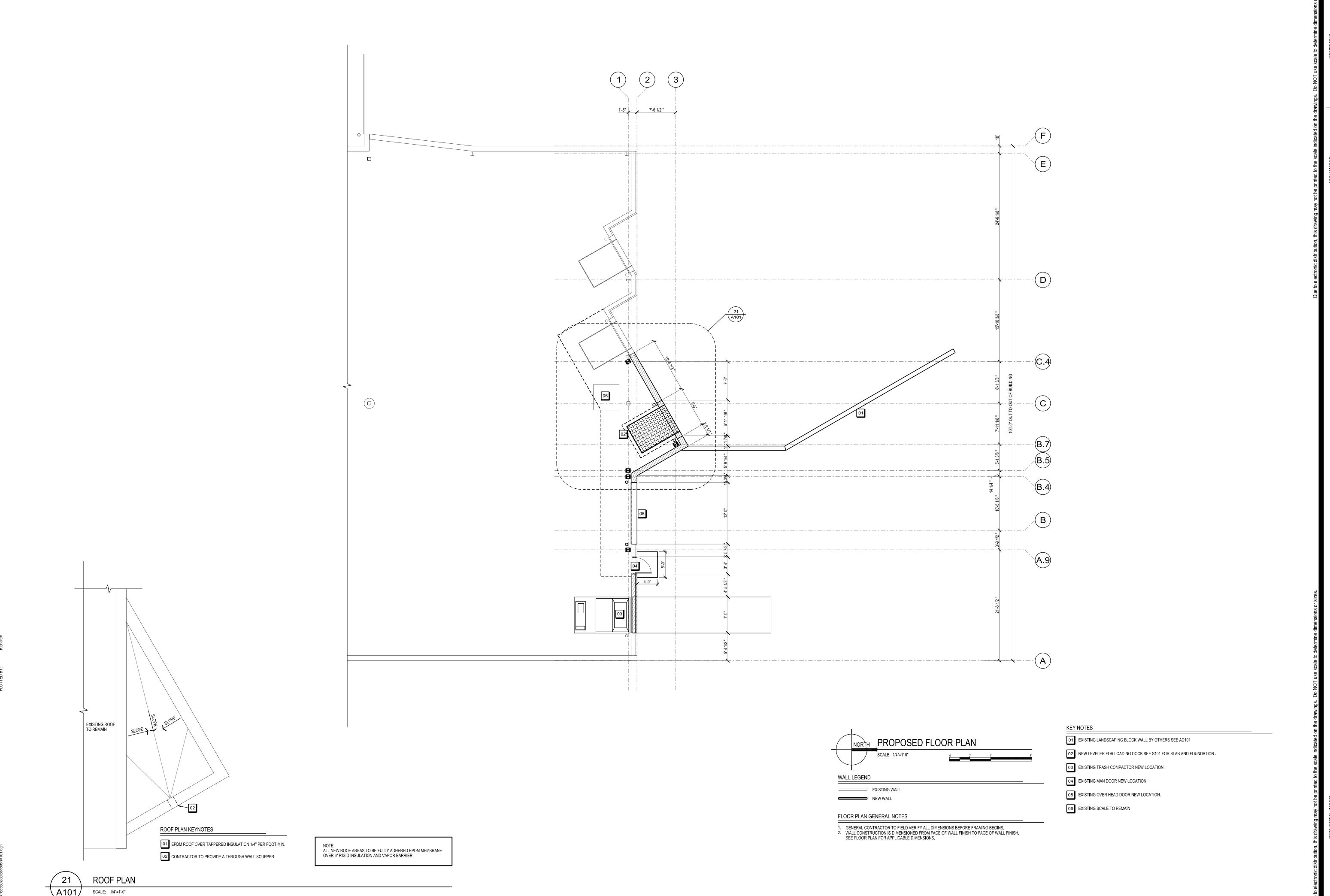
6. ALL ITEMS TO BE SAVED AND/OR RELOCATED ARE TO BE STORED IN A PROPER MANNER SO NO DAMAGE

2. REMOVE ALL EXISTING, WALL PANELING / EXTERIOR CLADDING SYSTEM.

4. ALL EXISTING LIGHT FIXTURES TO BE REMOVED, UNLESS OTHERWISE NOTED.

MECHANICAL, ELECTRICAL, PLUMBING OR ARCHITECTURAL ITEMS.

WILL OCCUR TO THESE ITEMS DURING THEIR STORAGE PERIOD



CITY OF WAUKESHA PLAN COMMISSION: 02/27/2019

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ALLIED RESOURCE ADDITION
2 GAUGE CONSTRUCTION
WAUKESHA, WI

APPROVED BY
JJS
REVIEWED BY
JJS
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A101

FIRST FLOOR PLAN

1. REFERENCE STRUTURAL DRAWINGS FOR CONCRETE SLAB SIZES AND SLAB RELATED INFORMATION. 2. REFERENCE STRUCTURAL, FOR ITEMS NOT SHOWN. COORDINATE AS REQUIRED INCLUDING NECESSARY FRAMING, BLOCKING, ETC.

3. FIELD VERIFY ALL DIMENSIONS PRIOR TO FABRICATION OF ANY FRAMES, STRUCTURAL ITEMS, ETC.

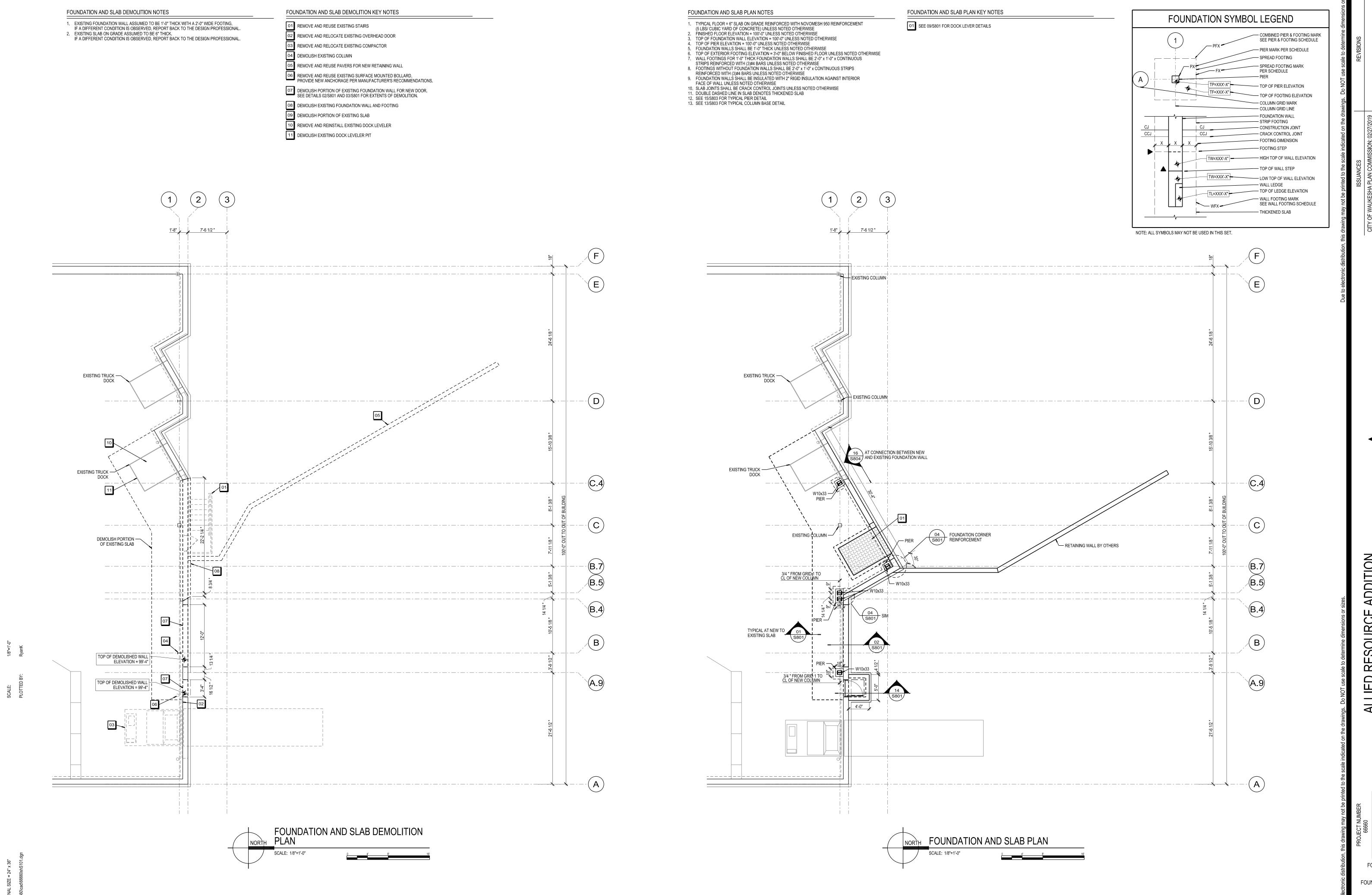
KEY NOTES 1 NEW LOCATION OF EXISTING DOOR

2 NEW LOCATION OF EXISTING OVERHEAD DOOR

3 TRASH COMPACTOR LOCATION, FINAL OPENING PER COMPACTOR REQUIREMENTS.

4 NEW LOADING DOCK

5 MATCH EXISTING METAL SIDING



FOUNDATION AND SLAB
DEMOLITION PLAN
FOUNDATION AND SLAB PLAN

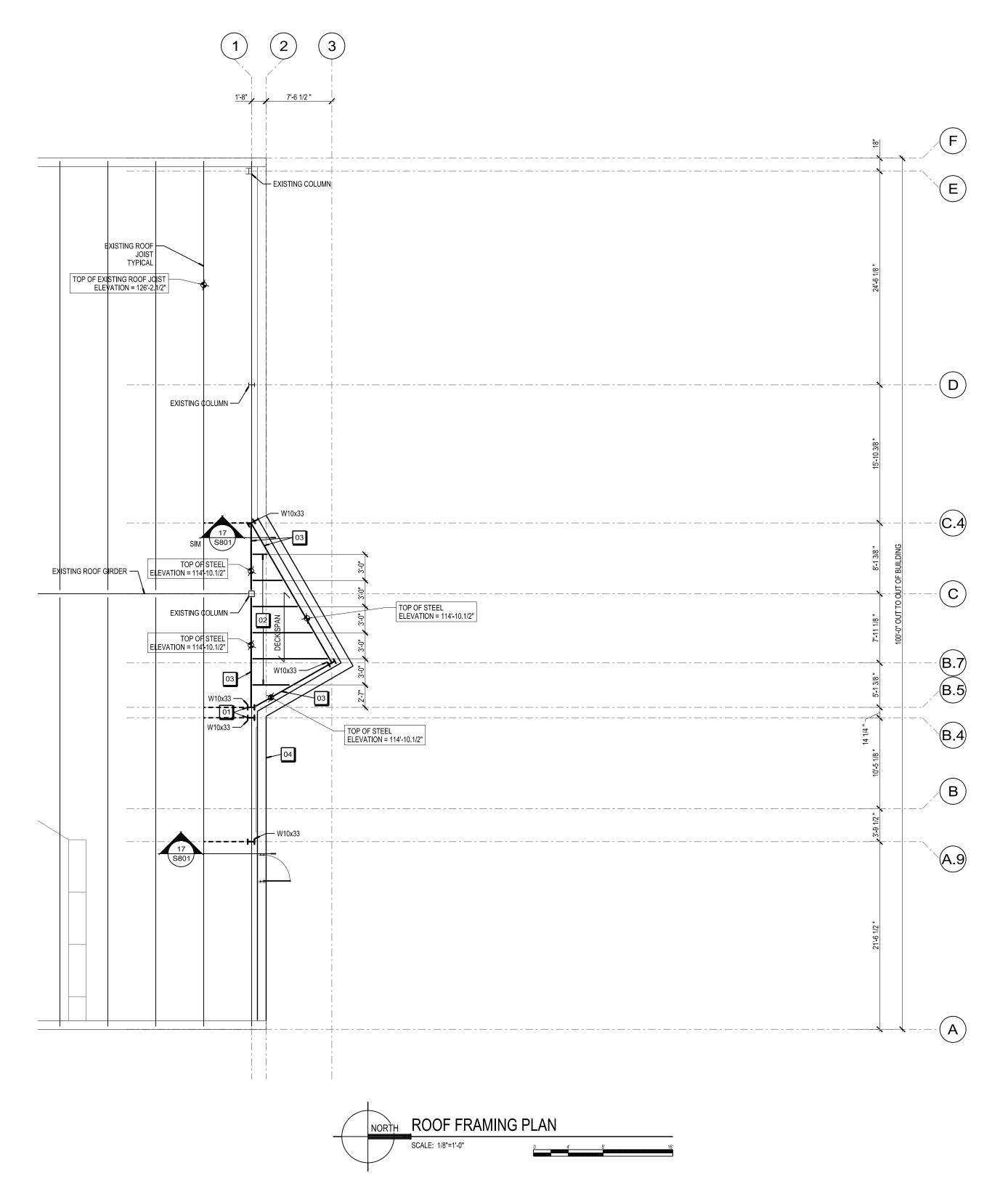
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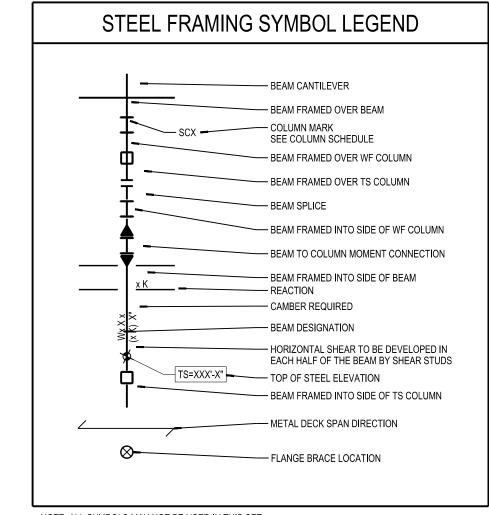
 STEEL DECK SHALL BE 1.1/2"x22 GAGE FASTEND USING A 36/4 PATTERN WITH 5/8" PUDDLE WELDS. PROVIDE (1) SIDE LAP FASTENER PER SPAN USING A 5/8" PUDDLE WELD.
 STRUCTURAL STEEL CONNECTION BOLTS SHALL BE 3/4" DIAMETER UNLESS NOTED OTHERWISE STEEL ROOF FRAMING KEY NOTES

01 ATTACH TOP OF COLUMN PER DETAIL 17/S801
02 W8x15

03 W8x21

04 IF EXISTING WALL BRACING IS ENCOUNTERED, REPORT BACK TO THE DESIGN PROFESSIONAL FOR RELOCATION PLAN.





NOTE: ALL SYMBOLS MAY NOT BE USED IN THIS SET.

rchitecture | Er Interiors | La

Young In Indiana

DE CONSTRUCTION WAUKESHA. WI

APPROVED BY
JJS
SEVIEWED BY
JJS
DRAWN BY
RCK
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S102

FRAMING ELEVATIONS

- THE FOLLOWING NOTES APPLY TO THE PLANS AND/OR SPECIFICATIONS UNLESS NOTED OTHERWISE. IN THE CASE OF CONFLICT WITH PLANS A ND/OR SPECIFICATIONS. THE MORE RESTRICTIVE REQUIREMENT SHALL APPLY.
- CONTRACTOR SHALL BECOME FAMILIAR WITH EACH DRAWING AND DETAIL CONTAINED IN THE DRAWING SET AND REPORT ANY ERRORS, OMISSIONS, DISCREPANCIES, OR DETAILS NOT REFERENCED FOR INSTRUCTIONS FROM THE DESIGN PROFESSIONAL.

- VERIFY AND COORDINATE EXISTING CONDITIONS, DIMENSIONS, AND CONSTRUCTION IN PROGRESS WITH THE SHOP DRAWINGS FOR THE VARIOUS MATERIALS AND BUILDING COMPONENTS PRIOR TO SUBMITTAL, ORDERING ANY MATERIAL, OR COMMENCEMENT OF ANY WORK. ALL DIMENSIONAL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGN PROFESSIONAL.

VERIFY AND COORDINATE WITH ALL CONTRACTORS THE SIZE AND LOCATION OF ALL ARCHITECTURAL AND MECHANICAL APPURTENANCES AND OPENINGS.

- CONSTRUCTION PRACTICE, MEANS AND METHODS, AND JOBSITE SAFETY SHALL REMAIN THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

- CONTRACTOR SHALL REFER TO A RCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, CONCRETE STRUCTURAL, AND OTHER DISCIPLINE DRAWINGS TO COORDINATE ALL MISCELLANEOUS WORK PROVIDED FOR OTHER DISCIPLINES. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING ALL DIVISION OF MATERIALS AND LABOR FOR THE WORK.
- ALL WORK SHALL BE PERFORMED IN COMPLIANCE WITH THE FEDERAL OCCUPATIONAL SA FETY AND HEALTH ADMINISTRATION (OSHA) AND ALL FEDERAL, STATE, AND LOCAL CODES AND ORDINANCES AS THEY PERTAIN TO THIS PROJECT.
- THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACING, SUPPORTS, SHORING, ETC. UNTIL PERMANENT BRACING AND SUPPORT SYSTEMS ARE IN PLACE AND FUNCTIONAL. THE DESIGN, ADEQUACY, AND SAFETY OF TEMPORARY BRACING, SUPPORTS, SHORING, ETC., SHALL REMAIN THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- STRUCTURAL FRAMING AND CONNECTIONS HAVE BEEN DESIGNED FOR THE FINAL COMPLETED CONDITION AND HAVE NOT BEEN INVESTIGATED FOR POTENTIAL LOADINGS ENCOUNTERED DURING CONSTRUCTION. INVESTIGATION OF THE FRAMING AND CONNECTIONS FOR A DEQUACY DURING CONSTRUCTION SHALL REMAIN THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- CONTRACTOR SHALL REROUTE EXISTING MECHANICAL, ELECTRICAL, AND PLUMBING WORK NOT OTHERWISE INDICATED TO FACILITATE NEW CONSTRUCTION AND SHALL SUBMIT PLANS FOR APPROVAL INDICATING EXISTING AND REPOUTED LOCATIONS.
- SUBSTITUTIONS FOR PROPRIETARY STRUCTURAL PRODUCTS DESIGNATED ON THE DRAWINGS SHALL BE A PPROVED BY THE DESIGN PROFESSIONAL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NECESSARY INFORMATION USED TO DETERMINE ADEQUACY OF PROPOSED SUBSTITUTIONS INCLUDING STRUCTURAL CALCULATIONS IF NECESSARY

SUBMITTAL REVIEW

SUBMITTALS ARE ALL ITEMS REQUESTED TO BE SUBMITTED FOR REVIEW AND INCLUDE STRUCTURAL CALCULATIONS AND SHOP DRAWINGS. SHOP DRAWINGS INCLUDE BOTH ERECTION AND PRODUCTION DRAWINGS.

SUBMITTALS SHALL BE PROVIDED FOR REVIEW AND MARKED "FOR APPROVAL".

- PRIOR TO SUBMITTAL TO THE DESIGN PROFESSIONAL, THE CONTRACTOR SHALI 1) REVIEW EACH SUBMISSION FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS; 2) REVIEW EACH SUBMISSION FOR CONFORMANCE WITH THE MEANS, METHODS,
- TECHNIQUES, SEQUENCES, OPERATIONS OF CONSTRUCTION, AND SAFETY PRECAUTIONS AND PROGRAMS INCENDENTAL THERETO, ALL OF WHICH ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR:
- 3) IDENTIFY ANY VARIATIONS FROM THE CONTRACT DOCUMENTS, 4) APPROVE AND STAMP COMPLIANT SUBMISSIONS THUS. NON COMPLIANT
- SUBMITTALS SHALL BE REVISED UNTIL COMPLIANT.

ALLOW TEN (10) WORKING DAYS FROM THE DATE OF SUBMISSION FOR THE DESIGN PROFESSIONAL'S SUBMITTAL REVIEW. ALLOW MORE FOR LARGE SUBMITTALS.

- SUBMITTALS SHALL BE REVIEWED AND MARKED "PROCESSED" OR "APPROVED" BY ALL REVIEWING PARTIES PRIOR TO THE START OF FABRICATION.
- REVIEW BY THE DESIGN PROFESSIONAL IS TO CONFIRM THAT THE DETAILED WORK (WHEN COMPLETE) CONFORMS TO THE DESIGN INTENT AND IS ABLE TO PERFORM AS AN INTEGRAL PART OF THE COMPLETED BUILDING SYSTEM SHOWN ON THE CONTRACT DOCUMENTS. A COMPLETED REVIEW BY THE DESIGN PROFESSIONAL IS NOT AN APPROVAL OF CHANGES SHOWN IN THE SUBMITTAL, HAS NOT CHECKED DIMENSIONS SHOWN IN THE SUBMITTAL, AND DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITIES FOR CORRECTNESS OF THE DETAILS, ERRORS, AND CONFORMANCE WITH THE CONTRACT.
- FINAL STRUCTURAL DESIGN CALCULATIONS AND SHOP DRAWINGS SHALL BE PROVIDED FOR RECORD PURPOSES AND MARKED "FOR CONSTRUCTION" PRIOR TO INSTALLATION.

CONSTRUCTION ELEMENTS DESIGNATED AS STRUCTURAL COMPONENTS ARE DESIGNED BY PARTIES OTHER THAN THE DESIGN PROFESSIONAL OF RECORD AND HAVE THE ADDITIONAL REQUIREMENTS OF SUBMITTAL TO AUTHORITIES HAVING JURISDICTION. SUBMIT STRUCTURAL COMPONENT FINAL STRUCTURAL DESIGN CALCULATIONS, SHOP DRAWINGS, AND THE REQUIRED SUBMITTAL FEE TO THE DESIGN PROFESSIONAL FOR REVIEW, SIGNATURE, AND SUBMITTAL TO THE AUTHORITY HAVING JURISDICTION.

STRUCTURAL COMPONENT SUBMITTALS TO THE AUTHORITY HAVING JURISDICTION ARE REQUIRED TO BE COMPLETED PRIOR TO INSTALLATION OR FINES COULD BE IMPOSED BY THE AUTHORITY HAVING JURISDICTION. ALL FINES WILL BE BACKCHARGED TO THE CONTRACTOR.

EARTHWORK

- CONTACT PROPER A UTHORITIES TO LOCATE EXISTING UNDERGROUND UTILITIES PRIOR TO EXCAVATION.
- A LICENSED GEOTECHNICAL ENGINEER ACCEPTABLE TO THE OWNER SHALL BE RETAINED BY THE CONTRACTOR TO INSPECT, TEST, APPROVE, DOCUMENT, AND REPORT ALL BEARING CONDITIONS AND COMPACTED FILL INSTALLATIONS PRIOR TO CONCRETE PLACEMENT. FOUNDATION CONSTRUCTION MAY BE ADJUSTED BY THE DESIGN PROFESSIONAL IF REQUIRED BY THE GEOTECHNICAL ENGINEER. CONSULT THE DESIGN PROFESSIONAL BEFORE PROCEEDING.
- CONTRACTOR SHALL COORDINATE AND SCHEDULE WHEN THE GEOTECHNICAL ENGINEER IS TO BE ON SITE NO FOOTINGS OR FOUNDATIONS SHALL BE PLACED WITHOUT PRIOR APPROVAL FROM THE GEOTECHINCAL ENGINEER.
- IF EXCAVATION INDICATES A SOIL BEARING CAPACITY LESS THAN DESIGN CAPACITY AT FOOTING DEPTH, CONSULT THE DESIGN PROFESSIONAL BEFORE PROCEEDING.
- FOOTINGS SHALL BE CAST ON UNDISTURBED SOIL, COMPACTED FILL, OR CONTROLLED LOW STRENGTH MATERIAL (CLSM).
- COMPACT ALL SUBGRADE BELOW FOOTINGS PRIOR TO CONCRETE PLACEMENT

- HOLES. TRENCHES, OR DISTURBANCES IN THE SOIL SHALL NOT BE ALLOWED WITHIN THE VOLUME DESCRIBED BY 45 DEGREE LINES SLOPING FROM THE BOTTOM EDGE OF THE FOOTING. IF SUCH ARE REQUIRED, CONSULT THE DESIGN PROFESSIONAL BEFORE
- BOTTOM OF FOOTINGS ADJACENT TO EXISTING FOUNDATIONS SHALL BE AT THE SAME ELEVATION AS THE EXISTING FOUNDATION UNLESS A 2:1 MAXIMUM SLOPE (HORIZONTAL TO VERTICAL) IS MAINTAINED BETWEEN BOTTOM OF FOOTINGS.
- DO NOT PLACE UNDERGROUND UTILITES OR PIPES BELOW FOOTINGS WITHOUT CONSULTING THE DESIGN PROFESSIONAL BEFORE PROCEEDING.
- TOPSOIL OR UNACCEPTABLE SOIL BELOW SLABS ON GRADE SHALL BE REMOVED. SUBGRADE UNDER SLABS SHALL BE ENGINEERED FILL COMPACTED IN 6" LAYERS.
- COMPACT ALL SUBGRADE MATERIAL PRIOR TO PLACMENT OF ANY FILL. REMOVE
- LOOSE MATERIAL AND DEBRIS THAT CANNOT BE ADEQUATELY COMPACTED.
- BACKFILL AGAINST ANY WALLS OR CONSTRUCTION SHALL NOT BE PLACED UNLESS THE WALLS ARE A DEQUATELY BRACED TO WITHSTAND THE LOADS IMPOSED DUE TO THE BACKFILLING OPERATION.
- DESIGN, FURNISH, AND INSTALL ALL TEMPORARY SHEETING AND SHORING NECESSARY TO MAINTAIN THE EXCAVATION AND PROTECT SURROUNDING STRUCTURES AND

PROVIDE ANY DEWATERING REQUIRED DURING EXCAVATION AND CONSTRUCTION.

PROVIDE MINIMUM 24 INCHES OF WASHED STONE OVER ALL DRAIN TILES AND 4 INCHES

- CONTRACTOR SHALL PROVIDE SUBMITTALS FOR THE CONCRETE.
- CONCRETE WORK SHALL CONFORM TO THE ACI MANUAL OF STANDARD PRACTICE INCLUDING BUT NOT LIMITED TO A CI 301 ("SPECIFICATIONS FOR STRUCTURAL CONCRETE") ACI 305("HOT WEATHER CONCRETING"), AND ACI 306("COLD WEATHER CONCRETING"). CONCRETE DETAILING SHALL CONFORM TO ACI SP-66("ACI DETAILING MANUAL").

A CERTIFIED TESTING AGENCY ACCEPTABLE TO THE OWNER SHALL BE RETAINED BY THE CONTRACTOR TO INSPECT, TEST, APPROVE, DOCUMENT, AND REPORT ON ALL

- CONTRACTOR SHALL NOTIFY DESIGN PROFESSIONAL, BUILDING INSPECTOR, AND TESTING AGENCY AT LEAST 24 HOURS PRIOR TO PLACING CONCRETE.
- SLABS ON GRADE SHALL BE CAST ALLOWING A SUFFICIENT NUMBER OF JOINTS TO ADEQUATELY CONTROL SHRINKAGE CRACKING. GENERALLY, JOINTS SHALL OCCUR ON COLUMN CENTERLINES. JOINTS SHALL BE SPACED IN SUCH A WAY THAT THE LENGTH TO WIDTH RATIO OF UN-JOINTED SLAB PORTION SHALL NEVER BE GREATER THAN 2. MAXIMUM SPACING BETWEEN JOINTS SHALL BE 15'-0".
- SLABS ON GRADE SHALL HAVE THE FOLLOWING FINISH UNLESS NOTED OTHERWISE: EXTERIOR SLABS - BROOM FINISH INTERIOR SLABS - TROWEL FINISH
- SLABS ON GRADE SHALL HAVE A 6 MIL VAPOR RETARDER BELOW SLAB UNLESS NOTED OTHERWISE
- WELDED WIRE REINFORCEMENT IN SLABS ON GRADE SHALL BE PLACED WITHIN 2" FROM THE TOP OF THE SLAB. CHAIRS SHALL BE USED. LIFTING THE WIRE DURING CONCRETE PLACEMENT IS NOT PERMITTED.
- SLOPE SLAB ON GRADE 1/4" PER FOOT TO DRAINS UNLESS NOTED OTHERWISE.
- SLOPE SLAB ON GRADE AT OVERHEAD DOORS 1/2" IN DEPTH OF DOOR JAMB AWAY FROM BUILDING.
- SAW-CUTTING SHALL BE DONE AS SOON AS THE CONCRETE HAS HARDENED SUFFICIENTLY TO PREVENT THE AGGREGATE FROM BEING DISLODGED BY THE SAW AND SHALL BE COMPLETED BEFORE SHRINKAGE STRESSES BECOME SUFFICIENT TO PRODUC! CRACKING, WITHIN 16 HOURS MAXIMUM OF THE INITIAL CASTING OPERATION.

ALLOW AT LEAST 24 HOURS BEFORE POURING A DJACENT WALL SECTIONS BETWEEN CONSTRUCTION JOINTS.

- MAX LENGTH OF POUR TO BE 50 FEET UNLESS CRACK INDUCERS ARE USED.
- ALL REINFORCEMENT SHALL BE SECURELY HELD IN POSITION BY SUITABLE ACCESSORIES PRIOR TO CONCRETE PLACEMENT EXCEPT AS NOTED OR SPECIFIED.
- DO NOT SAW-CUT SUSPENDED FLAT SLAB CONSTRUCTION JOINTS NOT PERMITTED UNLESS APPROVED IN WRITING BY THE DESIGN PROFESSIONAL. LOCATIONS MUST BE APPROVED BY THE DESIGN PROFESSIONAL.

CONSTRUCTION JOINTS IN BEAMS, JOISTS, OR SLABS SHALL BE LOCATED BETWEEN THE 1/4 POINT AND CENTERLINE OF SPAN UNLESS NOTED OTHERWISE OR AS DIRECTED BY THE DESIGN PROFESSIONAL.

- PIERS SHALL BE POURED MONOLITHICALLY WITH WALLS.
- BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 4'-0" BELOW FINISHED GRADE UNLESS NOTED OTHERWISE
- FOOTINGS SHALL BE CENTERED BELOW WALLS, PIERS, AND COLUMNS ABOVE UNLESS

NOTED OTHERWISE.

- SLOPE EXTERIOR STOOPS 1/4" PER FOOT AWAY FROM BUILDING.

TOP OF FOUNDATION WALL SHALL BE DROPPED 8" AT DOOR THRESHOLDS FOR FLOOR SLAB CAP. WIDTH OF CAP SHALL BE EQUAL TO WIDTH OF ROUGH OPENING.

- WHERE REINFORCING IS CALLED FOR IN PORTIONS OF THE BUILDING, IT SHALL BE DUPLICATED IN SIMILAR PORTIONS OF THE BUILDING.
- DOWELS INTO FOUNDATION SHALL BE SAME NUMBER AND SIZE AS WALL/PIER/COLUMN
- WHERE FOOTINGS ARE CONSTRUCTED ON OR ADJACENT TO CONTINUOUS STRIP FOOTINGS, DOWEL INTO EXISTING 9" MIN. AND GROUT SOLID.
- BARS SPLICES SHALL BE LAPPED WITH CLASS B SPLICE LENGTHS UNLESS NOTED OTHERWISE LAP WELDED WIRE REINFORCEMENT 6 INCHES.
- HORIZONTAL REINFORCING IN CONCRETE WALLS SHALL BE CONTINUOUS
- VERTICAL REINFORCING IN CONCRETE WALLS SHALL END 2" FROM THE TOP OF THE
- PROVIDE (2) #4 BARS AS STIRRUP CARRY BARS WHERE TOP STEEL IS UNAVAILABLE TO SUPPORT STIRRUPS.
- PROVIDE CORNER BARS OF SAME SIZE AND SPACING AS HORIZONTAL BARS AT WALL CORNERS. BARS SHALL BE 40 BAR DIAMETERS IN LENGTH EACH WAY.
- PROVIDE INTERSECTION BARS OF SAME SIZE AND SPACING AS HORIZONTAL BARS AT WALL INTERSECTIONS. BARS SHALL BE 40 BAR DIAMETERS IN LENGTH WITH STANDARD HOOKS EACH WAY AT INTERSECTING WALL.
- DO NOT CUT OR PLACE HOLES IN CONCRETE SLABS, BEAMS, COLUMNS, OR WALLS WITHOUT PRIOR APPROVAL OF THE DESIGN PROFESSIONAL

- WALL OR SLAB PENETRATIONS SHALL HAVE (2) #4 BARS DIAGONALLY AT CORNERS OF OPENINGS CENTERED IN SLAB: EXTEND 1'-0" BEY OND THE EDGE OF THE PENETRATION IN EACH DIRECTION.
- SLABS SHALL HAVE #4 BARS DIAGONALLY AT INSIDE CORNERS OF WALLS, PIERS, AND FOUNDATION DROPS FOR SLAB CAPS UNLESS A CONTROL JOINT IS PROVIDED AT THE CORNER.
- PIPES AND CONDUITS EMBEDDED IN OR PASSING THROUGH STRUCTURAL MEMBERS MUST BE A PPROVED BY THE DESIGN PROFESSIONAL. PIPE A ND CONDUIT EMBEDDED IN CONCRETE SHALL NOT BE LARGER IN OUTSIDE DIAMETER AT ITS WIDEST POINT OR FITTING THAN 2 INCHES OR 1/3 OF THE THICKNESS OF THE SLAB, BEAM, OR WALL; AND SHALL BE LOCATED AND PLACED AS SUCH:
 - 1. NOT CLOSER THAN THREE DIAMETERS ON CENTER. 2. CONCRETE COVER IS NOT LESS THAN 2 INCHES.

3. NO REINFORCING SHALL BE DISPLACED.

WHERE AN APPROVED PIPE OR CONDUIT EXTENDS THROUGH A BEAM, PROVIDE ONE ADDITIONAL STIRRUP ON EACH SIDE OF THE OPENING.

COORDINATE AND VERIFY SIZE AND LOCATION OF ALL OPENINGS, SLEEVES, CHASES, CONDUITS, DEPRESSED AREAS, FLOOR FINISHES, FILLS, ANCHORS, STONE AND MASONRY INSERTS, HANGERS, CURBS, AND OTHER MISCELLANEOUS ITEMS BEFORE

- FOUNDATION WALLS SHALL HAVE PILASTERS UNDER ALL WALL PILASTERS.
- CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE
- A MINIMUM OF 4 CONCRETE CYLINDERS SHALL BE TAKEN FOR EACH CONCRETE PORTION TESTED.

CONCRETE REINFORCMENT

- CONTRACTOR SHALL PROVIDE SUBMITTALS FOR THE CONCRETE REINFORCEMENT.

- CONTRACTOR SHALL PROVIDE SUBMITTALS FOR THE STRUCTURAL STEEL
- WELDS SHALL BE AWS PRE-QUALIFIED, MADE WITH A E70XX ELECTRODE, AND SHALL BE PERFORMED BY WELDERS, AWS CERTIFIED FOR WELDS MADE
- COLUMN BASE PLATES WITH OVERSIZE HOLES SHALL HAVE PLATE WASHERS PROVIDED WITH A NCHOR BOLTS.
- ALL BEAM TO COLUMN FLANGE CONNECTIONS ARE TO CONFORM TO AISC SINGLE PLATE SHEAR CONNECTIONS UNLESS NOTED OTHERWISE - SEE SINGLE PLATE SCHEDULE
- ALL BEAM TO COLUMN WEB CONNECTIONS ARE TO CONFORM TO AISC DOUBLE ANGLE SHEAR CONNECTIONS UNLESS NOTED OTHERWISE.
- PROVIDE ANGLE FRAMING WITH LONG LEGS VERTICAL FOR ROOF PENETRATIONS GREATER THAN 6" x 6" UNLESS NOTED OTHERWISE

THE MINIMUM END CONNECTION OF ANY MEMBER SHALL BE MADE WITH TWO (2) A325 3/4" DIA METER BOLTS OR EQUIVALENT WELD UNLESS NOTED OTHERWISE.

TENSION BOLTED CONNECTIONS IN A CCORDANCE WITH A ISC REQUIREMENTS AND USE LOAD INDICATING WASHERS. CALIBRATED WRENCH TIGHTENING IS NOT ALLOWED. TURN-OF-THE-NUT TENSIONING (IF USED) SHALL HAVE THE ADDITIONAL REQUIREMENT OF PAINT STICK MATCH MARKING. PAINT SHALL MARK THE CONNECTION SURFACE, THE BOLT, AND THE NUT.

- CONNECT GUSSET PLATES FOR DIAGONAL BRACING TO ALL INTERSECTING MEMBERS
- SHELF ANGLES AT COLUMNS SHALL SUPPORT DECK, GRATING, AND PLATE - ALL STRUCTURAL AND MISCELLA NEOUS STEEL EXPOSED TO WEATHER SHALL HAVE

CONTINUOUS SEAL WELDS AT ALL JOINTS, IN ADDITION TO STRENGTH WELDS.

- CLEAN, PREPARE, AND SHOP PRIME STRUCTURAL STEEL MEMBERS IN ACCORDANCE WITH S.S.P.C. STANDARDS SP-1 AND SP-6.

ALL STEEL REMAINING EXPOSED TO VIEW SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC CODE OF STANDARD PRACTICE SECTION PERTAINING TO ARCHITECTURALLY EXPOSED STRUCTURAL STEEL, WITHOUT GAPS OR OPEN JOINTS

STEEL DECK

- CONTRACTOR SHALL PROVIDE SUBMITTALS FOR THE STEEL DECK.
- DECK END LAPS SHALL BE 3-INCH MINIMUM AND SHALL OCCUR AT SUPPORTS. - L3 x 3 x 3/16 x CONTINUOUS SHALL BE PLACED AT UNSUPPORTED DECK EDGES AROUND PERIMETER OF ROOF DECK UNLESS NOTED.
- PROVIDE DECK CLOSURE CHANNELS AT PERIMETER OF ALL DECK PENETRATIONS INCLUDING HVACAND ROOF DRAIN SUMP PANS
- PROVIDE ROOF DRAIN SUMP PANS FOR INTERNAL DRAINS UNLESS NOTED OTHERWISE
- ROLLED IN HANGER TABS ARE NOT PERMITTED IN STEEL DECK.

DECK SHALL BE 3 SPAN CONTINUOUS, MINIMUM UNLESS NOTED OTHERWISE. CONSULT THE DESIGN PROFESSIONAL BEFORE PROCEEDING IF 3 SPAN CONTINUOUS CONDITION IS NOT POSSIBLE

WELD LIGHT GAIGE STEEL POUR STOPS TO BEAMS AND/OR JOISTS.

MISCELLANEOUS

PROVISIONS SHALL BE TAKEN TO ASSURE NON-STRUCTURAL COMPONENTS ARE NOT DISTORTED OR DAMA GED BY DEFLECTION OF THE STRUCTURE. INCLUDE DEFLECTION TRACK AT ALL INTERIOR NON-LOAD BEARING STUD WALLS.

EDGE OF ROOF FLASHINGS SHALL ALLOW RAINWATER TO PASS THROUGH.

POWDER-A CTUATED FASTENERS SHALL FOLLOW ANSI A 10.3 SPECIFICATIONS

ADHESIVE ANCHORS SHALL BE SIMPSON STRONG TIE ACRY LIC-TIE ADHESIVE UNLESS

EXPANSION ANCHORS SHALL BE SIMPSON STRONG TIE STRONG-BOLT UNLESS NOTED OTHERWISE OR APPROVED BY THE DESIGN PROFESSIONAL.

THE MINIMUM END CONNECTION OF GIRTS SHALL BE MADE WITH TWO (2) A325

1/2" DIA METER BOLTS OR EQUIVALENT WELD UNLESS NOTED OTHERWISE

SLEEVE ANCHORS SHALL BE SIMPSON STRONG TIE SLEEVE-ALL UNLESS NOTED OTHERWISE OR APPROVED BY THE DESIGN PROFESSIONAL.

NOTED OTHERWISE OR APPROVED BY THE DESIGN PROFESSIONAL.

STRUCTURAL DESIGN SPECIFICATIONS

- THE DESIGN PROFESSIONAL WAIVES ANY AND ALL RESPONSIBILITY AND LIABILITY FOR PROBLEMS A RISING FROM FAILURE TO FOLLOW THESE PLANS, SPECIFICATIONS, AND THE DESIGN INTENT THEY CONVEY, OR FOR PROBLEMS ARISING FROM OTHERS' FAILURE TO OBTAIN AND/OR FOLLOW THE DESIGN PROFESSIONAL'S GUIDANCE WITH RESPECT TO ANY ERRORS, OMISSIONS, INCONSISTENCIES, AMBIGUITIES, OR CONFLICTS
- DESIGN, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH LOCAL, STATE, AND FEDERAL REGULATIONS APPLICABLE TO WORK AND PROJECT LOCATION
- BUILDING ANALYSIS AND DESIGN WERE AND SHALL BE BASED ON THE STATE OF WISCONSIN BUILDING CODE AS AMENDED TO DATE.

EDITION AND DATE OF REFERENCED STANDARDS SHALL BE AS LISTED IN THE BUILDING

DESIGN LOADS:

55.6		
ROOFING =	30.5	PSF
CEILING =	5	PSF
MISCELLA NEOUS =	5	PSF
ROOF CONCENTRATED AT ANY POINT =	2000	LBS
FLOOR CONCENTRATED AT ANY POINT =	2000	LBS
FLOOR LIVE		
HEAVY STORAGE =	250	PSF
CONCENTRATED LOAD AT ANY POINT ON PRIMARY FR	AMING MEN	MBERS
(NON-CONCURRENT WITH UNIFORM LOAD) =	300	LBS
SMALL VEHICLE ACCESSIBLE AREAS =	50	PSF
LARGE VEHICLE ACCESSIBLE AREAS =	HS20-44	
ROOF LIVE		
TRIBUTARY AREA: <= 200	20	PSF
200-600	INTERPOL	ATE
=> 600	12	PSF

CONCENTRATED LOAD AT ANY POINT ON PRIMARY FRAMING MEMBERS (NON-CONCURRENT WITH UNIFORM LOAD) = 2000 LBS SNOW EXPOSURE FACTOR (C_e) = 1.0 THERMAL FACTOR $(C_i) = 1.2$ SNOW IMPORTANCE FACTOR (I_s) = 1.00 GROUND SNOW LOAD $(P_g) = 30$ PSF FLAT ROOF SNOW LOAD (P_f) = 25.2 PSF ROOF SLOPE FACTOR (C_{s}) = 1.0 SLOPED ROOF SNOW LOAD $(P_s) = 25.2$ PSF WIND (ASCE 7-98)

3 SECOND GUST BASIC WIND SPEED (V) = 90 MPH

WIND IMPORTANCE FACTOR $(I_{M}) = 1.00$

EAST-WEST WIND EXPOSURE CATEGORY = B BUILDING CATEGORY: PARTIALLY ENCLOSED INTERNAL PRESSURE COEFFICIENTS: +0.55 COMPONENTS AND CLADDING LOADS: NOTE - REFER TO ASCE 7 FOR ZONE DIAGRAMS ZONE WIDTH (A) = 10 FT

NORTH-SOUTH WIND EXPOSURE CATEGORY = B

ROOFS:		
TRIBUTARY AREA LESS THAN:	10	SQ FT
ZONE 1 =	10.5	PSF
	-19.1	PSF
ZONE 2 =	10.5	PSF
	-29.0	PSF
ZONE 3 =	10.5	PSF
	-41.4	PSF
TRIBUTARY AREA GREATER THAN:	100	SQ FT
ZONE 1 =	9.3	PSF
	-17.9	PSF
ZONE 2 =	9.3	PSF
	-20.4	PSF
ZONE 3 =	9.3	PSF
	-20.4	PSF
WINDWARD WALLS:		

NOTE - INTERPOLATE FOR TRIBUTARY AREAS NOT SHOWN

TRIBUTARY AREA LESS THAN: 10 SQ FT ZONE 4 = 19.1 PSF 19.1 PSF -24.1 PSF TRIBUTARY AREA GREATER THAN: 500 SQ FT 15.4 PSF -16.7 PSF

ZONE 5 = 15.4 PSF -16.7 PSF LEEWARD AND SIDE WALLS: TRIBUTARY AREA LESS THAN: 10 ZONE4 =19.1 PSF -20.4 PSF -24.1 PSF

-16.7 PSF

TRIBUTARY AREA GREATER THAN: 500 SQ FT ZONE 4 = 15.4 PSF -16.7 PSF ZONE 5 = 15.4 PSF

SEISMIC IMPORTANCE FACTOR, $I_E =$ SEISMIC USE GROUP = SHORT PERIOD MAPPED SPECTRAL RESPONSE (S_s) = 0.089 g 1 SECOND PERIOD MA PPED SPECTRAL RESPONSE (S_D) = 0.047 g SITE CLASS = D SHORT PERIOD DESIGN SPECTRAL RESPONSE $(S_{DS}) = 0.094$ 1 SECOND PERIOD DESIGN SPECTRAL RESPONSE $(S_{D1}) = 0.075$ g SEISMIC DESIGN CATEGORY = B NORTH-SOUTH DESIGN BASE SHEAR = 0.13 K EAST-WEST DESIGN BASE SHEAR = 0.13 K NORTH-SOUTH SEISMIC RESPONSE COEFFICIENT (C_s) = 0.032 EAST-WEST SEISMIC RESPONSE COEFFICIENT (C_s) = 0.032

NORTH-SOUTH RESPONSE MODIFICATION FACTOR (R) = 3

EAST-WEST RESPONSE MODIFICATION FACTOR (R) = 3

A NA LYSIS PROCEDURE = ELF BASIC SEISMIC FORCE RESISTING SYSTEM NORTH-SOUTH: SYSTEM: STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN

EAST-WEST:

SYSTEM: STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE, EXCLUDING CANTILEVER COLUMN

- FLOOR LIVE LOADS SHALL BE POSTED USING DURABLE SIGNS IN EACH AREA OF EACH

- THE LATERAL SUPPORT SYSTEM INCORPORATES A ROOF DIAPHRAGM.

STORY TO WHICH THEY APPLY.

- DEFLECTION DUE TO WIND LOADS SHALL BE CALCULATED BASED ON 0.7 TIMES THE COMPONENTS AND CLADDING LOADS.

- THE LATERAL SUPPORT SYSTEM INCORPORATES A FLOOR DIAPHRAGM.

STRUCTURAL COMPONENT DESIGN NOTES:

- DRIFTING AND SLIDING SNOW SHALL BE CONSIDERED IF REQUIRED WORST CASE LOADING COMBINATIONS SHALL BE USED UNBALANCED LOADING SHALL BE CONSIDERED

STRUCTURAL COMPONENTS SHALL BE DESIGNED INCLUDING THE WEIGHT OF ALL SPRINKLER MAINS WHEN FILLED.

FOOTINGS WERE DESIGNED USING A NET SOIL BEARING CAPACITY OF 2000 PSF.

THE DESIGN SOIL BEARING CAPACITY WAS BASED ON PRESUMPTIVE VALUES AND SHALL BE CONFIRMED BEFORE OR DURING CONSTRUCTION.

DESIGN AND FABRICATION OF WELDS SHALL BE IN ACCORDANCE WITH THE AWS STRUCTURAL WELDING CODE AND THE AISC MANUAL OF STANDARD PRACTICE

DESIGN, FABRICATION, AND ERECTION OF STEEL DECK, ACCESSORIES, AND ATTA CHMENTS SHALL BE IN ACCORDANCE WITH THE S.D.I. DESIGN SPECIFICATIONS

AND MANUAL OF STANDARD PRACTICE. DESIGN, FABRICATION, AND ERECTION OF HOT FORMED STEEL MEMBERS SHALL BE IN ACCORDANCE WITH THE AISC DESIGN SPECIFICATIONS AND MANUAL OF STANDARD

DESIGN, FABRICATION, AND ERECTION OF CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318 ("BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE").

UM 28 DAY CONCRETE CYLINDER STRENG	TH SHALL BE:
MUD SLABS	500 PSI
FOOTINGS	3000 PSI
FOUNDATION WALLS	4000 PSI
PIERS	4000 PSI
SLABS ON GRADE	4000 PSI

- EXTERIOR EXPOSED CONCRETE SHALL BE AIR ENTRAINED.
- REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60.

STRUCTURAL STEEL

W-SHAPE BEAMS AND COLUMNS SHALL CONFORM TO ASTM A992, Fy = 50ksi PLATES, ANGLES, AND CHANNELS SHALL CONFORM TO ASTM A36, Fy = 36ks

STRUCTURAL STEEL CONNECTIONS NOT FULLY DETAILED ON THE CONTRACT DOCUMENTS SHALL BE DESIGNED BY THE SUPPLIER.

STEEL BEAM CONNECTIONS SHALL SUPPORT MINIMUM 75% THE TOTAL UNIFORM

 \cdot ALL STRUCTURAL STEEL CONNECTIONS SHALL BE DESIGNED FOR A MINIMUM VERTICAL LOAD OF 10 KIPS.

CAPACITY SHOWN IN THE AISC TABLES OF UNIFORM LOAD CONSTANTS FOR THE GIVEN

BEAM, SPAN, AND STEEL, EXCEPT WHERE SPECIFICALLY NOTED OR DETAILED. INCLUDE ECCENTRICTY IN CONNECTION CAPACITY

BASE PLATE GROUT SHALL CONFORM TO ASTM C1107 GRADE C WITH A MINIMUM COMPRESSIVE STRENGTH: f'c = 5000 PSI

LIMIT DEFLECTION ON EXTERIOR WALL COMPONENTS TO L/600 AT VENEER LOCATIONS.