Stormwater Management Plan for

Sorce Services LLC

Waukesha, Wisconsin

Project No. 138-005-003

September 28, 2020

Stormwater Management Plan for

Sorce Services LLC 220 South Prairie Avenue City of Waukesha, Wisconsin

Prepared by

Endpoint Solutions 6871 South Lovers Lane Franklin, Wisconsin 53132



Prepared for

Sorce Services LLC 7619 Nordale Avenue Franksville, WI 53126

EXECUTIVE SUMMARY

The intent of this document is to present the results of the stormwater management plan for the proposed Sorce Services expansion, on Prairie Avenue, in the City of Waukesha, Waukesha County, WI.

The results of this report provide the water quantity and water quality calculations required by the City of Waukesha and the State of Wisconsin.

- See Table 1 for water quantity summary
- See Table 2 for water quality summary

	Total		
	E Total (cfs)	P Total (cfs)	
1-year, 24-hour Storm	0.25	0.23	
2-year, 24-hour Storm	0.43	0.25	
10-year, 24- hour Storm	1.44	0.81	
100-year, 24- hour Storm	4.68	3.67	

Table 1 – Water Quantity

Table 2 – Proposed SLAMM Output

	Total Suspended Solids	Percent Reduction
Proposed Site w/o Controls	1,877.0 lbs.	
Proposed Site with Controls	234.1 lbs.	87.53%

INTRODUCTION

Endpoint Solutions has been retained by Sorce Services, LLC to prepare a stormwater management plan for the proposed expansion of their property on Prairie Avenue, in the City of Waukesha, Waukesha County, Wisconsin. The existing property is approximately 2.0 acres and will be adding approximately 2.25 acres from the adjacent property to the southwest. The expansion area was formerly a recreational field that has been allowed to become overgrown over time. Sorce Properties intends to develop this site by expanding their recycling/storage operations into this area. The land will largely be a fenced in gravel storage area, along with some greenspace and a stormwater facility.

Stormwater management for this site is regulated by the City of Waukesha Municipal Code Chapter 32: Stormwater Management and Erosion Control, as well as The Wisconsin Department of Natural Resources NR 151, Wisconsin Administrative Code. The analysis presented in this report addresses water quality and water quantity.

METHOD OF ANALYSIS

Hydrologic analysis included in this report was performed using the HydroCAD hydrologic simulation computer model, version 10.00 by HydroCAD Software Solutions LLC. The discharges were generated using the SCS Dimensionless Unit Hydrograph Method for a 24-hour duration storm. Model parameters include drainage area, SCS runoff curve number, time of concentration and 24-hour precipitation with an MSE Type III distribution.

The runoff curve number is determined by the soils, vegetation cover and impervious cover to estimate runoff in the SCS Method. Higher curve numbers mean more potential runoff. Composite curve numbers were determined from the existing and proposed land cover types.

The time of concentration is the time required for water to flow from the most remote part of the drainage area to the point of design. The water moves through the watershed or subbasin as sheet flow, shallow concentrated flow, open channel flow or any combination of these aforementioned items.

Frequency (years)	Rainfall Depth (inches)	
1	24	2.4
2	24	2.7
10	24	3.81
100	24	6.18

Table	3 -	- D	esign	S	to	rm	E	ve	nts	
C 117	1	1		•	•	1 /	~	1	01	

EXISTING SITE CONDITIONS

The site is undeveloped and land cover can be described as densely vegetated cover. The site drains from the north to the south and grades are generally flat with average slopes around 2 percent. No wetland areas were located within the site.

According to the broad NRCS Web Soil Survey, underlying soils within the site contain soils belonging to NRCS Hydrologic Soil Groups B and C (see *Soils Exhibit*). Only a small portion of the site is the C category (approximately 6%) and that area is entirely within proposed pond area. See Table 4 for a brief description of the existing underlying soils according to the NRCS soil survey.

	% of Site		NRCS Unit	Hydrologic Soil
NRCS Soil Name	Area	% slope	Symbol	Group (HSG)
Warsaw Silt Loam	84.7	0 - 2	WhA	В
Kane Silt Loam	15.3	1 – 3	KeA	С

Table 4 – NRCS Soil Survey Summary

The existing property is one watershed (see *Existing Hydrology Exhibit*). The drainage pattern is from the northeast corner of the site to the southwest corner and off-site.

Existing conditions assumed "good hydrologic conditions" for appropriate land covers. Runoff Curve Number was determined based on the grassland cover description in the City's Chapter 32 ordinance and the above proportionate areas, resulting in an existing area RCN of 63. Time of Concentration calculations have been determined individually for each watershed.

Design storms for the existing hydrology model include the 1, 2, 10, and 100-year, 24-hour storm events. See Table 5 for an existing hydrology summary. A full summary of results is located in the *Existing Hydrology Calculations* tab of this report.

	Area	Composite	Тс		Peak Di	scharge (cfs)	
Watershed ID	(acres)	RCN	(minutes)	1-year	2-year	10-year	100-year
E1	2.413	63	30.5	0.25	0.43	1.44	4.68

PROPOSED SITE CONDITIONS

Runoff generated by the developed site is divided into two watersheds (see *Proposed Hydrology Exhibit*). The drainage from the majority of the site discharges to the proposed detention pond along the west edge of the property. The remainder of the site is

undetained areas draining to the west. The ultimate discharge for the entire property drains to the storm sewer immediately south of the property.

WATER QUANTITY AND QUALITY DESIGN

Water quantity requirements (peak discharge rates) are regulated by The Wisconsin Department of Natural Resources NR151, Wisconsin Administrative Code and the City of Waukesha Municipal Code Chapter 32. The City of Waukesha requires postconstruction sites reduce peak stormwater discharges rates for the 2, 10 and 100-year, 24hour design storms to that of pre-developed conditions. The 1-year, 24-hour design storm is also provided.

Runoff curve numbers for all watersheds have been totaled according to the site plan. The watersheds that are virtually all impervious, the times of concentrations were calculated to be less than 6 minutes; therefore, a minimum time of concentration of 6 minutes were assigned to these drainage basins per Technical Release No. 55 recommendation.

See Table 6 for the proposed watersheds and resulting peak discharges which occur prior to detention.

					Peak D	Discharge	
Watershed	Area		Tc	1-	2-	10-	100-
Description	(acres)	CN	(minutes)	year	year	year	year
P-1	0.04	60	6	0.03	0.04	0.08	0.21
(Undetained)	0.04	09					
P-2	2.37	89	6	6.10	6.99	10.44	18.21
P Total (pre-detention)	2.41	89	6	6.13	7.03	10.52	18.42

Table 6 – Proposed Peak Discharge Rates (before detention)

A wet detention pond was utilized to provide the required detention as well as water quality. The pond has a single outlet pipe which drains to an adjacent public storm sewer network. The pond outlet restricts the runoff discharge rate within allowable levels of the governing authorities. The pond has an emergency overflow weir which will provide a controlled release point in the event the primary outflow device's capacity is exceeded.

See Tables 7 & 8 for proposed peak discharge rates and routed water surface elevations after detention.

Detention	Storm Event	Discharge (cfs)	Maximum Elevation	Storage (cu. ft.)
Storm Water Pond	1-yr	0.23	17.03	10,039
(Top EL:20.00)	2-yr	0.24	17.18	11,588
(Spillway EL: 19.20)	10-yr	0.80	17.58	16,152
(NWL EL: 16.00) (Bot EL: 11.00)	100-yr	3.62	18.18	23,362

Table 7 – Storm Water Facilities

Table 8 – Discharge Rates Summary (after detention)

TOTAL CITE

IUIAL SILE					
Storm Event	Existing	Proposed			
1-yr	0.25	0.23			
2-yr	0.43	0.25			
10-yr	1.44	0.81			
100-yr	4.68	3.67			

Water quality requirements are regulated by The Wisconsin Department of Natural Resources NR151, Wisconsin Administrative Code and the City of Waukesha Municipal Code Chapter 32. These codes require best management practices to be designed to reduce the total suspended solids load of new development sites by 80%, based on an average annual rainfall, as compared to no runoff management controls.

An analysis using the approved WINSLAMM model to verify TSS removal rates has been conducted. For detailed calculations, see *WINSLAMM Calculations*.

Table 9 below summarizes the SLAMM output data.

Table 9 -	- Proposed	SLAMM	Output
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	Total Suspended Solids	Percent Reduction
Proposed Site w/o Controls	1,877.0 lbs.	
Proposed Site with Controls	234.1 lbs.	87.53%

INFILTRATION REQUIREMENTS

Stormwater infiltration requirements are regulated by The Wisconsin Department of Natural Resources NR151, Wisconsin Administrative Code and the City of Waukesha Municipal Code Chapter 32. These codes exempt redevelopments and infill areas from infiltration; therefore, infiltration facilities have not been provided. Soils in the area are typically clays and provide another reason for being exempt.

Appendix A - Soil Data

USGS Web Soil Survey



National Cooperative Soil Survey

Conservation Service

МАР	LEGEND	MAP INFORMATION
Area of Interest (AOI) Image: Area of Interest (AOI) Soils Image: Area of Interest (AOI) Soil Image: Area of Interest (AOI) Soils Image: Area of Interest (AOI) Soil Image: Area of Interest (AOI) Image:	LEGENDImage: Spoil AreaImage: Spo	<section-header><section-header><text><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></text></section-header></section-header>
 Saline Spot Sandy Spot Severely Eroded Spot Sinkhole Slide or Slip Sodic Spot 		Date(s) aerial images were photographed: Aug 1, 2019—Oct 20, 2019 The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
KeA	Kane silt loam, 1 to 3 percent slopes	0.1	5.6%
WhA	Warsaw silt loam, 0 to 2 percent slopes	1.6	94.4%
Totals for Area of Interest		1.7	100.0%



Milwaukee and Waukesha Counties, Wisconsin

KeA—Kane silt loam, 1 to 3 percent slopes

Map Unit Setting

National map unit symbol: g94g Elevation: 670 to 1,100 feet Mean annual precipitation: 28 to 36 inches Mean annual air temperature: 37 to 55 degrees F Frost-free period: 135 to 170 days Farmland classification: Prime farmland if drained

Map Unit Composition

Kane and similar soils: 90 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Kane

Setting

Landform: Flats Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Parent material: Loess over loamy outwash over stratified, calcareous sandy and gravelly outwash

Typical profile

Ap,A,AB - 0 to 14 inches: silt loam *BA,2Bt,2BC - 14 to 36 inches:* silty clay loam *3C - 36 to 60 inches:* Error

Properties and qualities

Slope: 1 to 3 percent Depth to restrictive feature: More than 80 inches Drainage class: Somewhat poorly drained Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 1.98 in/hr) Depth to water table: About 12 to 36 inches Frequency of flooding: None Frequency of ponding: Rare Calcium carbonate, maximum content: 40 percent Available water capacity: Moderate (about 7.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2w Hydrologic Soil Group: C Forage suitability group: High AWC, high water table (G095BY007WI)

USDA

Other vegetative classification: High AWC, high water table (G095BY007WI) *Hydric soil rating:* No

Minor Components

Sebewa

Percent of map unit: 7 percent Landform: Depressions Landform position (three-dimensional): Dip Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Warsaw

Percent of map unit: 3 percent Landform: Rises Landform position (three-dimensional): Rise Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Data Source Information

Soil Survey Area: Milwaukee and Waukesha Counties, Wisconsin Survey Area Data: Version 16, Jun 8, 2020



Milwaukee and Waukesha Counties, Wisconsin

WhA—Warsaw silt loam, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2tjx6 Elevation: 750 to 980 feet Mean annual precipitation: 33 to 37 inches Mean annual air temperature: 45 to 48 degrees F Frost-free period: 138 to 193 days Farmland classification: All areas are prime farmland

Map Unit Composition

Warsaw and similar soils: 90 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Warsaw

Setting

Landform: Stream terraces Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Parent material: Loamy glaciofluvial deposits over calcareous sandy and gravelly outwash

Typical profile

Ap - 0 to 14 inches: silt loamBt1 - 14 to 31 inches: sandy clay loam2Bt2 - 31 to 36 inches: gravelly sandy clay loam2C - 36 to 79 inches: stratified sand to gravel

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: 35 to 40 inches to strongly contrasting textural stratification
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: About 60 to 79 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 25 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Available water capacity: Moderate (about 6.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2s

USDA

Hydrologic Soil Group: B
Forage suitability group: Mod AWC, adequately drained (G095BY005WI)
Other vegetative classification: Mod AWC, adequately drained (G095BY005WI)
Hydric soil rating: No

Minor Components

Kane

Percent of map unit: 10 percent Landform: Stream terraces Landform position (three-dimensional): Tread Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Data Source Information

Soil Survey Area: Milwaukee and Waukesha Counties, Wisconsin Survey Area Data: Version 16, Jun 8, 2020



Appendix B

Storm Water Quantity Calculations

- HydroCAD Results



Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.487	61	>75% Grass cover, Good, HSG B (1S, 6S)
0.175	74	>75% Grass cover, Good, HSG C (1S, 6S)
1.548	98	Paved parking, HSG C (6S)
0.203	98	Water Surface, HSG D (6S)
2.043	61	Woods/grass comb., Fair, HSG B (5S)
0.370	71	Woods/grass comb., Fair, HSG C (5S)
4.825	76	TOTAL AREA

138-005-003_Hydrology	MSE 24-hr 3 1-yr Rainfall=2.40"
Prepared by Endpoint Solutions	Printed 9/25/2020
HydroCAD® 10.00-25 s/n 11055 © 2019 Hy	rdroCAD Software Solutions LLC Page 3
Time span=0. Runoff by SCS Reach routing by Stor-Ind+	00-48.00 hrs, dt=0.05 hrs, 961 points TR-20 method, UH=SCS, Weighted-Q Trans method - Pond routing by Stor-Ind method
Subcatchment 1S: P-1 (Undetained)	Runoff Area=1,753 sf 0.00% Impervious Runoff Depth=0.41" Tc=6.0 min CN=WQ Runoff=0.03 cfs 0.001 af
Subcatchment 5S: E-1	Runoff Area=105,100 sf 0.00% Impervious Runoff Depth=0.21" Flow Length=645' Tc=30.5 min CN=WQ Runoff=0.25 cfs 0.042 af
Subcatchment 6S: P-2	Runoff Area=103,330 sf 73.81% Impervious Runoff Depth=1.67" Tc=6.0 min CN=WQ Runoff=6.10 cfs 0.330 af
Reach 5R: Proposed Outfall	Inflow=0.23 cfs 0.320 af
	Outflow=0.23 cfs 0.320 af
Reach 7R: Existing Outfall	Inflow=0.25 cfs 0.042 af
	Outflow=0.25 cfs 0.042 af
Pond 9P: Wet Pond	Peak Elev=17.03' Storage=10,039 cf Inflow=6.10 cfs 0.330 af
Primary=0.23 cf	s 0.318 af Secondary=0.00 cfs 0.000 af Outflow=0.23 cfs 0.318 af
Total Runoff Area = 4.82	5 ac Runoff Volume = 0.374 af Average Runoff Depth = 0.93"

Runoff Area = 4.825 ac Runoff Volume = 0.374 af Average Runoff Depth = 0.93" 63.71% Pervious = 3.074 ac 36.29% Impervious = 1.751 ac

Summary for Subcatchment 1S: P-1 (Undetained)

Runoff = 0.03 cfs @ 12.15 hrs, Volume= 0.001 af, Depth= 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-yr Rainfall=2.40"

	Are	ea (sf)	CN	Description			
		627	61	>75% Gras	s cover, Go	ood, HSG B	
		1,126	74	>75% Gras	s cover, Go	ood, HSG C	
		1,753		Weighted A	verage		
		1,753		100.00% Pe	ervious Are	а	
	Тс	Length	Slop	e Velocity	Capacity	Description	
(m	nin)	(feet)	(ft/ft	i) (ft/sec)	(cfs)		
	6.0					Direct Entry,	

Subcatchment 1S: P-1 (Undetained)



Summary for Subcatchment 5S: E-1

Runoff = 0.25 cfs @ 12.60 hrs, Volume= 0.042 af, Depth= 0.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-yr Rainfall=2.40"

	Ai	rea (sf)	CN	Description				
*		89,001	61	Woods/gras	ss comb., F	air, HSG B		
*		16,099	71	Woods/gras	ss comb., F	air, HSG C		
	1 1	05,100 05,100	100Weighted Average100100.00% Pervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	e Velocity (ft/sec)	Capacity (cfs)	Description		
	10.3	150	0.1270	0.24		Sheet Flow, Grass: Dense n= 0.240 P2= 2.70"		
	20.2	495	0.0067	0.41		Shallow Concentrated Flow, Woodland Kv= 5.0 fps		
	30.5	645	Total					

Subcatchment 5S: E-1



Summary for Subcatchment 6S: P-2

Runoff = 6.10 cfs @ 12.13 hrs, Volume= 0.330 af, Depth= 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-yr Rainfall=2.40"

Area (sf)	CN	Description				
67,417	98	Paved parki	ng, HSG C	;		
20,584	61	>75% Grass	s cover, Go	ood, HSG B		
8,852	98	Water Surfa	ice, HSG D)		
6,477	74	>75% Grass	>75% Grass cover, Good, HSG C			
103,330		Weighted A	verage			
27,061		26.19% Per	vious Area			
76,269		73.81% Impervious Area				
Tc Length (min) (feet)	Sloj (ft/	pe Velocity ft) (ft/sec)	Capacity (cfs)	Description		
6.0				Direct Entry		



Direct Entry,

Subcatchment 6S: P-2



Summary for Reach 5R: Proposed Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.412 ac, 7	2.58% Impe	ervious,	Inflow De	epth > 1	.59" fo	r 1-y	r event	
Inflow	=	0.23 cfs @	13.47 hrs,	Volume	=	0.320 af				
Outflow	=	0.23 cfs @	13.47 hrs,	Volume	=	0.320 af	, Atten=	0%,	Lag= 0.0) min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Reach 5R: Proposed Outfall

Summary for Reach 7R: Existing Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	2.413 ac,	0.00% Impe	ervious,	Inflow De	epth =	0.21	1" for '	1-yr	event	
Inflow	=	0.25 cfs @	12.60 hrs,	Volume	=	0.042 a	af				
Outflow	=	0.25 cfs @	12.60 hrs,	Volume	=	0.042 a	af, A	Atten= 0°	%, L	_ag= 0.	0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Reach 7R: Existing Outfall

Summary for Pond 9P: Wet Pond

Inflow Area =	2.372 ac, 73.81% Impervious,	Inflow Depth = 1.67" for 1-yr event
Inflow =	6.10 cfs @ 12.13 hrs, Volume	= 0.330 af
Outflow =	0.23 cfs @ 13.59 hrs, Volume	= 0.318 af, Atten= 96%, Lag= 87.5 min
Primary =	0.23 cfs @ 13.59 hrs, Volume	= 0.318 af
Secondary =	0.00 cfs $\overline{@}$ 0.00 hrs, Volume	= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 17.03' @ 13.59 hrs Surf.Area= 10,595 sf Storage= 10,039 cf

Plug-Flow detention time= 566.7 min calculated for 0.318 af (96% of inflow) Center-of-Mass det. time= 548.4 min (1,310.1 - 761.7)

<u>Volume</u>	Invert	Avail.Sto	rage Storage	Description			
#1	16.00'	49,29	91 cf Custom	Stage Data (Pri	ismatic) Listed below (Recalc)		
Elevatio	on Si	urf.Area	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)			
16.0	00	8,852	0	0			
17.0	00	10,538	9,695	9,695			
18.0	00	12,280	11,409	21,104			
19.0	00	14,079	13,180	34,284			
20.0	00	15,935	15,007	49,291			
Device	Routing	Invert	Outlet Device	S			
#1	Primary	16.00'	12.0" Round	Culvert			
	-		L= 61.8' CM	P, projecting, no	headwall, Ke= 0.900		
			Inlet / Outlet I	nvert= 16.00' / 1	5.70' S= 0.0049 '/' Cc= 0.900		
			n= 0.015 Cor	ncrete sewer w/n	nanholes & inlets, Flow Area= 0.79 sf		
#2	Device 1	16.00'	3.0" Vert. Ori	fice C= 0.600			
#3	Device 1	17.50'	24.0" Horiz. C	Drifice/Grate C	C= 0.600		
			Limited to wei	r flow at low hea	ads		
#4	Secondary	19.20'	20.0' long x 3	3.0' breadth Bro	ad-Crested Rectangular Weir		
			Head (feet) 0	.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60 1.80 2.00		
			2.50 3.00 3.5	50 4.00 4.50			
			Coef. (English	n) 2.44 2.58 2.0	68 2.67 2.65 2.64 2.64 2.68 2.68		
			2.72 2.81 2.9	92 2.97 3.07 3	.32		
Primary 1=Cu 2= 3=	Primary OutFlow Max=0.23 cfs @ 13.59 hrs HW=17.03' (Free Discharge) 1=Culvert (Passes 0.23 cfs of 1.98 cfs potential flow) 2=Orifice (Orifice Controls 0.23 cfs @ 4.59 fps) -3=Orifice/Grate (Controls 0.00 cfs)						
Second	ary OutFlow	Max=0.00 c	fs @ 0.00 hrs I	HW=16.00' (Fre	ee Discharge)		

4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Prepared by Endpoint Solutions HydroCAD® 10.00-25 s/n 11055 © 2019 HydroCAD Software Solutions LLC



Pond 9P: Wet Pond

138-005-003_Hydrology	MSE 24-hr 3 2-yr Rainfall=2.70"
Prepared by Endpoint Solutions	Printed 9/25/2020
HydroCAD® 10.00-25 s/n 11055 © 2019 H	ydroCAD Software Solutions LLC Page 11
Time span=0 Runoff by SCS Reach routing by Stor-Ind	.00-48.00 hrs, dt=0.05 hrs, 961 points 5 TR-20 method, UH=SCS, Weighted-Q +Trans method - Pond routing by Stor-Ind method
Subcatchment 1S: P-1 (Undetained)	Runoff Area=1,753 sf 0.00% Impervious Runoff Depth=0.56" Tc=6.0 min CN=WQ Runoff=0.04 cfs 0.002 af
Subcatchment 5S: E-1	Runoff Area=105,100 sf 0.00% Impervious Runoff Depth=0.31" Flow Length=645' Tc=30.5 min CN=WQ Runoff=0.43 cfs 0.062 af
Subcatchment 6S: P-2	Runoff Area=103,330 sf 73.81% Impervious Runoff Depth=1.92" Tc=6.0 min CN=WQ Runoff=6.99 cfs 0.379 af
Reach 5R: Proposed Outfall	Inflow=0.25 cfs 0.368 af Outflow=0.25 cfs 0.368 af
Reach 7R: Existing Outfall	Inflow=0.43 cfs 0.062 af Outflow=0.43 cfs 0.062 af
Pond 9P: Wet Pond Primary=0.24 c	Peak Elev=17.18' Storage=11,588 cf Inflow=6.99 cfs 0.379 af fs 0.366 af Secondary=0.00 cfs 0.000 af Outflow=0.24 cfs 0.366 af
Total Runoff Area = 4.82	25 ac Runoff Volume = 0.444 af Average Runoff Depth = 1.10"

Runoff Area = 4.825 ac Runoff Volume = 0.444 af Average Runoff Depth = 1.10" 63.71% Pervious = 3.074 ac 36.29% Impervious = 1.751 ac

Summary for Subcatchment 1S: P-1 (Undetained)

Runoff = 0.04 cfs @ 12.15 hrs, Volume= 0.002 af, Depth= 0.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-yr Rainfall=2.70"

A	rea (sf)	CN	Description						
	627	61	61 >75% Grass cover, Good, HSG B						
	1,126	74	>75% Grass cover, Good, HSG C						
	1,753 Weighted Average								
	1,753		100.00% Pe	ervious Are	a				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment 1S: P-1 (Undetained)



Summary for Subcatchment 5S: E-1

Runoff = 0.43 cfs @ 12.56 hrs, Volume= 0.062 af, Depth= 0.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-yr Rainfall=2.70"

	A	rea (sf)	CN	Description							
*		89,001	61	Woods/grass comb., Fair, HSG B							
*		16,099	71	Woods/gras	ss comb., F	air, HSG C					
105,100Weighted Average105,100100.00% Pervious Are					verage ervious Are	а					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	10.3	150	0.1270	0.24		Sheet Flow,					
	20.2	495	0.0067	0.41		Grass: Dense n= 0.240 P2= 2.70" Shallow Concentrated Flow, Woodland Kv= 5.0 fps					
	20 E	CAE	Takal								

30.5 645 Total

Subcatchment 5S: E-1



Summary for Subcatchment 6S: P-2

Runoff = 6.99 cfs @ 12.13 hrs, Volume= 0.379 af, Depth= 1.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-yr Rainfall=2.70"

Area (sf) CN	Description						
67,41	7 98	Paved park	ing, HSG C	;				
20,58	4 61	>75% Gras	s cover, Go	ood, HSG B				
8,85	2 98	Water Surfa	Vater Surface, HSG D					
6,47	7 74	74 >75% Grass cover, Good, HSG C						
103,33	0	Weighted A	verage					
27,061 26.19% Pervious Area								
76,269 73.81% Impervious Area				ea				
Tc Leng (min) (fee	th Slo et) (ft/	pe Velocity /ft) (ft/sec)	Capacity (cfs)	Description				
60	<i>,</i> , , , , , , , , , , , , , , , , , ,	//////////_/_		Direct Entry				



Direct Entry,

Subcatchment 6S: P-2



Summary for Reach 5R: Proposed Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.412 ac,	72.58% Impe	ervious,	Inflow [Depth > 1	1.83"	for 2-y	r event	
Inflow	=	0.25 cfs @	13.46 hrs,	Volume	=	0.368 a	f			
Outflow	=	0.25 cfs @	13.46 hrs,	Volume	=	0.368 a	f, Atter	ר= 0%,	Lag= 0.0 m	۱in

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Reach 5R: Proposed Outfall

Summary for Reach 7R: Existing Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.413 ac,	0.00% Imperviou	s, Inflow Depth =	0.31" for 2-yr	event
Inflow	=	0.43 cfs @	12.56 hrs, Volur	ne= 0.062 a	af	
Outflow	=	0.43 cfs @	12.56 hrs, Volur	ne= 0.062 a	af, Atten= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Reach 7R: Existing Outfall

Summary for Pond 9P: Wet Pond

Inflow Area =	2.372 ac, 73.81% Impervious,	Inflow Depth = 1.92" for 2-yr event
Inflow =	6.99 cfs @ 12.13 hrs, Volume	= 0.379 af
Outflow =	0.24 cfs @ 13.61 hrs, Volume	= 0.366 af, Atten= 97%, Lag= 88.7 min
Primary =	0.24 cfs @ 13.61 hrs, Volume	e= 0.366 af
Secondary =	0.00 cfs $\overline{@}$ 0.00 hrs, Volume	= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 17.18' @ 13.61 hrs Surf.Area= 10,846 sf Storage= 11,588 cf

Plug-Flow detention time= 597.1 min calculated for 0.366 af (97% of inflow) Center-of-Mass det. time= 578.0 min (1,338.3 - 760.3)

<u>Volume</u>	Invert	Avail.Sto	rage Storage	e Description					
#1	16.00'	49,2	91 cf Custor	n Stage Data (Pri	ismatic) Listed below (Recalc)				
Elevatio	on S	urf.Area	Inc.Store	Cum.Store					
(feet) (sq-ft)		(sq-ft)	(cubic-feet)	(cubic-feet)					
16.0	00	8,852	0	0					
17.0	00	10,538	9,695	9,695					
18.0	00	12,280	11,409	21,104					
19.0	00	14,079	13,180	34,284					
20.0	00	15,935	15,007	49,291					
Device	Routing	Invert	Outlet Devic	es					
#1	Primary	16.00'	12.0" Roun	d Culvert					
	-		L= 61.8' CN	MP, projecting, no	headwall, Ke= 0.900				
			Inlet / Outlet	Invert= 16.00' / 1	5.70' S= 0.0049 '/' Cc= 0.900				
			n= 0.015 Co	n= 0.015 Concrete sewer w/manholes & inlets, Flow Area= 0.79 sf					
#2	Device 1	16.00'	3.0" Vert. Orifice C= 0.600						
#3	Device 1	17.50'	24.0" Horiz. Orifice/Grate C= 0.600						
			Limited to we	eir flow at low hea	ads				
#4	Secondary	19.20'	20.0' long x	3.0' breadth Bro	ad-Crested Rectangular Weir				
			Head (feet)	0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60 1.80 2.00				
			2.50 3.00 3	.50 4.00 4.50					
			Coef. (Englis	sh) 2.44 2.58 2.0	68 2.67 2.65 2.64 2.64 2.68 2.68				
			2.72 2.81 2	92 2.97 3.07 3	.32				
Primary 1=Cu 2= 3=	• OutFlow A Ilvert (Pass •Orifice (Or •Orifice/Gra	Max=0.24 cfs (ses 0.24 cfs of ifice Controls te (Controls	@ 13.61 hrs	HW=17.18' (Free ntial flow) 94 fps)	e Discharge)				
Second	ary OutFlov	v Max=0.00 c	fs @ 0.00 hrs	HW=16.00' (Fre	ee Discharge)				

4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Hydrograph Inflow 6.99 cfs Outflow
 Primary 7-Inflow Area=2.372 ac Secondary 6-Peak Elev=17.18' Storage=11,588 cf 5-Flow (cfs) 4-3-2-1-0.24 cfs 0.00 cfs 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Pond 9P: Wet Pond

138-005-003_Hydrology	MSE 24	4-hr 3 10-yr Rainfall=3.81"							
Prepared by Endpoint Solutions		Printed 9/25/2020							
HydroCAD® 10.00-25 s/n 11055 © 2019	HydroCAD Software Solutions LLC	Page 19							
Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS, Weighted-Q Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method									
Subcatchment 1S: P-1 (Undetained)	Runoff Area=1,753 sf 0.00% Im Tc=6.0 min CN=V	pervious Runoff Depth=1.19" VQ Runoff=0.08 cfs 0.004 af							
Subcatchment 5S: E-1	Runoff Area=105,100 sf 0.00% Im Flow Length=645' Tc=30.5 min CN=V	pervious Runoff Depth=0.80" VQ Runoff=1.44 cfs 0.161 af							
Subcatchment 6S: P-2	Runoff Area=103,330 sf 73.81% Im Tc=6.0 min CN=W	pervious Runoff Depth=2.87" Q Runoff=10.44 cfs 0.568 af							
Reach 5R: Proposed Outfall		Inflow=0.81 cfs 0.554 af Outflow=0.81 cfs 0.554 af							
Reach 7R: Existing Outfall		Inflow=1.44 cfs 0.161 af Outflow=1.44 cfs 0.161 af							
Pond 9P: Wet Pond Primary=0.80	Peak Elev=17.58' Storage=16,152 cfs 0.550 af Secondary=0.00 cfs 0.000	cf Inflow=10.44 cfs 0.568 af af Outflow=0.80 cfs 0.550 af							
Total Runoff Area = 4.825 ac Runoff Volume = 0.733 af Average Runoff Depth = 1.82"									

 $63.71\% \text{ Pervious} = 3.074 \text{ ac} \qquad 36.29\% \text{ Impervious} = 1.751 \text{ ac}$

Summary for Subcatchment 1S: P-1 (Undetained)

Runoff = 0.08 cfs @ 12.14 hrs, Volume= 0.004 af, Depth= 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10-yr Rainfall=3.81"

A	rea (sf)	CN	Description						
	627	61	51 >75% Grass cover, Good, HSG B						
	1,126	74	>75% Grass cover, Good, HSG C						
	1,753 Weighted Average								
	1,753		100.00% Pe	ervious Are	а				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment 1S: P-1 (Undetained)



Summary for Subcatchment 5S: E-1

Runoff = 1.44 cfs @ 12.50 hrs, Volume= 0.161 af, Depth= 0.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10-yr Rainfall=3.81"

	A	rea (sf)	CN I	Description			
*		89,001	61 \	Noods/gras	ss comb., F	air, HSG B	
*		16,099	71	Noods/gras	ss comb., F	air, HSG C	
105,100Weighted Average105,100100.00% Pervious Area					verage ervious Are	a	
	103,100 100.00 % Pervicus An					u	
	Тс	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	10.3	150	0.1270	0.24		Sheet Flow,	
						Grass: Dense n= 0.240 P2= 2.70"	
	20.2	495	0.0067	0.41		Shallow Concentrated Flow,	
_						Woodland Kv= 5.0 fps	
	30 5	645	Total				

Subcatchment 5S: E-1



Summary for Subcatchment 6S: P-2

Runoff = 10.44 cfs @ 12.13 hrs, Volume= 0.568 af, Depth= 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10-yr Rainfall=3.81"

Area (sf)	CN	Description			
67,417	98	Paved parking, HSG C			
20,584	61	>75% Grass cover, Good, HSG B			
8,852	98	Water Surface, HSG D			
6,477	74	>75% Grass cover, Good, HSG C			
103,330		Weighted Average			
27,061	61 26.19% Pervious Area				
76,269		73.81% Impervious Area			
Tc Length (min) (feet)	Sloj (ft/	pe Velocity Capacity Description /ft) (ft/sec) (cfs)			
<u> </u>					



Direct Entry,

Subcatchment 6S: P-2



Summary for Reach 5R: Proposed Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.412 ac,	72.58% Impe	ervious,	Inflow	Depth >	2.76'	for	10-yr even	ıt
Inflow	=	0.81 cfs @	12.94 hrs,	Volume	=	0.554	af			
Outflow	=	0.81 cfs @	12.94 hrs,	Volume	=	0.554	af, A	tten= 0	%, Lag= 0).0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Reach 5R: Proposed Outfall

Summary for Reach 7R: Existing Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.413 ac,	0.00% Impe	ervious,	Inflow De	epth =	0.8	0" for	10-	yr even	t
Inflow	=	1.44 cfs @	12.50 hrs,	Volume	=	0.161 a	af				
Outflow	=	1.44 cfs @	12.50 hrs,	Volume	=	0.161 a	af,	Atten= 0)%,	Lag= 0	.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Reach 7R: Existing Outfall

Summary for Pond 9P: Wet Pond

Inflow Area	=	2.372 ac, 7	'3.81% Impe	ervious, Inflo	w Depth = 2.	.87" for 1	0-yr event
Inflow	=	10.44 cfs @	12.13 hrs,	Volume=	0.568 af		
Outflow	=	0.80 cfs @	12.94 hrs,	Volume=	0.550 af,	, Atten= 92	%, Lag= 48.8 min
Primary	=	0.80 cfs @	12.94 hrs,	Volume=	0.550 af		
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	0.000 af		

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 17.58' @ 12.94 hrs Surf.Area= 11,556 sf Storage= 16,152 cf

Plug-Flow detention time= 604.5 min calculated for 0.549 af (97% of inflow) Center-of-Mass det. time= 587.9 min (1,344.4 - 756.6)

Volume	Invert	Avail.Sto	rage Storage	Description			
#1	16.00'	49,29	91 cf Custom	Stage Data (Pr	ismatic) Listed below (Recalc)		
Elevatio (fee	on Si t)	urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
16.0 17.0 18.0 19.0 20.0	00 00 00 00 00	8,852 10,538 12,280 14,079 15,935	0 9,695 11,409 13,180 15,007	0 9,695 21,104 34,284 49,291			
Device	Routing	Invert	Outlet Device	S			
#1	Primary	16.00'	12.0" Round L= 61.8' CM Inlet / Outlet I n= 0.015 Cor	P, projecting, no nvert= 16.00' / 1 ncrete sewer w/r	headwall, Ke= 0.900 5.70' S= 0.0049 '/' Cc= 0.900 nanholes & inlets. Flow Area= 0.79 sf		
#2 #3	Device 1 Device 1	16.00' 17.50'	3.0" Vert. Ori 24.0" Horiz. C Limited to we	fice C= 0.600 Drifice/Grate C ir flow at low hea	C= 0.600 ads		
#4	Secondary	19.20'	20.0' long x 3.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.68 2.68 2.72 2.81 2.92 2.97 3.07 3.32				
Primary 1=Cu 2= 3=	OutFlow M Ivert (Pass Orifice (Or Orifice/Gra	lax=0.79 cfs (es 0.79 cfs of ifice Controls te (Weir Con	@ 12.94 hrs H 2.80 cfs poten 0.29 cfs @ 5.8 trols 0.50 cfs @	W=17.58' (Free tial flow) 2 fps)) 0.95 fps)) Discharge)		

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=16.00' (Free Discharge) 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Hydrograph Inflow 11 10.44 cfs Outflow
 Primary 10-Inflow Area=2.372 ac Secondary 9 Peak Elev=17.58' 8-Storage=16,152 cf 7-Flow (cfs) 6-5-4 3-2 0.80 cfs 1-0.00 cfs 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

Pond 9P: Wet Pond

138-005-003_Hydrology	MSE 24-hr 3 100-yr Rainfall=6.18"
Prepared by Endpoint Solutions	Printed 9/25/2020
HydroCAD® 10.00-25 s/n 11055 © 2019 HydroCAD®	vdroCAD Software Solutions LLC Page 27
Time span=0 Runoff by SCS Reach routing by Stor-Ind-	.00-48.00 hrs, dt=0.05 hrs, 961 points TR-20 method, UH=SCS, Weighted-Q +Trans method - Pond routing by Stor-Ind method
Subcatchment 1S: P-1 (Undetained)	Runoff Area=1,753 sf 0.00% Impervious Runoff Depth=2.90" Tc=6.0 min CN=WQ Runoff=0.21 cfs 0.010 af
Subcatchment 5S: E-1	Runoff Area=105,100 sf 0.00% Impervious Runoff Depth=2.27" Flow Length=645' Tc=30.5 min CN=WQ Runoff=4.68 cfs 0.456 af
Subcatchment 6S: P-2	Runoff Area=103,330 sf 73.81% Impervious Runoff Depth=5.02" Tc=6.0 min CN=WQ Runoff=18.21 cfs 0.992 af
Reach 5R: Proposed Outfall	Inflow=3.67 cfs 0.980 af Outflow=3.67 cfs 0.980 af
Reach 7R: Existing Outfall	Inflow=4.68 cfs 0.456 af Outflow=4.68 cfs 0.456 af
Pond 9P: Wet Pond Primary=3.62 cf	Peak Elev=18.18' Storage=23,362 cf Inflow=18.21 cfs 0.992 af s 0.971 af Secondary=0.00 cfs 0.000 af Outflow=3.62 cfs 0.971 af
Total Runoff Area = 4.82	25 ac Runoff Volume = 1.458 af Average Runoff Depth = 3.63"

63.71% Pervious = 3.074 ac 36.29% Impervious = 1.751 ac

Summary for Subcatchment 1S: P-1 (Undetained)

Runoff = 0.21 cfs @ 12.13 hrs, Volume= 0.010 af, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-yr Rainfall=6.18"

A	rea (sf)	CN	Description				
	627	61	>75% Gras	s cover, Go	ood, HSG B		
	1,126	74	>75% Gras	s cover, Go	ood, HSG C		
	1,753		Weighted A	verage			
	1,753		100.00% Pervious Area				
Tc	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Subcatchment 1S: P-1 (Undetained)

Hydrograph



Summary for Subcatchment 5S: E-1

Runoff = 4.68 cfs @ 12.46 hrs, Volume= 0.456 af, Depth= 2.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-yr Rainfall=6.18"

	Ai	rea (sf)	CN	Description							
*		89,001	61	Woods/gras	Voods/grass comb., Fair, HSG B						
*		16,099	71	Woods/gras	ss comb., F	air, HSG C					
	1 1	05,100 05,100		Weighted A 100.00% Pe	verage ervious Are	a					
	Tc (min)	Length (feet)	Slope (ft/ft)	e Velocity (ft/sec)	Capacity (cfs)	Description					
	10.3	150	0.1270	0.24		Sheet Flow, Grass: Dense n= 0.240 P2= 2.70"					
	20.2	495	0.0067	0.41		Shallow Concentrated Flow, Woodland Kv= 5.0 fps					
	30.5	645	Total								

Subcatchment 5S: E-1



Summary for Subcatchment 6S: P-2

18.21 cfs @ 12.13 hrs, Volume= 0.992 af, Depth= 5.02" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-yr Rainfall=6.18"

Area (sf) Cl	N De	escription					
67,4	17 9	8 Pa	aved parki	ing, HSG C				
20,5	84 6	51 >7	75% Grass	s cover, Go	od, HSG B			
8,8	52 9	8 W	ater Surfa	ace, HSG D)			
6,4	77 7	′4 >7	>75% Grass cover, Good, HSG C					
103,3	30	W	eighted A	verage				
27,0	61	26	6.19% Per	vious Area				
76,2	69	73.81% Impervious Area						
Tc Len	igth S	Slope	Velocity	Capacity	Description			
(min) (fe	eet)	(ft/ft)	(ft/sec)	(cfs)				
60					Direct Entry			



Direct Entry,

Subcatchment 6S: P-2



Summary for Reach 5R: Proposed Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.412 ac, 7	72.58% Impe	ervious,	Inflow I	Depth >	4.88	3" for	100-	-yr eve	nt
Inflow	=	3.67 cfs @	12.37 hrs,	Volume	=	0.980 a	af				
Outflow	=	3.67 cfs @	12.37 hrs,	Volume	=	0.980 a	af, A	Atten= 0)%, I	Lag= 0	.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Reach 5R: Proposed Outfall

Summary for Reach 7R: Existing Outfall

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2.413 ac,	0.00% Impervious,	Inflow Depth = 2.	.27" for 100-yr event
Inflow	=	4.68 cfs @	12.46 hrs, Volume	e= 0.456 af	
Outflow	=	4.68 cfs @	12.46 hrs, Volume	e= 0.456 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Reach 7R: Existing Outfall

Summary for Pond 9P: Wet Pond

Inflow Area	=	2.372 ac, 7	3.81% Impervious	, Inflow Depth =	5.02" for	100-yr event
Inflow	=	18.21 cfs @	12.13 hrs, Volum	ie= 0.992	af	
Outflow	=	3.62 cfs @	12.42 hrs, Volum	ie= 0.971	af, Atten= 80	0%, Lag= 17.2 min
Primary	=	3.62 cfs @	12.42 hrs, Volum	ie= 0.971	af	
Secondary	=	0.00 cfs @	0.00 hrs, Volum	ie= 0.000	af	

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 18.18' @ 12.42 hrs Surf.Area= 12,606 sf Storage= 23,362 cf

Plug-Flow detention time= 407.6 min calculated for 0.970 af (98% of inflow) Center-of-Mass det. time= 396.2 min (1,148.2 - 752.1)

Volume	Invert	Avail.Sto	rage Storage	Description	
#1	16.00'	49,29	91 cf Custom	n Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio	on Si	urf.Area	Inc.Store	Cum.Store	
(tee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
16.0	00	8,852	0	0	
17.0	00	10,538	9,695	9,695	
18.0	00	12,280	11,409	21,104	
19.0	00	14,079	13,180	34,284	
20.0	00	15,935	15,007	49,291	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	16.00'	12.0" Round	d Culvert P projecting po	beadwall Ke= 0.900
			Inlet / Outlet	Invert= 16.00' / 1	5.70' S= 0.0049 '/' Cc= 0.900
			n= 0.015 Co	ncrete sewer w/r	nanholes & inlets, Flow Area= 0.79 sf
#2	Device 1	16.00'	3.0" Vert. Or	ifice C= 0.600	
#3	Device 1	17.50'	24.0" Horiz.	Orifice/Grate (C= 0.600
#4	Secondary	19 20'	20.0' long x	3.0' breadth Bro	ad-Crested Rectangular Weir
	Cocondary	10.20	Head (feet) (0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.	50 4.00 4.50	
			Coef. (Englis	h) 2.44 2.58 2.	68 2.67 2.65 2.64 2.64 2.68 2.68
			2.72 2.81 2.	92 2.97 3.07 3	.32
Primary 1=Cu 2= 3=	OutFlow M Ivert (Barre Orifice (Pa Orifice/Gra	Max=3.62 cfs (el Controls 3.6 Isses < 0.34 c te (Passes <	@ 12.42 hrs H 2 cfs @ 4.61 f fs potential flov 11.53 cfs pote	W=18.18' (Free ps) w) ntial flow)	e Discharge)
• •		NA 0.00			B: I)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=16.00' (Free Discharge) 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs) Pond 9P: Wet Pond



Appendix C

Storm Water Quality Calculations

- WinSLAMM

Data file name: P:\Sorce - 138\005 - 809 Phillip Drive\003 - Civil Design & Permitting\Hydrology\138-005-003 Slamm.mdb WinSLAMM Version 10.4.1 Rain file name: C:\WinSLAMM Files\Rain Files\WisReg - Milwaukee WI 1969.RAN Particulate Solids Concentration file name: C:\WinSLAMM Files\v10.1 WI AVG01.pscx Runoff Coefficient file name: C:\WinSLAMM Files\WI_SL06 Dec06.rsvx Residential Street Delivery file name: C:\WinSLAMM Files\WI Res and Other Urban Dec06.std Institutional Street Delivery file name: C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std Commercial Street Delivery file name: C:\WinSLAMM Files\WI_Com Inst Indust Dec06.std Industrial Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Other Urban Street Delivery file name: C:\WinSLAMM Files\WI_Res and Other Urban Dec06.std Freeway Street Delivery file name: C:\WinSLAMM Files\Freeway Dec06.std Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance: False Pollutant Relative Concentration file name: C:\WinSLAMM Files\WI_GEO03.ppdx Source Area PSD and Peak to Average Flow Ratio File: C:\WinSLAMM Files\NURP Source Area PSD Files.csv Cost Data file name: Seed for random number generator: -42 Study period starting date: 01/05/69 Study period ending date: 12/31/69 Date: 09-25-2020 Time: 13:33:14 Site information: LU# 1 - Commercial: P-1 Total area (ac): 2.412 13 - Paved Parking 1: 1.548 ac. Connected PSD File: C:\WinSLAMM Files\NURP.cpz 45 - Large Landscaped Areas 1: 0.661 ac. Moderately Compacted Clayey PSD File: C:\WinSLAMM Files\NURP.cpz 70 - Water Body Areas: 0.203 ac. PSD File: Control Practice 1: Wet Detention Pond CP# 1 (DS) - DS Wet Pond # 2 Particle Size Distribution file name: Not needed - calculated by program Initial stage elevation (ft): 5 Peak to Average Flow Ratio: 3.8 Maximum flow allowed into pond (cfs): No maximum value entered Outlet Characteristics: Outlet type: Orifice 1 1. Orifice diameter (ft): 0.25 2. Number of orifices: 1 3. Invert elevation above datum (ft): 5 Outlet type: Broad Crested Weir 1. Weir crest length (ft): 20 2. Weir crest width (ft): 3 3. Height from datum to bottom of weir opening: 8.2 Outlet type: Vertical Stand Pipe 1. Stand pipe diameter (ft): 2 2. Stand pipe height above datum (ft): 6.5 Pond stage and surface area Pond Area Natural Seepage Other Outflow Stage Entry Number (ft) (acres) (in/hr) (cfs) 0.ÒÓ 0.00Ó0 ò.00 0.00 0 1 1.00 0.0400 0.00 0.00 2.00 0.0614 0.00 0.00 2 3.00 0.0836 3 0.00 0.00 4 4.00 0.1064 0.00 0.00 5 5.00 0.2032 0.00 0.00 6 6.00 0.2419 0.00 0.00 7 7.00 0.2819 0.00 0.00 0.3232 8 8.00 0.00 0.00

9.00

9

0.3658

0.00

0.00

Outfall Total with Controls:

Annualized Total After Outfall Controls:

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213623

216590

-0.11%

17.55

87.53%

234.1

237.3

Appendix D

Hydrology Exhibits



