#### Fox Run Redevelopment

#### Legal Description:

PT SE1/4 SEC 8 & SW1/4 SEC 9 T6N R19E COM SE COR SE1/4, N 153.02' TO BEG, N56 11'57 E 29.89', N22 19'E 198.74', N23 54'E 260.33',S85 41'32 W 206.36', N 183.94', S88 49'42 W 815',S1 10'18 E 433.16',N88 49'42 E 306.08', S1 10'18E 223.7', N88 49'42E418.32', N56 11'57 E 92.84'TO BEG DOC NO 4246692

PT SE1/4 SEC 8 T6N R19E COM SECOR SE1/4, S88 49'42 W ON S LI SE1/4 996.79', DUEN 358.51' TO BEG;DUE N 401.49';N89 49'42 E 181.79'; S1 10'18 E 433.16'; S88 49'42 W 93.92'; N1 10'18 W 31.75'; N88 49'42 E96.08' TO BEG 1.785 AC DOC NO 4246692

Sections 8 and 9, Township 6 North, Range 19 East, City of Waukesha, Waukesha County, Wisconsin

#### STORM WATER MANAGEMENT PLAN

January 27, 2020

Owner: Fox Run 3,LLC c/o Somerstone LLC 19035 W. Capitol Drive, Suite 108 Brookfield, WI 53045 Telephone: 414-708-1200

Project Engineer:
JAHNKE & JAHNKE ASSOCIATES LLC
Paul Jenswold, P.E.
711 W. Moreland Blvd.
Waukesha, WI 53188-2479
Telephone: 262-542-5797

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Email: pjenswold@jahnkeandjahnke.com

#### STORMWATER MANAGEMENT PLAN NARRATIVE

#### FOX RUN REDEVELOPMENT CITY OF WAUKESHA

#### Executive Summary:

A partnership of VJS Construction Services, Inc., Bedford Development LLC, and Somerstone LLC is proposing to redevelop the Fox Run Shopping Center. Under this plan, the strip mall, grocery store and restaurant are to be razed and the Chase Bank is to remain. The site is to be divided into 6 lots. A medical facility will possibly be located on Lot 1 (where Sentry is now), a future bank will possibly be located on Lot 3 (where Denny's is now), the Chase Bank will remain and 72-unit apartment complex is to be constructed on Lot 5. Lot 4 will be vacant for now and Outlot 6 will be used for storm water management. The parking lot and interior drive network have been reworked. The two existing access points for the property remain and a third right-in / right-out access point on Sunset Drive is being applied for from the County. This new access drive is located between the Lot 1 and the reservoir.

The proposed project is located in the southwest part of the City on the northwest corner of St. Paul Avenue and Sunset Drive and is part of the Southeast ¼ of Section 8 and the Southwest ¼ of Section 9, Township 6 North, Range 19 East. This project is located within the Middle Fox River Watershed.

This analysis will demonstrate that the site complies with the City's Storm Water Management Requirements.

#### Design Criteria:

This development is required to meet all the requirements found within the City of Waukesha's Stormwater Management and Erosion Control Ordinance (Chp. 23), and the Wisconsin Department of Natural Resources NR 151 and NR 216.

Peak Flow Reduction: The site was analyzed for peak flow reduction in which the 100-year, 10-year, 2-year and 1-year post-development runoff rates from site shall not exceed the corresponding pre-development rates.

Total Suspended Solids: Taxkey WAKC 1328999001 (11.637 acres 86.7% of the site) is classified as a redevelopment site. Taxkey WAKC 1328999002 (1.782 acres 13.3% of the site) is classified as new development. Therefore, the Total Suspended Solids (TSS) from the proposed paved areas must be reduced by 45.32% (blended rate) per Wisconsin NR 151 and the City's requirements.

Infiltration: This site is classified as High Imperviousness which has a requirement to infiltrate at least 60% of the predevelopment infiltration volume or use 2% of the site as the effective infiltration area or to the maximum extent practical.

#### **Existing Conditions:**

The existing site has a taxkey numbers of WAKC 1328999001 and 1328999002. The parcels add up to 13.3 acres and are zoned B-5. The existing soils on the site are predominantly Casco Loam (Type B), Colwood Silt Loam (Type C/D Hydric), Hochheim Loam (Type D), Loamy

Land (Type D), Martinton Silt Loam (Type C), Matherton Silt Loam (Type D) and Sebewa Silt Loam (Type D hydric) per the NRCS soil survey. See the soil map in the Appendix. Unfortunately, the main location of Casco Loam (Type B) soil is contaminated with PECs. See the Phase 2 Environmental Assessment.

#### <u>Post-Development Conditions:</u>

As mentioned above, the strip mall, grocery store and restaurant are to be razed and the Chase Bank is to remain. The site is to be divided into 6 lots. A medical facility will possibly be located on Lot 1 (where Sentry is now), a future bank will possibly be located on Lot 3 (where Denny's is now), the Chase Bank will remain and 72-unit apartment complex is to be constructed on Lot 5. Lot 4 will be vacant for now and Outlot 6 will be used for storm water management. The parking lot and interior drive network have been reworked. The two existing access points for the property remain and a third right-in / right-out access point on Sunset Drive is being applied for from the County. This new access drive is located between the Lot 1 and the reservoir.

The post-development conditions model uses blended CNs per soil type and percent impervious for each basin. This was done because the lot layouts are only conceptual at this point and an assumption had to be made for Lot 5 that has no interested buyer yet. Where possible, the conceptual impervious surface was measured and rounded. The percent impervious and blended CNs are as follows:

Basin	Percent Impervious
P1	65%/35%
P2	50%/50%
P3	80%/20%
P4	70%/30%
P5	80%/20%
P6	10%/90%

Blended CNs were created using 61, 71, and 78 respectively for lawn areas (B-D) and 98 for impervious areas regardless of the soil type. They are as follows:

Percent Impervious	Type B	Type C	Type D
10%/90%	65	74	89
50%/50%	85	87	88
65%/35%	85	89	91
70%/30%	87	90	92
80%/20%	91	93	94

#### **Analysis Methods**

HydroCAD (Version 10.00-22) software has been used to analyze stormwater characteristics for this stormwater management plan. HydroCAD uses the accepted TR-55 methodology for determining peak discharge runoff rates. Rainfall depths for the 1-year, 2-year, 10-year and 100-year storm events are 2.4", 2.7", 3.81 and 6.18" respectively, and were obtained from the City's Ordinance Chp. 32. Accordingly, the MSE 3 rainfall distribution curve was used for these models.

#### Peak Flow Analysis

The whole site and contributing off-site areas were analyzed for peak flow runoff rates in each basin both existing and proposed. The basin maps for the analysis can be found in the Appendix. The pre-development hydrologic parameters for the basins used in the model are as follows:

Pre-Development Hydrologic Parameters								
Subcatchment	Area, sf	Runoff CN	Impervious Area, sf	Time of Concentration, minutes				
E1	151,236	87	78,050	4.3				
E1A	69,403	82	15,519	8.1				
E2	228,279	97	221,388	6.0				
E2A	18,526	85	6,127	6.8				
E3	102,726	94	83,489	4.2				
E4	52,446	90	30,465	7.8				
E5	50,737	89	28,041	2.8				

The post-development hydrologic parameters for the basins used in the model are as follows:

Post-Development Hydrologic Parameters									
Subcatchment	Area, sf	Runoff CN	Impervious Percentage	Time of Concentration, minutes					
P1	382,431	90	65%	8.2					
P1A	86,183	83	25%	8.1					
P2	9,957	87	50%	3.2					
P3	98,621	94	80%	14.7					
P4	41,373	92	70%	6.8					
P5	28,163	94	80%	4.6					
P6	30,596	79	10%	7.4					

The results of this analysis are summarized below per discharge point from the site:

Pre / Post-Development Release Rate Comparison								
Discharge Point	Existing 1-yr Storm Q, cfs	Proposed 1-yr Storm Q, cfs	Existing 2-yr Storm Q, cfs	Proposed 2-yr Storm Q, cfs	Existing 10-yr Storm Q, cfs	Proposed 10-yr Storm Q, cfs	Existing 100-yr Storm Q, cfs	Proposed 100-yr Storm Q, cfs
24" Storm Sewer	18.19	1.17	20.69	2.98	29.92	18.37	49.52	46.92
Concrete Channel	10.12	0.91	12.27	1.16	20.50	2.18	38.52	4.55
Kohl's Pond	7.64	5.03	8.79	5.81	13.03	8.67	21.93	14.65
Highway Ditch	3.27	2.59	3.87	3.02	6.11	4.59	10.89	7.90
Highway Storm Sewer	2.90	2.05	3.42	2.36	5.35	3.50	9.46	5.90
TOTAL DISCHARGE with OFF- SITE	41.00	9.76	47.75	11.42	73.00	29.74	127.06	68.54

Note that the proposed release rates are lower for each storm respectively at all discharge points and the total discharge of the site including the off-site drainage that flows through the site.

It should be noted that the proposed site has less impervious surface than the existing site. Therefore, the bioretention pond is not necessary to meet the peak flow reduction criteria. It is needed to meet the water quality criteria.

#### Kohl's Pond

We will call the existing pond to the east the Kohl's pond because it was built when Kohl's was built, even though two other properties including Fox Run also discharge runoff to the pond. It should be noted that the 2001 Storm Water Management Plan for the pond incorrectly characterized the Fox Run property. The report states that 9.88 acres of Fox Run drain the culvert under St. Paul, of which 3.13 are captured by the new (Kohl's) pond. Our survey demonstrates that only 2.36 acres drain to the Kohl's pond (Basin E3) and that only 0.95 acre drain directly to the St. Paul culvert (Basin E4). The majority of the site drains to Pebble Creek either by the storm sewer or by the concrete channel.

The City staff has expressed skepticism about whether the Kohl's pond works as designed. While this analysis seems out of the scope of this project, what can be said is that this development will be sending less acreage to the Kohl's pond then it was designed to take and after redevelopment, the impervious surface will be significantly decreased as well.

#### **Runoff Water Quality**

A minimum of 45.32% (blended rate) TSS removal is required per NR 151 and the City's ordinance as discussed above. WinSLAMM (Version 10.3.4) was used for the analysis. By using the bio retention pond and the grassy swales, the reduction was achieved.

Sediment Load Reduction						
Area	Area (AC)	Sediment Load (LB)	Sediment Discharge (LB)	Sediment Reduction		
Modeled Area	11.19	4418	1526	65.46%		

This analysis was conducted for the entire site except for Basin P3, which drains to the Kohl's pond. For the purposes of this report, it is assumed that the Kohl's pond produces the TSS removal that it was designed to in the 2001 SWMP. For the sake of argument, the model was run including Basin P3 and the resulting sediment removal was 52.85%, which is still higher than the required removal rate.

The bottom line on the preliminary bio retention design is that it is oversized based on the City's storm water goals. The pond could be tightened up for the final design.

#### Infiltration

Most of the site has Type D soils, some with hydric components. There is one large area with Type B soils, however, that is where the contaminated soils are located. Therefore, this site is not suitable for infiltration. However, the bio retention pond will produce a small amount of infiltration.

#### Soils Investigation

Soil borings will be conducted for the final design of the storm water management. This preliminary SWMP relies solely on soil types for now.

#### Preserve Natural Topography

The site has been mass graded in the past. There is no natural topography to preserve.

#### <u>Installation Schedule of Stormwater Management Practices</u>

The bio retention pond without the engineered soil will be constructed first and the sides stabilized immediately. It will be used as a sediment basin during construction. After the areas upstream are substantially stabilized, the pond will be cleaned out and then engineered soils and plants installed. As the contractor grades the site, he will grade the grassy swales and immediately seed, install erosion mat and ditch checks.

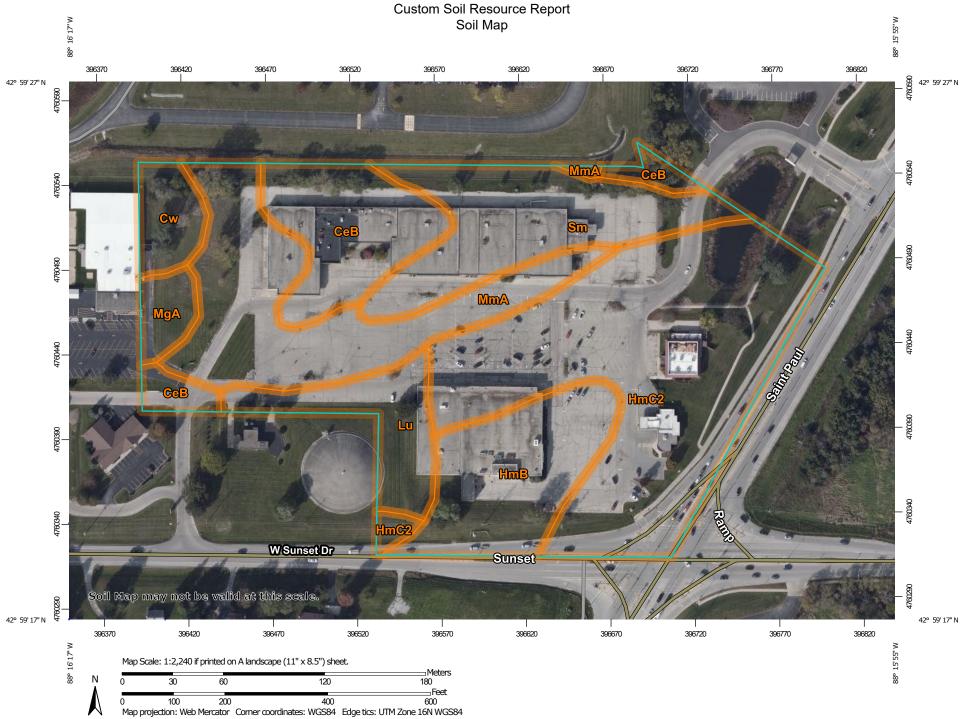
#### Maintenance Plan and Cost Estimate

The bio retention pond will cost about \$50,000 to construct. There will be the occasional wash out, silt build up, or shrub removal and plan replacement that will need to take place. The pond should be inspected on a yearly basis. It is estimated that one or more of these items will need to be address every 5 years at a present day cost of about \$2,000 for small repairs to \$20,000 for full replacement of engineered soil and plants.

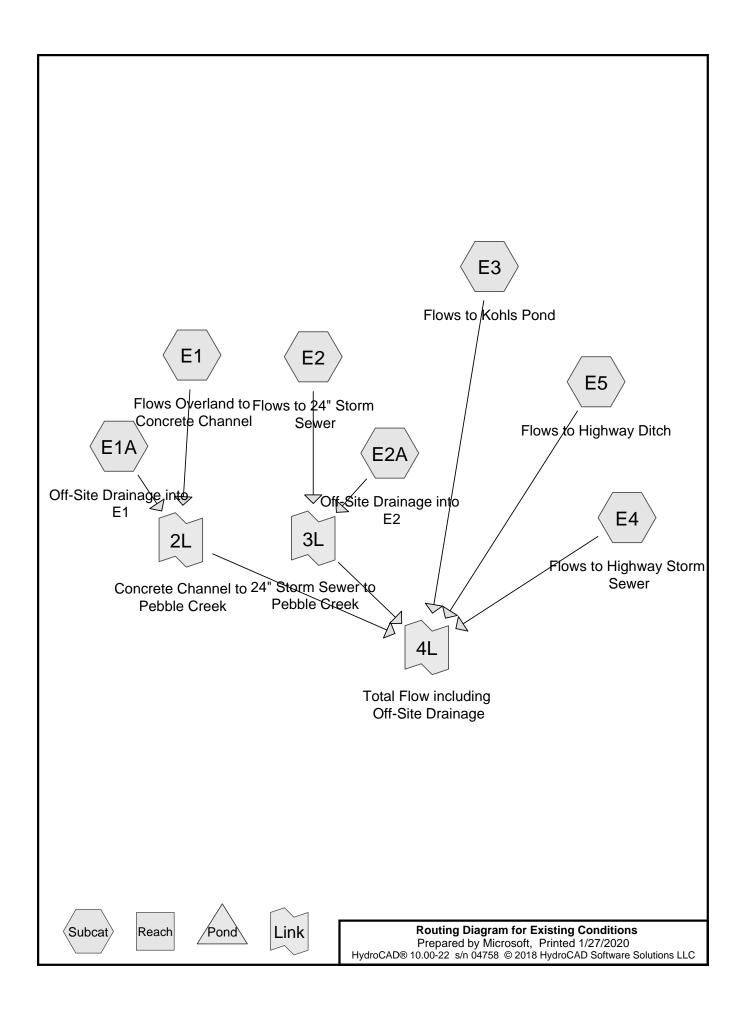
#### Conclusion

The storm water management features for the development have been designed to comply with the storm water goals set forth by the Wisconsin Department of Natural Resources and the City of Waukesha.

# APPENDIX A SOIL MAP



# APPENDIX B PRE-DEVELOPMENT FLOW DATA



Page 2

# **Summary for Subcatchment E1: Flows Overland to Concrete Channel**

Runoff = 8.13 cfs @ 12.11 hrs, Volume= 0.340 af, Depth> 1.18"

	Α	rea (sf)	CN [	Description							
		3,652	61 >	61 >75% Grass cover, Good, HSG B							
		20,009	98 F	Paved park	ing, HSG B						
		448	98 F	Roofs, HSG	βΒ̈́						
*		12,835	71 >	75% Gras	s cover, Go	ood, HSG C					
		867	98 F	Paved park	ing, HSG C						
*		56,699	78 >	75% Ġras	s cover, Go	ood, HSG D					
		56,268	98 F	Paved park	ing, HSG D						
		458	98 F	Roofs, HSC	G D						
	1	51,236	87 V	Veighted A	verage						
		73,186	4	l8.39% Pei	vious Area						
		78,050	5	51.61% lmp	pervious Are	ea					
	Tc	Length	Slope	Velocity	Capacity	Description					
<u>(r</u>	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	1.1	8	0.0400	0.12		Sheet Flow,					
						Grass: Short n= 0.150 P2= 2.70"					
	1.4	92	0.0150	1.11		Sheet Flow,					
						Smooth surfaces n= 0.011 P2= 2.70"					
	0.4	67	0.0150	2.49		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	0.6	170	0.0100	4.54	3.56	Pipe Channel,					
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'					
						n= 0.013					
	8.0	140	0.0400	3.00		Shallow Concentrated Flow,					
						Grassed Waterway Kv= 15.0 fps					
	4.3	477	Total								

# Summary for Subcatchment E1A: Off-Site Drainage into E1

Runoff = 2.43 cfs @ 12.16 hrs, Volume= 0.117 af, Depth> 0.88"

	Α	rea (sf)	CN	Description					
*		53,884	78	>75% Gras	s cover, Go	ood, HSG D			
		5,392	98	Paved park	ing, HSG D				
		10,127	98	Roofs, HSC	B Ď				
		69,403	82	Weighted Average					
		53,884		77.64% Pei	rvious Area				
		15,519		22.36% lmp	pervious Ar	ea			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	7.2	100	0.0540	0.23		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.70"			
	0.9	195	0.0540	3.49		Shallow Concentrated Flow,			
						Grassed Waterway Kv= 15.0 fps			
	8.1	295	Total						

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#### Summary for Subcatchment E2: Flows to 24" Storm Sewer

Runoff = 17.39 cfs @ 12.13 hrs, Volume= 0.873 af, Depth> 2.00"

	Α	rea (sf)	CN D	escription								
		1,552	61 >	>75% Grass cover, Good, HSG B								
		12,583	98 Paved parking, HSG B									
	29,163 98 Roofs, HSG B											
*		5,339	78 >	75% Gras	s cover, Go	ood, HSG D						
		90,863	98 F	aved park	ing, HSG D							
		88,779	98 F	Roofs, HSC	G Ď							
	2	28,279	97 V	Veighted A	verage							
		6,891	3	.02% Perv	ious Area							
	2	21,388	9	6.98% Imp	pervious Ar	ea						
	Tc	Length	Slope	Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	3.0	50	0.1200	0.28		Sheet Flow,						
						Grass: Short n= 0.150 P2= 2.70"						
	0.6	50	0.0400	1.45		Sheet Flow,						
						Smooth surfaces n= 0.011 P2= 2.70"						
	1.7	260	0.0150	2.49		Shallow Concentrated Flow,						
						Paved Kv= 20.3 fps						
	0.7	140	0.0050	3.21	2.52	Pipe Channel,						
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'						
						n= 0.013						
	6.0	500	Total	·	·							

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# **Summary for Subcatchment E2A: Off-Site Drainage into E2**

Runoff = 0.82 cfs @ 12.14 hrs, Volume= 0.037 af, Depth> 1.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-YR Rainfall=2.40"

	A	rea (sf)	CN I	Description			
*		12,399	78 :	>75% Gras	s cover, Go	ood, HSG D	
		6,127	98 I	Roofs, HSG	D D		
		18,526	85 \	<b>Neighted A</b>	verage		
		12,399	6	66.93% Per	vious Area		
		6,127	(	33.07% Imp	pervious Ar	ea	
	Tc	Length	Slope	,	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	6.8	90	0.0500	0.22		Sheet Flow,	

Grass: Short n= 0.150 P2= 2.70"

#### **Summary for Subcatchment E3: Flows to Kohls Pond**

Runoff = 7.64 cfs @ 12.10 hrs, Volume= 0.337 af, Depth> 1.71"

	Α	rea (sf)	CN [	Description							
		4,373	61 >	>75% Grass cover, Good, HSG B							
		1,521	98 F	Paved park	ing, HSG B						
*		14,864	78 >	75% Ġras	s cover, Go	ood, HSG D					
		74,374	98 F	Paved park	ing, HSG D						
		3,543	98 F	Roofs, HSG	G D						
_		4,051	98 V	Vater Surfa	ace, HSG D						
	1	02,726	94 V	Veighted A	verage						
	19,237 18.73% Pe				vious Area						
		83,489	8	31.27% Imp	pervious Ar	ea					
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	1.5	100	0.0150	1.13		Sheet Flow,					
						Smooth surfaces n= 0.011 P2= 2.70"					
	2.7	400	0.0150	2.49		Shallow Concentrated Flow,					
_						Paved Kv= 20.3 fps					
	4.2	500	Total								

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#### **Summary for Subcatchment E4: Flows to Highway Storm Sewer**

Runoff = 2.90 cfs @ 12.15 hrs, Volume= 0.139 af, Depth> 1.38"

	Δ	rea (sf)	CN [	Description					
*		21,981	78 >	75% Grass cover, Good, HSG D					
		30,299	98 F	Paved park	ing, HSG D				
		166	98 F	Roofs, HSG	G D				
		52,446	90 \	Veighted A	verage				
		21,981	4	11.91% Per	vious Area				
		30,465	5	58.09% Imp	ervious Ar	ea			
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.0	60	0.1660	0.33		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.70"			
	3.9	40	0.0400	0.17		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.70"			
	0.2	50	0.0500	3.35		Shallow Concentrated Flow,			
						Grassed Waterway Kv= 15.0 fps			
	0.7	320	0.1500	7.86		Shallow Concentrated Flow,			
_						Paved Kv= 20.3 fps			
	7.8	470	Total						

# **Summary for Subcatchment E5: Flows to Highway Ditch**

Runoff = 3.27 cfs @ 12.09 hrs, Volume= 0.127 af, Depth> 1.31"

_	Δ	rea (sf)	CN E	Description								
,	+	22,696	78 >	75% Gras	s cover, Go	ood, HSG D						
		21,802	98 F	Paved park	ved parking, HSG D ofs, HSG D							
_		6,239	98 F	Roofs, HSC								
		50,737	89 V	Veighted A	verage							
		22,696	4	4.73% Pei	rvious Area							
		28,041 55.27% Impervious Area										
	Tc	Length	Slope	Velocity	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	1.5	100	0.0150	1.13		Sheet Flow,						
						Smooth surfaces n= 0.011 P2= 2.70"						
	1.1	160	0.0150	2.49		Shallow Concentrated Flow,						
						Paved Kv= 20.3 fps						
	0.2	45	0.0500	3.35		Shallow Concentrated Flow,						
_						Grassed Waterway Kv= 15.0 fps						
	2.8	305	Total									

MSE 24-hr 3 1-YR Rainfall=2.40" Printed 1/27/2020

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# **Summary for Link 2L: Concrete Channel to Pebble Creek**

Inflow Area = 5.065 ac, 42.41% Impervious, Inflow Depth > 1.08" for 1-YR event

Inflow = 10.12 cfs @ 12.12 hrs, Volume= 0.457 af

Primary = 10.12 cfs @ 12.12 hrs, Volume= 0.457 af, Atten= 0%, Lag= 0.0 min

MSE 24-hr 3 1-YR Rainfall=2.40" Printed 1/27/2020

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#### **Summary for Link 3L: 24" Storm Sewer to Pebble Creek**

Inflow Area = 5.666 ac, 92.18% Impervious, Inflow Depth > 1.93" for 1-YR event

Inflow = 18.19 cfs @ 12.13 hrs, Volume= 0.910 af

Primary = 18.19 cfs @ 12.13 hrs, Volume= 0.910 af, Atten= 0%, Lag= 0.0 min

MSE 24-hr 3 1-YR Rainfall=2.40" Printed 1/27/2020

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# Summary for Link 4L: Total Flow including Off-Site Drainage

Inflow Area = 15.458 ac, 68.77% Impervious, Inflow Depth > 1.53" for 1-YR event

Inflow = 41.00 cfs @ 12.12 hrs, Volume= 1.970 af

Primary = 41.00 cfs @ 12.12 hrs, Volume= 1.970 af, Atten= 0%, Lag= 0.0 min

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# **Summary for Subcatchment E1: Flows Overland to Concrete Channel**

Runoff = 9.77 cfs @ 12.11 hrs, Volume= 0.411 af, Depth> 1.42"

	Α	rea (sf)	CN [	Description						
		3,652	61 >	75% Gras	s cover, Go	ood, HSG B				
		20,009	98 F	Paved park	ing, HSG B					
		448	98 F	Roofs, HSG	βΒ̈́					
*		12,835	71 >	75% Gras	s cover, Go	ood, HSG C				
		867	98 F	98 Paved parking, HSG C						
*		56,699	78 >	78 >75% Grass cover, Good, HSG D						
		56,268	98 F	98 Paved parking, HSG D						
		458	98 F	8 Roofs, HSG D						
	1									
	151,236 87 Weighted Average 73,186 48.39% Pervious Area									
		78,050	5	51.61% lmp	pervious Are	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
<u>(r</u>	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	1.1	8	0.0400	0.12		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.70"				
	1.4	92	0.0150	1.11		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 2.70"				
	0.4	67	0.0150	2.49		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	0.6	170	0.0100	4.54	3.56	Pipe Channel,				
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'				
						n= 0.013				
	8.0	140	0.0400	3.00		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	4.3	477	Total							

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# Summary for Subcatchment E1A: Off-Site Drainage into E1

Runoff 3.03 cfs @ 12.16 hrs, Volume= 0.145 af, Depth> 1.09" =

_	Д	rea (sf)	CN I	Description								
*		53,884	78 :	78 >75% Grass cover, Good, HSG D								
		5,392	98	Paved park	ved parking, HSG D							
		10,127			ofs, HSG D							
		69,403	82 \	Weighted A	verage							
		53,884 15,519		22.36% lmp	pervious Ar	ea						
	Tc	Length	Slope	Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·						
	7.2	100	0.0540	0.23		Sheet Flow,						
						Grass: Short n= 0.150 P2= 2.70"						
	0.9	195	0.0540	3.49		Shallow Concentrated Flow,						
						Grassed Waterway Kv= 15.0 fps						
	8.1	295	Total			<u> </u>						

# Summary for Subcatchment E2: Flows to 24" Storm Sewer

Runoff = 19.71 cfs @ 12.13 hrs, Volume= 0.998 af, Depth> 2.29"

	A	rea (sf)	CN [	Description					
		1,552	61 >	75% Gras	s cover, Go	ood, HSG B			
		12,583	98 F	Paved park	ing, HSG B				
		29,163	98 F	Roofs, HSG	βB				
*		5,339	78 >	75% Gras	s cover, Go	ood, HSG D			
		90,863	98 F	Paved park	ing, HSG D				
		88,779	98 F	Roofs, HSC	S Ď				
	2	28,279	97 V	Veighted A	verage				
	6,891 3.02% Pervious Are								
	2	21,388	ç	6.98% Imp	mpervious Area				
	Тс	Length	Slope	Velocity	Capacity	Description			
(m	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
(	3.0	50	0.1200	0.28		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.70"			
(	0.6	50	0.0400	1.45		Sheet Flow,			
						Smooth surfaces n= 0.011 P2= 2.70"			
•	1.7	260	0.0150	2.49		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
(	).7	140	0.0050	3.21	2.52	Pipe Channel,			
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'			
						n= 0.013			
6	6.0	500	Total						

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# Summary for Subcatchment E2A: Off-Site Drainage into E2

Runoff = 1.00 cfs @ 12.14 hrs, Volume= 0.045 af, Depth> 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-YR Rainfall=2.70"

	Α	rea (sf)	CN I	Description			
*		12,399	78 :	>75% Gras	s cover, Go	od, HSG D	
_		6,127	98	Roofs, HSG	B D		
	18,526 85 Weighted Average						
		12,399		66.93% Pei	vious Area		
		6,127	;	33.07% Imp	pervious Ar	ea	
	Tc	Length	Slope	,	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	6.8	90	0.0500	0.22		Sheet Flow,	

Grass: Short n= 0.150 P2= 2.70"

# Summary for Subcatchment E3: Flows to Kohls Pond

Runoff = 8.79 cfs @ 12.10 hrs, Volume= 0.392 af, Depth> 1.99"

	Α	rea (sf)	CN [	Description						
		4,373	61 >	75% Gras	s cover, Go	ood, HSG B				
		1,521	98 F	Paved park	ing, HSG B					
*		14,864	78 >	75% Ġras	s cover, Go	ood, HSG D				
		74,374	98 F	Paved park	ing, HSG D					
		3,543	98 F	Roofs, HSG	G D					
_		4,051	98 V	Vater Surfa	ace, HSG D					
	102,726 94 Weighted Average									
	19,237			18.73% Pervious Area						
		83,489	8	31.27% Imp	pervious Ar	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	1.5	100	0.0150	1.13		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 2.70"				
	2.7	400	0.0150	2.49		Shallow Concentrated Flow,				
_						Paved Kv= 20.3 fps				
	4.2	500	Total							

# **Summary for Subcatchment E4: Flows to Highway Storm Sewer**

Runoff = 3.42 cfs @ 12.15 hrs, Volume= 0.165 af, Depth> 1.64"

	Д	rea (sf)	CN I	Description		
*		21,981	78 :	>75% Gras	s cover, Go	ood, HSG D
		30,299		Paved park		
		166	98 I	Roofs, HSG	S D	
		52,446	90 \	Neighted A	verage	
		21,981	4	41.91% Pei	vious Area	
		30,465	ļ	58.09% lmp	ervious Ar	ea
	,					
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	3.0	60	0.1660	0.33		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	3.9	40	0.0400	0.17		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	0.2	50	0.0500	3.35		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
	0.7	320	0.1500	7.86		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	7.8	470	Total			

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#### **Summary for Subcatchment E5: Flows to Highway Ditch**

Runoff = 3.87 cfs @ 12.09 hrs, Volume= 0.152 af, Depth> 1.57"

_	Α	rea (sf)	CN [	Description							
*	,	22,696	78 >	, ,							
		21,802	98 F	1 3'							
_		6,239	98 F	98 Roofs, HSG D							
		50,737	89 V	<b>Neighted A</b>	verage						
		22,696	4	14.73% Pei	rvious Area						
		28,041	5	55.27% lmp	pervious Ar	ea					
	T 1 11 01 1/1 1/2 0 1/2				_						
	Tc	Length	Slope		Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	1.5	100	0.0150	1.13		Sheet Flow,					
						Smooth surfaces n= 0.011 P2= 2.70"					
	1.1	160	0.0150	2.49		Shallow Concentrated Flow,					
						Paved Kv= 20.3 fps					
	0.2	45	0.0500	3.35		Shallow Concentrated Flow,					
_						Grassed Waterway Kv= 15.0 fps					
	2.8	305	Total								

MSE 24-hr 3 2-YR Rainfall=2.70" Printed 1/27/2020

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#### **Summary for Link 2L: Concrete Channel to Pebble Creek**

Inflow Area = 5.065 ac, 42.41% Impervious, Inflow Depth > 1.32" for 2-YR event

Inflow = 12.27 cfs @ 12.12 hrs, Volume= 0.556 af

Primary = 12.27 cfs @ 12.12 hrs, Volume= 0.556 af, Atten= 0%, Lag= 0.0 min

MSE 24-hr 3 2-YR Rainfall=2.70" Printed 1/27/2020

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# **Summary for Link 3L: 24" Storm Sewer to Pebble Creek**

Inflow Area = 5.666 ac, 92.18% Impervious, Inflow Depth > 2.21" for 2-YR event

Inflow = 20.69 cfs @ 12.13 hrs, Volume= 1.044 af

Primary = 20.69 cfs @ 12.13 hrs, Volume= 1.044 af, Atten= 0%, Lag= 0.0 min

MSE 24-hr 3 2-YR Rainfall=2.70" Printed 1/27/2020

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# Summary for Link 4L: Total Flow including Off-Site Drainage

Inflow Area = 15.458 ac, 68.77% Impervious, Inflow Depth > 1.79" for 2-YR event

Inflow = 47.75 cfs @ 12.12 hrs, Volume= 2.308 af

Primary = 47.75 cfs @ 12.12 hrs, Volume= 2.308 af, Atten= 0%, Lag= 0.0 min

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# **Summary for Subcatchment E1: Flows Overland to Concrete Channel**

Runoff = 15.97 cfs @ 12.11 hrs, Volume= 0.686 af, Depth> 2.37"

	Α	rea (sf)	CN [	Description						
		3,652	61 >	75% Gras	s cover, Go	ood, HSG B				
		20,009	98 F	Paved park	ing, HSG B					
		448	98 F	Roofs, HSG	βΒ̈́					
*		12,835	71 >	75% Gras	s cover, Go	ood, HSG C				
		867	98 F	98 Paved parking, HSG C						
*		56,699	78 >	78 >75% Grass cover, Good, HSG D						
		56,268	98 F	98 Paved parking, HSG D						
		458	98 F	8 Roofs, HSG D						
	1									
	151,236 87 Weighted Average 73,186 48.39% Pervious Area									
		78,050	5	51.61% lmp	pervious Are	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
<u>(r</u>	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	1.1	8	0.0400	0.12		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.70"				
	1.4	92	0.0150	1.11		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 2.70"				
	0.4	67	0.0150	2.49		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	0.6	170	0.0100	4.54	3.56	Pipe Channel,				
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'				
						n= 0.013				
	8.0	140	0.0400	3.00		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	4.3	477	Total							

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# Summary for Subcatchment E1A: Off-Site Drainage into E1

Runoff = 5.39 cfs @ 12.15 hrs, Volume= 0.260 af, Depth> 1.95"

_	Α	rea (sf)	CN	Description		
*		53,884	78	>75% Gras	s cover, Go	ood, HSG D
		5,392	98	Paved park	ing, HSG D	
_		10,127	98	Roofs, HSC	B Ď	
		69,403	82	Weighted A	verage	
		53,884	•	77.64% Pei	rvious Area	
		15,519		22.36% lmp	pervious Ar	ea
	Тс	Length	Slope	,	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.2	100	0.0540	0.23		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	0.9	195	0.0540	3.49		Shallow Concentrated Flow,
_						Grassed Waterway Kv= 15.0 fps
	8.1	295	Total	·		

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# Summary for Subcatchment E2: Flows to 24" Storm Sewer

Runoff = 28.26 cfs @ 12.13 hrs, Volume= 1.461 af, Depth> 3.35"

	Α	rea (sf)	CN D	escription						
		1,552	61 >	75% Gras	s cover, Go	ood, HSG B				
		12,583	98 F	aved park	ing, HSG B					
	29,163 98 Roofs, HSG B									
*		5,339 78 >75% Grass cover, Good, HSG D								
		90,863 98 Paved parking, HSG D								
	2	28,279	97 V	Veighted A	verage					
		6,891	3	3.02% Pervious Area						
	221,388			96.98% Impervious Area						
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	3.0	50	0.1200	0.28		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.70"				
	0.6	50	0.0400	1.45		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 2.70"				
	1.7	260	0.0150	2.49		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	0.7	140	0.0050	3.21	2.52	Pipe Channel,				
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'				
						n= 0.013				
	6.0	500	Total	·	·					

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# **Summary for Subcatchment E2A: Off-Site Drainage into E2**

Runoff = 1.68 cfs @ 12.14 hrs, Volume= 0.078 af, Depth> 2.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10-YR Rainfall=3.81"

	Α	rea (sf)	CN I	Description			
*		12,399	78 :	>75% Gras	s cover, Go	ood, HSG D	
		6,127	98	Roofs, HSG	B D		_
	18,526 85 Weighted Average						
		12,399		66.93% Pei	vious Area		
	6,127 33.07% Impervious Are					ea	
	Тс	Length	Slope	,	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_
	6.8	90	0.0500	0.22		Sheet Flow,	

Grass: Short n= 0.150 P2= 2.70"

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# Summary for Subcatchment E3: Flows to Kohls Pond

Runoff = 13.03 cfs @ 12.10 hrs, Volume= 0.597 af, Depth> 3.04"

	Α	rea (sf)	CN E	Description					
		4,373	61 >	>75% Grass cover, Good, HSG B					
	1,521		98 F	Paved parking, HSG B					
*	* 14,864 78 >75% Grass cover, Good, HSG D								
74,374 98 Paved parking, HSG D									
		3,543	98 F	Roofs, HSG D					
		4,051 98 Water Surface, HSG D							
	1	02,726	94 V	Weighted Average					
	19,237		1	18.73% Pervious Area					
	83,489 81.27% Impervious Are				pervious Are	ea			
				_					
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	1.5	100	0.0150	1.13		Sheet Flow,			
						Smooth surfaces n= 0.011 P2= 2.70"			
	2.7	400	0.0150	2.49		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	4.2	500	Total						

#### **Summary for Subcatchment E4: Flows to Highway Storm Sewer**

Runoff = 5.35 cfs @ 12.15 hrs, Volume= 0.265 af, Depth> 2.64"

	Α	rea (sf)	CN I	Description						
*	21,981 78 >75% Grass cover, Good, HSG D									
		30,299								
		21,981	4	41.91% Pei	vious Area					
30,465 58.09% Impervious Area										
								Tc	Length	Slope
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	3.0	60	0.1660	0.33		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.70"				
	3.9	40	0.0400	0.17		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.70"				
	0.2	50	0.0500	3.35		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	0.7	320	0.1500	7.86		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	7.8	470	Total							

# **Summary for Subcatchment E5: Flows to Highway Ditch**

Runoff 6.11 cfs @ 12.09 hrs, Volume= 0.248 af, Depth> 2.55" =

			O							
_	A	rea (sf)	CN [	N Description						
*		22,696	78 >	3 >75% Grass cover, Good, HSG D						
		21,802	98 F	Paved park	ing, HSG D					
		6,239		Roofs, HSG D						
		50,737	89 V	Veighted A	verage					
	22,696 44.73% Pervious Area									
		28,041	5	55.27% lmr	pervious Ar	ea				
		,								
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·				
	1.5	100	0.0150	1.13		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 2.70"				
	1.1	160	0.0150	2.49		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	0.2	45	0.0500	3.35		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	2.8	305	Total			·				

MSE 24-hr 3 10-YR Rainfall=3.81" Printed 1/27/2020

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# **Summary for Link 2L: Concrete Channel to Pebble Creek**

Inflow Area = 5.065 ac, 42.41% Impervious, Inflow Depth > 2.24" for 10-YR event

Inflow = 20.50 cfs @ 12.11 hrs, Volume= 0.946 af

Primary = 20.50 cfs @ 12.11 hrs, Volume= 0.946 af, Atten= 0%, Lag= 0.0 min

MSE 24-hr 3 10-YR Rainfall=3.81"

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#### **Summary for Link 3L: 24" Storm Sewer to Pebble Creek**

Inflow Area = 5.666 ac, 92.18% Impervious, Inflow Depth > 3.26" for 10-YR event

Inflow = 29.92 cfs @ 12.13 hrs, Volume= 1.539 af

Primary = 29.92 cfs @ 12.13 hrs, Volume= 1.539 af, Atten= 0%, Lag= 0.0 min

MSE 24-hr 3 10-YR Rainfall=3.81"

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## **Summary for Link 4L: Total Flow including Off-Site Drainage**

Inflow Area = 15.458 ac, 68.77% Impervious, Inflow Depth > 2.79" for 10-YR event

Inflow = 73.00 cfs @ 12.11 hrs, Volume= 3.595 af

Primary = 73.00 cfs @ 12.11 hrs, Volume= 3.595 af, Atten= 0%, Lag= 0.0 min

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# **Summary for Subcatchment E1: Flows Overland to Concrete Channel**

Runoff = 29.37 cfs @ 12.10 hrs, Volume= 1.311 af, Depth> 4.53"

	Α	rea (sf)	CN [	Description						
		3,652	61 >	75% Gras	s cover, Go	ood, HSG B				
		20,009	98 F	Paved parking, HSG B						
		448	98 F	Roofs, HSG B						
*		12,835	71 >	>75% Grass cover, Good, HSG C						
		867	98 F	Paved park	ing, HSG C					
*		56,699	78 >	75% Ġras	s cover, Go	ood, HSG D				
		56,268	98 F	Paved park	ing, HSG D					
		458	98 F	Roofs, HSC	G D					
	1	51,236	87 V	Veighted A	verage					
		73,186	4	l8.39% Pei	vious Area					
78,050 51.61% Impervious Area										
	Tc	Length	Slope	Velocity	Capacity	Description				
<u>(r</u>	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	1.1	8	0.0400	0.12		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.70"				
	1.4	92	0.0150	1.11		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 2.70"				
	0.4	67	0.0150	2.49		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	0.6	170	0.0100	4.54	3.56	Pipe Channel,				
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'				
						n= 0.013				
	8.0	140	0.0400	3.00		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	4.3	477	Total							

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# Summary for Subcatchment E1A: Off-Site Drainage into E1

Runoff = 10.72 cfs @ 12.15 hrs, Volume= 0.531 af, Depth> 4.00"

_	Д	rea (sf)	CN I	Description								
*		53,884	78 :	>75% Gras	75% Grass cover, Good, HSG D							
		5,392	98	Paved park	ing, HSG D	)						
		10,127			Roofs, HSG D							
		69,403	82 \	Veighted Average								
		53,884		77.64% Pervious Area								
		15,519		22.36% Impervious Area								
	Tc	Length	Slope	Velocity	Capacity	Description						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·						
	7.2	100	0.0540	0.23		Sheet Flow,						
						Grass: Short n= 0.150 P2= 2.70"						
	0.9	195	0.0540	3.49		Shallow Concentrated Flow,						
						Grassed Waterway Kv= 15.0 fps						
	8.1	295	Total			<u> </u>						

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# Summary for Subcatchment E2: Flows to 24" Storm Sewer

Runoff = 46.36 cfs @ 12.13 hrs, Volume= 2.447 af, Depth> 5.60"

	Α	rea (sf)	CN D	escription			
		1,552	61 >	75% Gras	s cover, Go	ood, HSG B	
		12,583	98 F	aved park	ing, HSG B		
	29,163 98 Roofs, HSG B						
*		5,339	78 >	75% Gras	s cover, Go	ood, HSG D	
		90,863	98 F	aved park	ing, HSG D		
		88,779	98 F	Roofs, HSC	G Ď		
	2	28,279	97 V	Veighted A	verage		
		6,891	3	.02% Perv	ious Area		
	221,388 96.98% Impe					ea	
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	3.0	50	0.1200	0.28		Sheet Flow,	
						Grass: Short n= 0.150 P2= 2.70"	
	0.6	50	0.0400	1.45		Sheet Flow,	
						Smooth surfaces n= 0.011 P2= 2.70"	
	1.7	260	0.0150	2.49		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
	0.7	140	0.0050	3.21	2.52	Pipe Channel,	
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'	
						n= 0.013	
	6.0	500	Total	·	·		

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# Summary for Subcatchment E2A: Off-Site Drainage into E2

Runoff = 3.19 cfs @ 12.14 hrs, Volume= 0.153 af, Depth> 4.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-YR Rainfall=6.18"

	Α	rea (sf)	CN I	Description						
*		12,399	78 :	>75% Gras	s cover, Go	ood, HSG D				
		6,127	98	Roofs, HSG	B D			_		
		18,526	85	Neighted A	verage					
		12,399		66.93% Pei	vious Area					
		6,127	;	33.07% Imp	pervious Ar	ea				
	Тс	Length	Slope	,	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			_		
	6.8	90	0.0500	0.22		Sheet Flow,				

Grass: Short n= 0.150 P2= 2.70"

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# Summary for Subcatchment E3: Flows to Kohls Pond

Runoff = 21.93 cfs @ 12.10 hrs, Volume= 1.042 af, Depth> 5.30"

	Α	rea (sf)	CN [	Description						
		4,373	61 >	75% Gras	s cover, Go	ood, HSG B				
1,521 98 Paved parking, HSG B										
*		14,864	78 >	>75% Grass cover, Good, HSG D						
		74,374	98 F	Paved parking, HSG D						
		3,543	98 F	Roofs, HSG D						
_		4,051	98 V	Water Surface, HSG D						
	102,726 94 Weighted Average									
	19,237 18.73% Pervious Area				vious Area					
		83,489	8	31.27% Imp	pervious Ar	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	1.5	100	0.0150	1.13		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 2.70"				
	2.7	400	0.0150	2.49		Shallow Concentrated Flow,				
_						Paved Kv= 20.3 fps				
	4.2	500	Total							

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# **Summary for Subcatchment E4: Flows to Highway Storm Sewer**

Runoff = 9.46 cfs @ 12.15 hrs, Volume= 0.487 af, Depth> 4.86"

	Α	rea (sf)	CN [	Description						
*		21,981	78 >	>75% Grass cover, Good, HSG D						
		30,299	98 F	Paved parking, HSG D						
		166		Roofs, HSG D						
		52,446	90 V	Veighted Average						
		21,981		41.91% Pervious Area						
		30,465	5	8.09% Imp	ervious Ar	ea				
		,								
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·				
	3.0	60	0.1660	0.33		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.70"				
	3.9	40	0.0400	0.17		Sheet Flow,				
						Grass: Short n= 0.150 P2= 2.70"				
	0.2	50	0.0500	3.35		Shallow Concentrated Flow,				
						Grassed Waterway Kv= 15.0 fps				
	0.7	320	0.1500	7.86		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	7.8	470	Total							

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#### **Summary for Subcatchment E5: Flows to Highway Ditch**

Runoff = 10.89 cfs @ 12.09 hrs, Volume= 0.461 af, Depth> 4.75"

_	Δ	rea (sf)	CN E	Description						
,	+	22,696	78 >	75% Grass cover, Good, HSG D						
		21,802	98 F	Paved parking, HSG D						
_		6,239	98 F	Roofs, HSG D						
		50,737	89 V	Veighted A	verage					
		22,696	4	44.73% Pervious Area						
		28,041	5	5.27% lmp	pervious Ar	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	1.5	100	0.0150	1.13		Sheet Flow,				
						Smooth surfaces n= 0.011 P2= 2.70"				
	1.1	160	0.0150	2.49		Shallow Concentrated Flow,				
						Paved Kv= 20.3 fps				
	0.2	45	0.0500	3.35		Shallow Concentrated Flow,				
_						Grassed Waterway Kv= 15.0 fps				
	2.8	305	Total							

MSE 24-hr 3 100-YR Rainfall=6.18" Printed 1/27/2020

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# **Summary for Link 2L: Concrete Channel to Pebble Creek**

Inflow Area = 5.065 ac, 42.41% Impervious, Inflow Depth > 4.36" for 100-YR event

Inflow = 38.52 cfs @ 12.11 hrs, Volume= 1.842 af

Primary = 38.52 cfs @ 12.11 hrs, Volume= 1.842 af, Atten= 0%, Lag= 0.0 min

MSE 24-hr 3 100-YR Rainfall=6.18" Printed 1/27/2020

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# **Summary for Link 3L: 24" Storm Sewer to Pebble Creek**

Inflow Area = 5.666 ac, 92.18% Impervious, Inflow Depth > 5.51" for 100-YR event

Inflow = 49.52 cfs @ 12.13 hrs, Volume= 2.600 af

Primary = 49.52 cfs @ 12.13 hrs, Volume= 2.600 af, Atten= 0%, Lag= 0.0 min

# **Existing Conditions**Prepared by Microsoft

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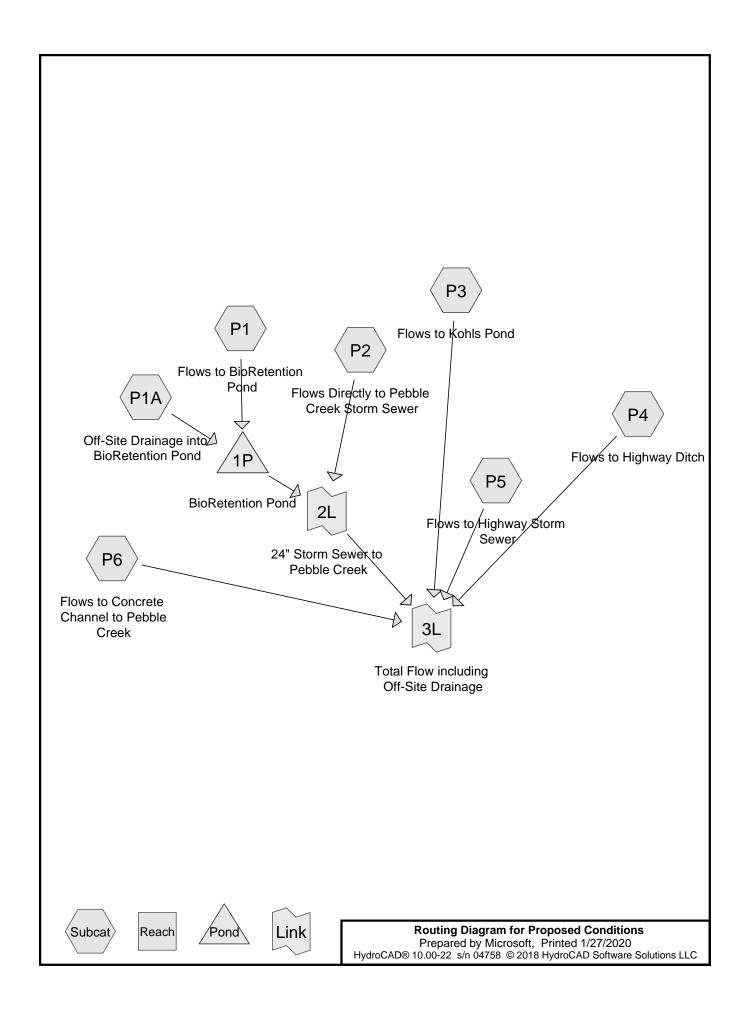
## Summary for Link 4L: Total Flow including Off-Site Drainage

Inflow Area = 15.458 ac, 68.77% Impervious, Inflow Depth > 4.99" for 100-YR event

Inflow = 127.06 cfs @ 12.11 hrs, Volume= 6.433 af

Primary = 127.06 cfs @ 12.11 hrs, Volume= 6.433 af, Atten= 0%, Lag= 0.0 min

# APPENDIX C POST-DEVELOPMENT FLOW DATA



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# Summary for Subcatchment P1: Flows to BioRetention Pond

Runoff = 20.77 cfs @ 12.15 hrs, Volume= 1.013 af, Depth> 1.38"

	Α	rea (sf)	CN [	Description		
*		52,689	85 7	ype B 65%	, 0	
*		10,870	89 7	ype C 65%	, 0	
*	3	18,872	91 7	Type D 65%	, 0	
	3	82,431	90 V	Veighted A	verage	
	382,431 100.00% Pervious Ai					a
	_		01		•	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.1	40	0.0200	0.13		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	1.0	60	0.0150	1.02		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.70"
	2.1	480	0.0050	3.72	4.57	Pipe Channel,
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013
	8.2	580	Total			

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# Summary for Subcatchment P1A: Off-Site Drainage into BioRetention Pond

Runoff = 3.21 cfs @ 12.16 hrs, Volume= 0.154 af, Depth> 0.93"

_	Α	rea (sf)	CN I	Description						
*		86,183	83 I	From Existi	rom Existing Conditions Model					
	86,183 100.00% Pervious A					a				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	7.2	100	0.0540	0.23		Sheet Flow, Grass: Short n= 0.150 P2= 2.70"				
	0.9	195	0.0540	3.49		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps				
	8.1	295	Total			•				

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# Summary for Subcatchment P2: Flows Directly to Pebble Creek Storm Sewer

Runoff = 0.57 cfs @ 12.10 hrs, Volume= 0.022 af, Depth> 1.18"

	,	Area (sf)	CN	Description					
*		4,051	85	Type B 50%	, o				
*		5,906	88	Type D 50%					
_		9,957 9,957	87	Weighted Average 100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description			
	2.8	30	0.0500	0.18		Sheet Flow,			
	0.4	40	0.050	0 1.52		Grass: Short n= 0.150 P2= 2.70"  Sheet Flow, Smooth surfaces n= 0.011 P2= 2.70"			
	3.2	70	Total						

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# Summary for Subcatchment P3: Flows to Kohls Pond

Runoff = 5.03 cfs @ 12.23 hrs, Volume= 0.323 af, Depth> 1.71"

	Α	rea (sf)	CN [	Description		
*		5,889	91 -	Гуре В 80%	, 0	
*		92,732	94	Type D 80%	<b>,</b>	
		98,621 98,621		Weighted A 100.00% Pe	verage ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	10.7	100	0.0200	0.16		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.70"
	3.1	280	0.0100	1.50		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps
	0.9	270	0.0050	5.09	16.00	<b>Pipe Channel,</b> 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
	14.7	650	Total	·	·	

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# **Summary for Subcatchment P4: Flows to Highway Ditch**

Runoff = 2.59 cfs @ 12.14 hrs, Volume= 0.122 af, Depth> 1.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-YR Rainfall=2.40"

	Α	rea (sf)	CN E	Description			
*		41,373	92 1	ype D 70%	6		
		41,373	1	00.00% Pe	ervious Are	a	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	6.8	95	0.0550	0.23		Sheet Flow,	

Grass: Short n= 0.150 P2= 2.70"

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# **Summary for Subcatchment P5: Flows to Highway Storm Sewer**

Runoff = 2.05 cfs @ 12.11 hrs, Volume= 0.092 af, Depth> 1.71"

	Α	rea (sf)	CN	Description		
*		28,163	94	Type D 80%	6	
		28,163		100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	2.2	20	0.0400	0.15		Sheet Flow,
	1.2	80	0.0150	1.08		Grass: Short n= 0.150 P2= 2.70"  Sheet Flow,
	1.2	180	0.0150	2.49		Smooth surfaces n= 0.011 P2= 2.70"  Shallow Concentrated Flow, Paved Kv= 20.3 fps
_	4.6	280	Total			

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#### **Summary for Subcatchment P6: Flows to Concrete Channel to Pebble Creek**

Runoff = 0.91 cfs @ 12.15 hrs, Volume= 0.043 af, Depth> 0.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-YR Rainfall=2.40"

	Α	rea (sf)	CN I	Description			
*		10,693	65	Type B 10%	, 0		
*		2,828	74	Type C 10%	, 0		
*		17,075	89	Type D 10%	, 0		
		30,596	79 \	Weighted A	verage		
	30,596 100.00% Pervious Area					a	
	Tc	Length	Slope	,	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.4	100	0.0500	0.23		Sheet Flow,	

Grass: Short n= 0.150 P2= 2.70"

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#### **Summary for Pond 1P: BioRetention Pond**

Inflow Area = 10.758 ac, 0.00% Impervious, Inflow Depth > 1.30" for 1-YR event

23.98 cfs @ 12.15 hrs. Volume= Inflow 1.167 af

1.53 cfs @ 13.39 hrs, Volume= Outflow 0.443 af, Atten= 94%, Lag= 74.2 min

0.39 cfs @ 13.39 hrs, Volume= Discarded = 0.289 af Primary 1.14 cfs @ 13.39 hrs, Volume= 0.154 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 28.59' @ 13.39 hrs Surf.Area= 23,240 sf Storage= 34,416 cf

Plug-Flow detention time= 186.0 min calculated for 0.441 af (38% of inflow)

Center-of-Mass det. time= 118.0 min (898.6 - 780.6)

Volume	Inv	vert Ava	ail.Storage	Storage D	Description	
#1	27.	00'	82,838 cf	Custom S	Stage Data (Pr	rismatic)Listed below (Recalc)
Elevation	on	Surf.Area	Inc	.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubi	c-feet)	(cubic-feet)	
27.0	00	20,018		0	0	
29.0	00	24,068	4	14,086	44,086	
30.0	00	26,457	2	25,263	69,349	
30.5	50	27,500	•	13,489	82,838	
Device	Routing	lr	nvert Outl	et Devices		
#1	Primary	2	5.00' <b>24.0</b>	" Round (	Culvert	
			I = 2	00 0' CPF	Square edge	headwall Ke= 0.500

#1	Primary	25.00'	24.0" Round Culvert
			L= 200.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 25.00' / 24.00' S= 0.0050 '/' Cc= 0.900
			n= 0.013, Flow Area= 3.14 sf
#2	Device 1	28.50'	48.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Primary	29.80'	Asymmetrical Weir, C= 3.27
			Offset (feet) -100.00 0.00 100.00
			Height (feet) 1.00 0.00 1.00
#4	Discarded	27.00'	0.600 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 20.00'

**Discarded OutFlow** Max=0.39 cfs @ 13.39 hrs HW=28.59' (Free Discharge) **4=Exfiltration** (Controls 0.39 cfs)

Primary OutFlow Max=1.13 cfs @ 13.39 hrs HW=28.59' (Free Discharge)

-1=Culvert (Passes 1.13 cfs of 20.31 cfs potential flow)
-2=Orifice/Grate (Weir Controls 1.13 cfs @ 0.99 fps)

-3=Asymmetrical Weir (Controls 0.00 cfs)

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# **Summary for Link 2L: 24" Storm Sewer to Pebble Creek**

Inflow Area = 10.986 ac, 0.00% Impervious, Inflow Depth > 0.19" for 1-YR event

Inflow = 1.17 cfs @ 13.38 hrs, Volume= 0.177 af

Primary = 1.17 cfs @ 13.38 hrs, Volume= 0.177 af, Atten= 0%, Lag= 0.0 min

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# **Summary for Link 3L: Total Flow including Off-Site Drainage**

Inflow Area = 15.549 ac, 0.00% Impervious, Inflow Depth > 0.58" for 1-YR event

Inflow = 9.76 cfs @ 12.15 hrs, Volume= 0.756 af

Primary = 9.76 cfs @ 12.15 hrs, Volume= 0.756 af, Atten= 0%, Lag= 0.0 min

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# Summary for Subcatchment P1: Flows to BioRetention Pond

Runoff = 24.50 cfs @ 12.15 hrs, Volume= 1.203 af, Depth> 1.64"

	Α	rea (sf)	CN [	Description		
*		52,689	85 7	ype B 65%	, 0	
*		10,870	89 7	ype C 65%	, 0	
*	3	18,872	91 7	Type D 65%	, 0	
	382,431 90 Weighted Average				verage	
	382,431 100.00% Pervious Ar				ervious Are	a
	_		01		•	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.1	40	0.0200	0.13		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	1.0	60	0.0150	1.02		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.70"
	2.1	480	0.0050	3.72	4.57	Pipe Channel,
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013
	8.2	580	Total			

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#### Summary for Subcatchment P1A: Off-Site Drainage into BioRetention Pond

Runoff = 3.97 cfs @ 12.16 hrs, Volume= 0.190 af, Depth> 1.15"

_	Α	rea (sf)	CN I	Description						
*		86,183	83 I	From Existi	rom Existing Conditions Model					
		86,183	•	100.00% Pe	ervious Are	a				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	7.2	100	0.0540	0.23		Sheet Flow, Grass: Short n= 0.150 P2= 2.70"				
	0.9	195	0.0540	3.49		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps				
	8.1	295	Total			•				

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#### Summary for Subcatchment P2: Flows Directly to Pebble Creek Storm Sewer

Runoff = 0.68 cfs @ 12.10 hrs, Volume= 0.027 af, Depth> 1.42"

	Α	rea (sf)	CN	Description		
*		4,051	85	Type B 50%	/o	
*		5,906	88	Type D 50%	6	
_		9,957 9,957	87	Weighted A 100.00% Pe		a
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
	2.8	30	0.0500	0.18		Sheet Flow,
	0.4	40	0.0500	) 1.52		Grass: Short n= 0.150 P2= 2.70"  Sheet Flow, Smooth surfaces n= 0.011 P2= 2.70"
	3.2	70	Total			

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# **Summary for Subcatchment P3: Flows to Kohls Pond**

Runoff = 5.81 cfs @ 12.23 hrs, Volume= 0.375 af, Depth> 1.99"

	Α	rea (sf)	CN [	Description		
*		5,889	91 7	ype B 80%	, 0	
*		92,732	94 7	ype D 80%	6	
		98,621 98,621		Veighted A 00.00% Pe	verage ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	10.7	100	0.0200	0.16		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	3.1	280	0.0100	1.50		Shallow Concentrated Flow,
	0.0	070	0.0050	F 00	40.00	Grassed Waterway Kv= 15.0 fps
	0.9	270	0.0050	5.09	16.00	Pipe Channel, 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
						n= 0.013
	14.7	650	Total			

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MSE 24-hr 3 2-YR Rainfall=2.70" Printed 1/27/2020

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# **Summary for Subcatchment P4: Flows to Highway Ditch**

Runoff = 3.02 cfs @ 12.14 hrs, Volume= 0.143 af, Depth> 1.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-YR Rainfall=2.70"

_	Α	rea (sf)	CN [	Description				
*		41,373	92	Гуре D 70%	6			
		41,373	1	100.00% Pe	ervious Are	a		
	Тс	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.8	95	0.0550	0.23		Sheet Flow,		
							<b>-</b> "	

Grass: Short n= 0.150 P2= 2.70"

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# **Summary for Subcatchment P5: Flows to Highway Storm Sewer**

Runoff = 2.36 cfs @ 12.11 hrs, Volume= 0.107 af, Depth> 1.99"

_	Α	rea (sf)	CN I	Description		
*		28,163	94	Type D 80%	6	
		28,163		100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	2.2	20	0.0400	0.15		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.70"
	1.2	80	0.0150	1.08		Sheet Flow, Smooth surfaces n= 0.011 P2= 2.70"
	1.2	180	0.0150	2.49		Shallow Concentrated Flow, Paved Kv= 20.3 fps
	4.6	280	Total			

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# **Summary for Subcatchment P6: Flows to Concrete Channel to Pebble Creek**

Runoff = 1.16 cfs @ 12.15 hrs, Volume= 0.054 af, Depth> 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-YR Rainfall=2.70"

	Α	rea (sf)	CN I	Description			
*		10,693	65	Type B 10%	, 0		
*		2,828	74	Type C 10%	, 0		
*		17,075	89	Type D 10%	, 0		
		30,596	79 \	Weighted A	verage		
	30,596 100.00% Pervious Area					a	
	Tc	Length	Slope	,	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.4	100	0.0500	0.23		Sheet Flow,	

Grass: Short n= 0.150 P2= 2.70"

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#### **Summary for Pond 1P: BioRetention Pond**

Inflow Area = 10.758 ac, 0.00% Impervious, Inflow Depth > 1.55" for 2-YR event 28.47 cfs @ 12.15 hrs. Volume= Inflow 1.393 af 3.32 cfs @ 12.68 hrs, Volume= Outflow 0.659 af, Atten= 88%, Lag= 31.4 min 0.40 cfs @ 12.68 hrs, Volume= Discarded = 0.298 af Primary 2.93 cfs @ 12.68 hrs, Volume= 0.361 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 28.67' @ 12.68 hrs Surf.Area= 23,403 sf Storage= 36,295 cf

Plug-Flow detention time= 148.9 min calculated for 0.657 af (47% of inflow) Center-of-Mass det. time= 85.6 min ( 863.3 - 777.7 )

Volume Invert Avail.Storage Storage Description #1 27.00' 82,838 cf Custom Stage Data (Prismatic)Listed below (Recalc)

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
27.00	20,018	0	0
29.00	24,068	44,086	44,086
30.00	26,457	25,263	69,349
30.50	27,500	13,489	82,838

Device	Routing	Invert	Outlet Devices
#1	Primary	25.00'	24.0" Round Culvert
	-		L= 200.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 25.00' / 24.00' S= 0.0050 '/' Cc= 0.900
			n= 0.013, Flow Area= 3.14 sf
#2	Device 1	28.50'	48.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Primary	29.80'	Asymmetrical Weir, C= 3.27
	•		Offset (feet) -100.00 0.00 100.00
			Height (feet) 1.00 0.00 1.00
#4	Discarded	27.00'	0.600 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 20.00'

**Discarded OutFlow** Max=0.40 cfs @ 12.68 hrs HW=28.67' (Free Discharge) **4=Exfiltration** (Controls 0.40 cfs)

Primary OutFlow Max=2.92 cfs @ 12.68 hrs HW=28.67' (Free Discharge)

-1=Culvert (Passes 2.92 cfs of 20.62 cfs potential flow)
-2=Orifice/Grate (Weir Controls 2.92 cfs @ 1.35 fps)

-3=Asymmetrical Weir (Controls 0.00 cfs)

# Proposed Conditions Prepared by Microsoft

MSE 24-hr 3 2-YR Rainfall=2.70" Printed 1/27/2020

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# **Summary for Link 2L: 24" Storm Sewer to Pebble Creek**

Inflow Area = 10.986 ac, 0.00% Impervious, Inflow Depth > 0.42" for 2-YR event

Inflow = 2.98 cfs @ 12.68 hrs, Volume= 0.388 af

Primary = 2.98 cfs @ 12.68 hrs, Volume= 0.388 af, Atten= 0%, Lag= 0.0 min

# Proposed Conditions Prepared by Microsoft

MSE 24-hr 3 2-YR Rainfall=2.70" Printed 1/27/2020

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#### **Summary for Link 3L: Total Flow including Off-Site Drainage**

Inflow Area = 15.549 ac, 0.00% Impervious, Inflow Depth > 0.82" for 2-YR event

Inflow = 11.42 cfs @ 12.15 hrs, Volume= 1.068 af

Primary = 11.42 cfs @ 12.15 hrs, Volume= 1.068 af, Atten= 0%, Lag= 0.0 min

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# Summary for Subcatchment P1: Flows to BioRetention Pond

Runoff = 38.41 cfs @ 12.15 hrs, Volume= 1.933 af, Depth> 2.64"

	Α	rea (sf)	CN [	Description		
*		52,689	85 7	ype B 65%	, 0	
*		10,870	89 7	ype C 65%	, 0	
*	3	18,872	91 7	ype D 65%	, 0	
	3	82,431	90 V	Veighted A	verage	
	382,431 100.00% Pervious Area				ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	5.1	40	0.0200	0.13	, ,	Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	1.0	60	0.0150	1.02		Sheet Flow,
	0.4	400	0.0050	2.70	4.57	Smooth surfaces n= 0.011 P2= 2.70"
	2.1	480	0.0050	3.72	4.57	<b>Pipe Channel,</b> 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013
_			Tatal			11- 0.013
	8.2	580	Total			

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# Summary for Subcatchment P1A: Off-Site Drainage into BioRetention Pond

Runoff = 6.94 cfs @ 12.15 hrs, Volume= 0.335 af, Depth> 2.03"

	Α	rea (sf)	CN [	Description						
*		86,183	83 F	From Existi	From Existing Conditions Model					
	86,183 100.00% Pervious Area					a				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	7.2	100	0.0540	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.70"				
	0.9	195	0.0540	3.49		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps				
	8.1	295	Total	-		-				

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# Summary for Subcatchment P2: Flows Directly to Pebble Creek Storm Sewer

Runoff = 1.11 cfs @ 12.09 hrs, Volume= 0.045 af, Depth> 2.37"

_	A	rea (sf)	CN	Description		
4	•	4,051	85	Type B 50%	, o	
4	•	5,906		Type D 50%		
-		9,957 9,957		Weighted A 100.00% Pe	verage ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
	2.8	30	0.0500	0.18		Sheet Flow,
	0.4	40	0.0500	1.52		Grass: Short n= 0.150 P2= 2.70"  Sheet Flow, Smooth surfaces n= 0.011 P2= 2.70"
	3.2	70	Total			

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# **Summary for Subcatchment P3: Flows to Kohls Pond**

Runoff = 8.67 cfs @ 12.22 hrs, Volume= 0.572 af, Depth> 3.03"

	Α	rea (sf)	CN I	Description		
*		5,889	91	Type B 80%	6	
*		92,732	94	Type D 80%	6	
		98,621 98,621		Weighted A 100.00% Pe	verage ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	10.7	100	0.0200	0.16		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.70"
	3.1	280	0.0100	1.50		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps
	0.9	270	0.0050	5.09	16.00	<b>Pipe Channel,</b> 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
	14.7	650	Total			

MSE 24-hr 3 10-YR Rainfall=3.81" Printed 1/27/2020

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# **Summary for Subcatchment P4: Flows to Highway Ditch**

Runoff = 4.59 cfs @ 12.14 hrs, Volume= 0.224 af, Depth> 2.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10-YR Rainfall=3.81"

_	Α	rea (sf)	CN [	Description				
*		41,373	92	Гуре D 70%	6			
	41,373 100.00% Pervious Area					a		
	Тс	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.8	95	0.0550	0.23		Sheet Flow,		
							<b>-</b> "	

Grass: Short n= 0.150 P2= 2.70"

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# **Summary for Subcatchment P5: Flows to Highway Storm Sewer**

Runoff = 3.50 cfs @ 12.11 hrs, Volume= 0.164 af, Depth> 3.04"

	Α	rea (sf)	CN	Description		
*		28,163	94	Type D 80%	6	
	28,163 100.00% Pervious Area					a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	2.2	20	0.0400	0.15		Sheet Flow,
	1.2	80	0.0150	1.08		Grass: Short n= 0.150 P2= 2.70"  Sheet Flow,
	1.2	180	0.0150	2.49		Smooth surfaces n= 0.011 P2= 2.70"  Shallow Concentrated Flow, Paved Kv= 20.3 fps
_	4.6	280	Total			

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# **Summary for Subcatchment P6: Flows to Concrete Channel to Pebble Creek**

Runoff = 2.18 cfs @ 12.15 hrs, Volume= 0.101 af, Depth> 1.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10-YR Rainfall=3.81"

	Α	rea (sf)	CN I	Description			
*		10,693	65	Type B 10%	, 0		
*		2,828	74	Type C 10%	, 0		
*		17,075	89	Type D 10%	, 0		
	30,596 79 Weighted Average						
	30,596 100.00% Pervious Area					a	
	Tc	Length	Slope	,	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.4	100	0.0500	0.23		Sheet Flow,	

Grass: Short n= 0.150 P2= 2.70"

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#### **Summary for Pond 1P: BioRetention Pond**

Inflow Area = 10.758 ac, 0.00% Impervious, Inflow Depth > 2.53" for 10-YR event

Inflow 45.35 cfs @ 12.15 hrs. Volume= 2.268 af

18.56 cfs @ 12.32 hrs, Volume= Outflow 1.519 af, Atten= 59%, Lag= 9.9 min

0.43 cfs @ 12.32 hrs, Volume= Discarded = 0.332 af Primary 18.13 cfs @ 12.32 hrs, Volume= 1.187 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 29.08' @ 12.32 hrs Surf.Area= 24,258 sf Storage= 46,007 cf

Plug-Flow detention time= 100.2 min calculated for 1.519 af (67% of inflow)

Center-of-Mass det. time= 45.8 min (815.5 - 769.7)

Volume	Inve	ert Avail.Sto	rage Stora	ge Description			
#1	27.0	0' 82,83	88 cf Custom Stage Data (Pr		rismatic)Listed below (Recalc)		
Elevation	าท	Surf.Area	Inc.Store	Cum.Store			
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)			
27.0	00	20,018	0	0			
29.0		24,068	44,086	•			
30.0		26,457	25,263	69,349			
30.5	50	27,500	13,489	82,838			
Device	Routing	Invert	Outlet Dev	ices			
#1	Primary	25.00'	24.0" Round Culvert				
			L= 200.0' CPP, square edge headwall, Ke= 0.500				
			Inlet / Outlet Invert= 25.00' / 24.00' S= 0.0050 '/' Cc= 0.900				
			n= 0.013, Flow Area= 3.14 sf				
#2	Device 1	28.50'		z. Orifice/Grate (			
	Duine	00.00		weir flow at low hea			
#3	Primary	29.80'	•	rical Weir, C= 3.27			
			`	t) -100.00 0.00 1 t) 1.00 0.00 1.00			
			i leight (lee	1, 1.00 0.00 1.00			

0.600 in/hr Exfiltration over Surface area Conductivity to Groundwater Elevation = 20.00'

Discarded OutFlow Max=0.43 cfs @ 12.32 hrs HW=29.07' (Free Discharge) **4=Exfiltration** (Controls 0.43 cfs)

Primary OutFlow Max=17.87 cfs @ 12.32 hrs HW=29.07' (Free Discharge)

-1=Culvert (Passes 17.87 cfs of 22.12 cfs potential flow)
-2=Orifice/Grate (Weir Controls 17.87 cfs @ 2.48 fps)

27.00'

-3=Asymmetrical Weir (Controls 0.00 cfs)

#4

Discarded

MSE 24-hr 3 10-YR Rainfall=3.81"

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# **Summary for Link 2L: 24" Storm Sewer to Pebble Creek**

Inflow Area = 10.986 ac, 0.00% Impervious, Inflow Depth > 1.35" for 10-YR event

Inflow = 18.37 cfs @ 12.32 hrs, Volume= 1.233 af

Primary = 18.37 cfs @ 12.32 hrs, Volume= 1.233 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

MSE 24-hr 3 10-YR Rainfall=3.81"

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## **Summary for Link 3L: Total Flow including Off-Site Drainage**

Inflow Area = 15.549 ac, 0.00% Impervious, Inflow Depth > 1.77" for 10-YR event

Inflow = 29.74 cfs @ 12.27 hrs, Volume= 2.294 af

Primary = 29.74 cfs @ 12.27 hrs, Volume= 2.294 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

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# Summary for Subcatchment P1: Flows to BioRetention Pond

Runoff = 67.95 cfs @ 12.15 hrs, Volume= 3.554 af, Depth> 4.86"

	Α	rea (sf)	CN [	Description		
*		52,689	85 7	ype B 65%	, 0	
*		10,870	89 7	ype C 65%	, 0	
*	3	18,872	91 7	Type D 65%	, 0	
	3	82,431	90 V	Veighted A	verage	
	382,431 100.00% Pervious Are				ervious Are	a
	_		01		•	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.1	40	0.0200	0.13		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	1.0	60	0.0150	1.02		Sheet Flow,
						Smooth surfaces n= 0.011 P2= 2.70"
	2.1	480	0.0050	3.72	4.57	Pipe Channel,
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013
	8.2	580	Total			

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## Summary for Subcatchment P1A: Off-Site Drainage into BioRetention Pond

Runoff = 13.59 cfs @ 12.15 hrs, Volume= 0.676 af, Depth> 4.10"

_	Α	rea (sf)	CN [	Description						
*		86,183	83 F	rom Existing Conditions Model						
		86,183	6,183 100.00% Pervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	7.2	100	0.0540	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.70"				
	0.9	195	0.0540	3.49		Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps				
_	8.1	295	Total			•				

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#### Summary for Subcatchment P2: Flows Directly to Pebble Creek Storm Sewer

Runoff = 2.04 cfs @ 12.09 hrs, Volume= 0.086 af, Depth> 4.53"

	,	Area (sf)	CN	Description					
*		4,051	85	Type B 50%	, o				
*		5,906	88	Type D 50%					
_		9,957 9,957	87	7 Weighted Average 100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description			
	2.8	30	0.0500	0.18		Sheet Flow,			
	0.4	40	0.050	0 1.52		Grass: Short n= 0.150 P2= 2.70"  Sheet Flow, Smooth surfaces n= 0.011 P2= 2.70"			
	3.2	70	Total						

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# **Summary for Subcatchment P3: Flows to Kohls Pond**

Runoff = 14.65 cfs @ 12.22 hrs, Volume= 0.998 af, Depth> 5.29"

	Α	rea (sf)	CN [	Description		
*		5,889	91 -	ype B 80%	, 0	
*		92,732	94	ype D 80%	6	
		98,621	94 \	Veighted A	verage	
		98,621	•	00.00% Pe	ervious Are	a
	_		-			<b>—</b>
	Tc	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	10.7	100	0.0200	0.16		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.70"
	3.1	280	0.0100	1.50		Shallow Concentrated Flow,
						Grassed Waterway Kv= 15.0 fps
	0.9	270	0.0050	5.09	16.00	Pipe Channel,
						24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
						n= 0.013
	14.7	650	Total			

MSE 24-hr 3 100-YR Rainfall=6.18" Printed 1/27/2020

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# **Summary for Subcatchment P4: Flows to Highway Ditch**

Runoff = 7.90 cfs @ 12.14 hrs, Volume= 0.402 af, Depth> 5.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-YR Rainfall=6.18"

_	Α	rea (sf)	CN [	Description			_
*		41,373	92	Гуре D 70%	6		_
		41,373	100.00% Pervious Area			a	
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	6.8	95	0.0550	0.23		Sheet Flow,	

Grass: Short n= 0.150 P2= 2.70"

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# **Summary for Subcatchment P5: Flows to Highway Storm Sewer**

Runoff = 5.90 cfs @ 12.11 hrs, Volume= 0.286 af, Depth> 5.30"

	Α	rea (sf)	CN	Description		
*		28,163	94	Type D 80%	6	
	28,163		100.00% Pervious		ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	2.2	20	0.0400	0.15		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.70"
	1.2	80	0.0150	1.08		Sheet Flow,
	1.2	180	0.0150	2.49		Smooth surfaces n= 0.011 P2= 2.70"  Shallow Concentrated Flow, Paved Kv= 20.3 fps
	4.6	280	Total			

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## **Summary for Subcatchment P6: Flows to Concrete Channel to Pebble Creek**

Runoff = 4.55 cfs @ 12.15 hrs, Volume= 0.216 af, Depth> 3.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-YR Rainfall=6.18"

	Α	rea (sf)	CN I	Description		
*		10,693	65	Гуре В 10%	, o	
*		2,828	74	Гуре C 10%	6	
*		17,075	89	Гуре D 10%	6	
		30,596	79 \	Neighted A	verage	
		30,596		100.00% Pe	ervious Are	ea
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.4	100	0.0500	0.23		Sheet Flow,

Grass: Short n= 0.150 P2= 2.70"

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#### **Summary for Pond 1P: BioRetention Pond**

Inflow Area = 10.758 ac, 0.00% Impervious, Inflow Depth > 4.72" for 100-YR event 81.55 cfs @ 12.15 hrs. Volume= Inflow 4.230 af 46.93 cfs @ 12.26 hrs, Volume= Outflow 3.467 af, Atten= 42%, Lag= 6.8 min 0.52 cfs @ 12.27 hrs, Volume= Discarded = 0.392 af 46.42 cfs @ 12.26 hrs, Volume= Primary 3.076 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 30.16' @ 12.27 hrs Surf.Area= 26,801 sf Storage= 73,742 cf

Plug-Flow detention time= 77.2 min calculated for 3.467 af (82% of inflow) Center-of-Mass det. time= 35.0 min (794.0 - 759.0)

Volume	Inv	vert Ava	ail.Storage	Storage [	Description	
#1	27	.00'	82,838 cf	Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (feet)		Surf.Area (sq-ft)		c.Store ic-feet)	Cum.Store (cubic-feet)	
27.0 29.0 30.0	00	20,018 24,068 26,457		0 44,086 25,263	0 44,086 69,349	
30.5 Device	50 Routing	27,500		13,489 82,838 et Devices		
#1	Primary		5.00' <b>24.0</b> L= 2 Inle	O" Round (200.0' CPI) t / Outlet In	<b>Culvert</b> P, square edge	headwall, Ke= 0.500 4.00' S= 0.0050 '/' Cc= 0.900

			L= 200:0 Of 1, oqual c cago ficadwall, fic= 0:0
			Inlet / Outlet Invert= 25.00' / 24.00' S= 0.0050 '/'
			n= 0.013, Flow Area= 3.14 sf
#2	Device 1	28.50'	<b>48.0" Horiz. Orifice/Grate</b> C= 0.600
			Limited to weir flow at low heads
#3	Primary	29.80'	Asymmetrical Weir, C= 3.27
	•		Offset (feet) -100.00 0.00 100.00
			Height (feet) 1.00 0.00 1.00
#4	Discarded	27.00'	0.600 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 20.00'
			-

**Discarded OutFlow** Max=0.51 cfs @ 12.27 hrs HW=30.15' (Free Discharge) **4=Exfiltration** (Controls 0.51 cfs)

Primary OutFlow Max=44.57 cfs @ 12.26 hrs HW=30.15' (Free Discharge)

-1=Culvert (Barrel Controls 25.70 cfs @ 8.18 fps)
-2=Orifice/Grate (Passes 25.70 cfs of 77.71 cfs potential flow)

-3=Asymmetrical Weir (Weir Controls 18.86 cfs @ 0.77 fps)

MSE 24-hr 3 100-YR Rainfall=6.18" Printed 1/27/2020

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# Summary for Link 2L: 24" Storm Sewer to Pebble Creek

Inflow Area = 10.986 ac, 0.00% Impervious, Inflow Depth > 3.45" for 100-YR event

Inflow = 46.92 cfs @ 12.26 hrs, Volume= 3.162 af

Primary = 46.92 cfs @ 12.26 hrs, Volume= 3.162 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

MSE 24-hr 3 100-YR Rainfall=6.18" Printed 1/27/2020

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## **Summary for Link 3L: Total Flow including Off-Site Drainage**

Inflow Area = 15.549 ac, 0.00% Impervious, Inflow Depth > 3.91" for 100-YR event

Inflow = 68.54 cfs @ 12.25 hrs, Volume= 5.064 af

Primary = 68.54 cfs @ 12.25 hrs, Volume= 5.064 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# APPENDIX D WINSLAMM DATA

Data file name: P:\WinSLAMM v10\19-9043 Fox Run\Fox Run Redevelopment.mdb

WinSLAMM Version 10.3.4

Rain file name: C:\WinSLAMM Files\Rain Files\WisReg - Milwaukee WI 1969.RAN Particulate Solids Concentration file name: C:\WinSLAMM Files\v10.1 WI\_AVG01.pscx

Runoff Coefficient file name: C:\WinSLAMM Files\WI\_SL06 Dec06.rsvx

Residential Street Delivery file name: C:\WinSLAMM Files\WI\_Res and Other Urban Dec06.std Institutional Street Delivery file name: C:\WinSLAMM Files\WI\_Com Inst Indust Dec06.std Commercial Street Delivery file name: C:\WinSLAMM Files\WI\_Com Inst Indust Dec06.std Industrial Street Delivery file name: C:\WinSLAMM Files\WI\_Com Inst Indust Dec06.std

Other Urban Street Delivery file name: C:\WinSLAMM Files\WI\_Res and Other Urban Dec06.std

Freeway Street Delivery file name: C:\WinSLAMM Files\Freeway Dec06.std

Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance: False

Pollutant Relative Concentration file name: C:\WinSLAMM Files\WI\_GEO03.ppdx

Source Area PSD and Peak to Average Flow Ratio File: C:\WinSLAMM Files\NURP Source Area PSD Files.csv

Cost Data file name:

Seed for random number generator: -42

Study period starting date: 01/05/69 Study period ending date: 12/31/69

Start of Winter Season: 12/06 End of Winter Season: 03/28

Date: 01-27-2020 Time: 10:01:08

Site information:

- LU# 1 Commercial: Commercial 1 Total area (ac): 8.790
  - 1 Roofs 1: 1.430 ac. Flat Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 13 Paved Parking 1: 1.430 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 25 Driveways 1: 2.860 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 45 Large Landscaped Areas 1: 3.070 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
- LU# 2 Commercial: Commercial 2 Total area (ac): 0.220
  - 25 Driveways 1: 0.110 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 45 Large Landscaped Areas 1: 0.110 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
- LU# 3 Commercial: Commercial 4 Total area (ac): 0.970
  - 1 Roofs 1: 0.170 ac. Flat Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 13 Paved Parking 1: 0.170 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 25 Driveways 1: 0.340 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 45 Large Landscaped Areas 1: 0.290 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
- LU# 4 Commercial: Commercial 5 Total area (ac): 0.640
  - 13 Paved Parking 1: 0.170 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 25 Driveways 1: 0.340 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
  - 45 Large Landscaped Areas 1: 0.130 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz
- LU# 5 Commercial: Commercial 6 Total area (ac): 0.570
  - 25 Driveways 1: 0.060 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

#### 45 - Large Landscaped Areas 1: 0.510 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

```
Control Practice 1: Biofilter CP# 1 (DS) - DS Biofilters # 1
  1. Top area (square feet) = 26457
  2. Bottom aea (square feet) = 20018
 3. Depth (ft): 5
 4. Biofilter width (ft) - for Cost Purposes Only: 80
  5. Infiltration rate (in/hr) = 0.5
 6. Random infiltration rate generation? No
  7. Infiltration rate fraction (side): 1
 8. Infiltration rate fraction (bottom): 1
  9. Depth of biofilter that is rock filled (ft) 0
  10. Porosity of rock filled volume = 0
  11. Engineered soil infiltration rate: 13
  12. Engineered soil depth (ft) = 2
  13. Engineered soil porosity = 0.45
  14. Percent solids reduction due to flow through engineered soil = 0
  15. Biofilter peak to average flow ratio = 3.8
  16. Number of biofiltration control devices = 1
  17. Particle size distribution file: Not needed - calculated by program
  18. Initial water surface elevation (ft): 0
 Soil Data
                          Soil Type Fraction in Eng. Soil
    Sands
                          0.700
    Compost as Amendment
                                   0.300
     Saturation water content percent (Porosity) = 0
     Field capacity (\%) = 0
     Permanent Wilting Point (%) = 0
     Infiltration rate (in/hr) = 13
 Biofilter Outlet/Discharge Characteristics:
    Outlet type: Broad Crested Weir
         1. Weir crest length (ft): 4
         2. Weir crest width (ft): 4
         3. Height of datum to bottom of weir opening: 4.8
    Outlet type: Vertical Stand Pipe
         1. Stand pipe diameter (ft): 48
         2. Stand pipe height above datum (ft): 3.5
    Outlet type: Drain Tile/Underdrain
         1. Underdrain outlet diameter (ft): 0.33
         2. Invert elevation above datum (ft): 0
         3. Number of underdrain outlets: 1
```

```
Control Practice 2: Grass Swale CP# 1 (DS) - DS Grass Swale # 1
Total drainage area (acres)= 8.790
Fraction of drainage area served by swales (ac) = 0.25
Swale density (ft/ac) = 88.80
```

Total swale length (ft) = 780

Average swale length to outlet (ft)= 70

Typical bottom width (ft) = 0.0

Typical swale side slope ( $_{\rm H:1V}$ ) = 4.0

Typical longitudinal slope (ft.H/ft.V) = 0.010

Swale retardance factor: D

Typical grass height (in) = 2.0

Swale dynamic infiltration rate (in/hr)= 0.600

Typical swale depth (ft) for cost analysis (optional) = 1.0

Particle size distribution file name: Not needed - calculated by program

Use total swale length instead of swale density for infiltration calculations: True

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Data file name: P:\WinSLAMM v10\19-9043 Fox Run\Fox Run Redevelopment.mdb

Data file description:

Rain file name: C:\WinSLAMM Files\Rain Files\WisReg - Milwaukee WI 1969.RAN Particulate Solids Concentration file name: C:\WinSLAMM Files\v10.1 WI AVG01.pscx

Runoff Coefficient file name: C:\WinSLAMM Files\WI\_SL06 Dec06.rsvx

Residential Street Delivery file name: C:\WinSLAMM Files\WI\_Res and Other Urban Dec06.std

Institutional Street Delivery file name: C:\WinSLAMM Files\WI\_Com Inst Indust Dec06.std Commercial Street Delivery file name: C:\WinSLAMM Files\WI\_Com Inst Indust Dec06.std Industrial Street Delivery file name: C:\WinSLAMM Files\WI\_Com Inst Indust Dec06.std

Other Urban Street Delivery file name: C:\WinSLAMM Files\WI\_Res and Other Urban Dec06.std

Freeway Street Delivery file name: C:\WinSLAMM Files\Freeway Dec06.std

Pollutant Relative Concentration file name: C:\WinSLAMM Files\WI\_GEO03.ppdx

Start of Winter Season: 12/06 End of Winter Season: 03/28 Model Run Start Date: 01/05/69 Model Run End Date: 12/31/69

Date of run: 01-27-2020 Time of run: 10:00:42

Total Area Modeled (acres): 11.190

Years in Model Run: 0.99

Runoff Percent Particulate Particulate Percent Volume Runoff Solids Solids Particulate (cu ft) Volume Conc. Yield Solids Reduction (mg/L) (lbs) Reduction

Total of all Land Uses without Controls: 572873 - 123.5 4418 -

Outfall Total with Controls: 200539 64.99% 121.9 1526 65.46%

Annualized Total After Outfall Controls: 203324 1547

# APPENDIX E BASIN MAPS

