

STORM WATER MANAGEMENT PLAN

FOR

NEENAH ENTERPRISES – STORAGE YARD EXPANSION

1220 S. Prairie Avenue Waukesha, WI

September 20, 2021



PREPARED BY: Christopher A. Jackson, PE CJ Engineering 9205 W. Center Street Suite 214 Milwaukee, WI 53222 Ph. 414-443-1312

CJE Job No.: 2151R0-SWMP

TABLE OF CONTENTS

- 1.) Introduction
- 2.) SWMP Requirements
- 3.) Developed Site Area & Cover
- 4.) 24-Hour Rainfall Values
- 5.) Method of Analysis
- 6.) Drainage Summary
- 7.) Water Quality
- 8.) Infiltration
- 9.) Conclusion
- 10.) Appendix

A. Existing Conditions

- i. NRCS Web Soils Data Viewer Soils Report
- ii. HydroCAD / TR-55 calculations Existing Conditions
- iii. SWMP Existing Conditions plan
- **B.** Proposed Conditions
 - i. SWMP Proposed Conditions Plan
 - ii. HydroCAD / TR-55 calculations Proposed Conditions
 - iii. WinSLAMM for Windows version 10.2.0 Water Quality inputs and results
 - iv. Maintenance Plan

Introduction:

The proposed development of the parking lot expansion for the Neenah Enterprises building in the City of Waukesha will consist of the development of a site located 1220 S. Prairie Avenue. The existing building will remain along with the north portion of the parking area and entrance drive. The existing grass area immediately on the north side of the existing building will be converted to a paved storage yard addition. Along with the new paved storage yard to the north and a small parking lot addition to the east of the building, a new storm water basin will be constructed. The proposed paved storage yard will drain to the proposed storm water basin to the west of the paved storage yard. The development will result in an increase of impervious area by 1.11 acres.

The development of this site will create a disturbed area equal to 1.69 acres. This storm water management plan has been created to show conformance with the City of Waukesha and WI DNR storm water requirements for this development by analyzing the entire 2.107 acres of contributing area. The storm water practice included in this project consists of creating a storm water basin located on the west side of the development. The proposed storm water swale located on the west side of the site. This is consistent with the existing drainage pattern of the existing site. This storm water basin will provide sediment removal for water quality as well as providing water quantity control.

The proposed development meets and exceeds the storm water management requirements of NR 151 and the City of Waukesha.

<u>Storm water requirements per the WI DNR – NR 151and Chapter 32 City of</u> Waukesha Stormwater Oridnance:

Runoff Quantity Control:

WI DNR AND City – Per Section 32.10.d.1 of the Waukesha City Ordinance: The calculated post-development peak storm water discharge rate shall not exceed the calculated pre-development discharge rates for the 1-year, 2-year, 10-year, and 100-year, 24-hour design storms

Runoff Quality Control: WI DNR & City – 80% to total suspend sediment (TSS) loads from parking areas and roads shall be removed prior to discharge. (Per NR 151.122 (2)) City of Waukesha – Per section 32.10.d.2(i): For new land development and in-fill development, 80% reduction in total suspended solids load;

Developed Site:	(See the Pro	posed Condition	ons Plan: Ap	ppendix "B").
-----------------	--------------	-----------------	--------------	---------------

Soil Types:	Per the NRCS soil survey for Waukesh County the underlying soils in the area of redevelopment consist of Lorenzo Loam (LyB2), HSG B.
Cover & CN:	CN 61, 75-100% Grass Cover, Good condition, HSG B. CN 98, Paved Parking, Drives & Roofs (impervious surface)

Analyzed Area: 2.107 Acres

24-Hour Rainfall Values: 1-Year: 2.40" 2-Year: 2.70" 10-Year: 3.81" 100-Year: 6.18"

All rainfall values per NRCS 24-hour Rain Fall Distribution & Values for Waukesha County Based on NOAA atlas 14 rainfall values and the MSE3 distribution for Waukesha County.

Method of Analysis:

The storm water runoff quantity was calculated using the methods outlines in TR-55 ("Urban Hydrology for Small Watersheds" by the U.S. Department of Agriculture's Soil Conservation Services). Calculations were performed with the "HydroCAD 10.00" computer software. Water quality calculations were done using WinSLAMM for Windows version 10.2.0

Area	1 Year Storm	2 Year Storm	10 Year Storm	100 Year Storm
Existing Conditions				
Subcatchment 1 (total runoff)	0.72 cfs	1.15 cfs	3.14 cfs	8.58 cfs
Proposed Conditions				
Subcatchment 1 (Into Pond)	4.31 cfs	5.10 cfs	8.09 cfs	14.46 cfs
Pond (Outflow)	0.27 cfs	0.32 cfs	1.14 cfs	3.31 cfs
Undetained	0.28 cfs	0.39 cfs	0.83 cfs	1.94 cfs
ReachTotal Peak Discharge	0.39 cfs	0.57 cfs	1.29 cfs	4.95 cfs

Drainage Summary: (See Summary of Calculations in Appendix)

Water Quality:

WDNR requirements for development for water quality per section NR151.122 (2) table 1, requires the project to remove over 80% of the total suspended solids (TSS) from parking areas and roads prior to discharge off site, as quantified using WinSLAMM for Windows version 10.2.0 (See appendix for calculation results and inputs). This is achieved by creating a storm water pond with permanent water storage. The TSS removal of site is as summarized below:

	Before Drainage System	After Controls	% Reduction
Disturbed Area	962.8 lbs	184.1 lbs	80.88%

Infiltration:

Per NR 151.124(4)c); A site has infiltration rate exemptions where the infiltration rate of the soil measured at the proposed bottom of the infiltration system is less than 0.6 inches per hour using a scientifically credible field test method. Based on WDNR technical standard 1002.5 Table 2. Loam soils have a design infiltration rate of 0.24 inches per hour the proposed site exempt from infiltration requirements.

Conclusion:

The proposed peak runoff rates under post-redevelopment conditions are below the peak runoff rate under the existing conditions. The storm water pond will remove over 80% of TSS from the proposed runoff from the disturbed areas after development. Therefore, the proposed development meets and exceeds the storm water management and water quality requirements for the City of Waukesha and WDNR NR 151.

APPENDIX



United States Department of Agriculture

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Milwaukee and Waukesha Counties, Wisconsin



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface	2
Soil Map	5
Soil Map	6
Legend	7
Map Unit Legend	9

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



MAP LEGEND		MAP INFORMATION		
Area of Interest (AOI)	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:15,800.		
Soils ∅ Soil Map Unit Polygons ∅ Soil Map Unit Lines ♥ Soil Map Unit Points △ Special Point Features ✓	Very Stony Spot Wet Spot Other Special Line Features	Warning: Soil Map may not be valid at this scale. Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed		
Image: Blowout Water Feat Image: Blowout Image: Blowout Image: Blowout Image: Blowout <td>ures Streams and Canals tion</td> <td>Scale.</td>	u res Streams and Canals tion	Scale.		
 Clay Spot Closed Depression Gravel Pit 	Rails Interstate Highways US Routes	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (FPSG:3857)		
Image: Statuting oper	Major Roads Local Roads d Aerial Photography	Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required		
 Mine or Quarry Miscellaneous Water Perennial Water Rock Outcrop 		This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.		
 Saline Spot Sandy Spot Severely Eroded Spot 		Wisconsin Survey Area Data: Version 16, Jun 8, 2020 Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.		
 Sinkhole Slide or Slip Sodic Spot 		Date(s) aerial images were photographed: May 20, 2020—Aug 20, 2020 The orthophoto or other base map on which the soil lines were		

MAP LEGEND

MAP INFORMATION

imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
KeA	Kane silt loam, 1 to 3 percent slopes	0.5	15.5%
LyB2	Lorenzo loam, 2 to 6 percent slopes, eroded	2.5	81.2%
WhA	Warsaw silt loam, 0 to 2 percent slopes	0.1	3.3%
Totals for Area of Interest		3.1	100.0%



Routing Diagram for CJE2151R1 Prepared by {enter your company name here}, Printed 9/20/2021 HydroCAD® 10.00-16 s/n 03450 © 2015 HydroCAD Software Solutions LLC

Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
1.750	61	>75% Grass cover, Good, HSG B (ES)
0.357	98	Paved parking, HSG B (ES)
2.107	67	TOTAL AREA

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES: EXISTING CONDITIONS Runoff Area=2.107 ac 16.94% Impervious Runoff Depth>0.29" Flow Length=170' Tc=9.7 min CN=67 Runoff=0.72 cfs 0.051 af

> Total Runoff Area = 2.107 ac Runoff Volume = 0.051 af Average Runoff Depth = 0.29" 83.06% Pervious = 1.750 ac 16.94% Impervious = 0.357 ac

Summary for Subcatchment ES: EXISTING CONDITIONS

Runoff = 0.72 cfs @ 12.21 hrs, Volume= 0.051 af, Depth> 0.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1 year Rainfall=2.40"

Area	a (ac)	CN	Desc	ription		
	1.750	61	>75%	6 Grass co	over, Good,	HSG B
	0.357	98	Pave	ed parking,	HSG B	
	2.107	67	Weig	hted Aver	age	
1.750 83.06% Pervious Area					us Area	
	0.357		16.94	4% Imperv	vious Area	
To (min	c Lengt) (fee	th t)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	2 10	0 0	.0290	0.18		Sheet Flow, A-B
0.5	5 7	'0 O	0.0200	2.28		Grass: Short n= 0.150 P2= 2.70" Shallow Concentrated Flow, B-C Unpaved Kv= 16.1 fps
9.7	7 17	'0 T	otal			

Subcatchment ES: EXISTING CONDITIONS



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES: EXISTING CONDITIONS Runoff Area=2.107 ac 16.94% Impervious Runoff Depth>0.41" Flow Length=170' Tc=9.7 min CN=67 Runoff=1.15 cfs 0.072 af

> Total Runoff Area = 2.107 ac Runoff Volume = 0.072 af Average Runoff Depth = 0.41" 83.06% Pervious = 1.750 ac 16.94% Impervious = 0.357 ac

Summary for Subcatchment ES: EXISTING CONDITIONS

Runoff = 1.15 cfs @ 12.20 hrs, Volume= 0.072 af, Depth> 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2 year Rainfall=2.70"

	Area	(ac) C	N Des	cription			
	1.	750 (61 >75	% Grass co	over, Good,	HSG B	
_	0.	357 9	98 Pav	ed parking	, HSG B		
	2.	107 6	67 Wei	ghted Aver	age		
	1.	750	83.0	6% Pervio	us Area		
	0.	357	16.9	4% Imper	∕ious Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	9.2	100	0.0290	0.18		Sheet Flow, A-B	_
	0.5	70	0.0200	2.28		Grass: Short n= 0.150 P2= 2.70" Shallow Concentrated Flow, B-C Unpaved Kv= 16.1 fps	
	9.7	170	Total				

Subcatchment ES: EXISTING CONDITIONS



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES: EXISTING CONDITIONS Runoff Area=2.107 ac 16.94% Impervious Runoff Depth>0.97" Flow Length=170' Tc=9.7 min CN=67 Runoff=3.14 cfs 0.170 af

> Total Runoff Area = 2.107 ac Runoff Volume = 0.170 af Average Runoff Depth = 0.97" 83.06% Pervious = 1.750 ac 16.94% Impervious = 0.357 ac

Summary for Subcatchment ES: EXISTING CONDITIONS

Runoff = 3.14 cfs @ 12.19 hrs, Volume= 0.170 af, Depth> 0.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10 year Rainfall=3.81"

_	Area	(ac) (CN De	scription			
	1.	750	61 >7	5% Grass c	over, Good	, HSG B	
_	0.	357	98 Pa	ved parking	, HSG B		
	2.	107	67 We	eighted Ave	rage		
	1.	750	83	06% Pervic	ous Area		
	0.	357	16	.94% Imper	vious Area		
	Tc (min)	Length (feet)	Slope (ft/ft	e Velocity) (ft/sec)	Capacity (cfs)	Description	
	9.2	100	0.0290	0.18		Sheet Flow, A-B	
_	0.5	70	0.0200) 2.28		Grass: Short n= 0.150 P2= 2.70" Shallow Concentrated Flow, B-C Unpaved Kv= 16.1 fps	
	9.7	170	Total				

Subcatchment ES: EXISTING CONDITIONS



Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment ES: EXISTING CONDITIONS Runoff Area=2.107 ac 16.94% Impervious Runoff Depth>2.54" Flow Length=170' Tc=9.7 min CN=67 Runoff=8.58 cfs 0.446 af

> Total Runoff Area = 2.107 ac Runoff Volume = 0.446 af Average Runoff Depth = 2.54" 83.06% Pervious = 1.750 ac 16.94% Impervious = 0.357 ac

Summary for Subcatchment ES: EXISTING CONDITIONS

Runoff = 8.58 cfs @ 12.18 hrs, Volume= 0.446 af, Depth> 2.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100 year Rainfall=6.18"

_	Area	(ac) C	N Des	cription		
	1.	750 (61 >75	% Grass co	over, Good	, HSG B
_	0.	357 9	98 Pav	ed parking	, HSG B	
_	2.	107 (67 Wei	ghted Aver	age	
1.750 83.06% Pervious Area					uš Area	
	0.	357	16.9	4% Imper	/ious Area	
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.2	100	0.0290	0.18		Sheet Flow, A-B
						Grass: Short n= 0.150 P2= 2.70"
	0.5	70	0.0200	2.28		Shallow Concentrated Flow, B-C
_						Unpaved Kv= 16.1 fps
	97	170	Total			

Subcatchment ES: EXISTING CONDITIONS





NO RECORDS OF WATER LINES PER THE CITY OF -WAUKESHA WATER UTILITY









Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.639	61	>75% Grass cover, Good, HSG B (PS-1, PS-2)
1.468	98	Paved parking, HSG B (PS-1, PS-2)
2.107	87	TOTAL AREA

CJE2151R1	MSE 24-hr 3 1 year Rainfall=2.40"
Prepared by {enter your company name here}	Printed 9/20/2021
HydroCAD® 10.00-16 s/n 03450 © 2015 HydroCAD Software Solutions	LLC Page 3

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PS-1: PROPOSED	Runoff Area=1.772 ac 76.81% Impervious Runoff Depth>1.31" Tc=6.0 min CN=89 Runoff=4.31 cfs 0.194 af
Subcatchment PS-2: UNDETAINED	Runoff Area=0.335 ac 31.94% Impervious Runoff Depth>0.48" Tc=6.0 min CN=73 Runoff=0.28 cfs 0.013 af
Reach PR: TOTAL OUTFALL	Inflow=0.39 cfs 0.136 af Outflow=0.39 cfs 0.136 af
Pond 1P: STORM WATER POND Primary=0.27 cfs 0	Peak Elev=830.31' Storage=5,443 cf Inflow=4.31 cfs 0.194 af .122 af Secondary=0.00 cfs 0.000 af Outflow=0.27 cfs 0.122 af

Total Runoff Area = 2.107 acRunoff Volume = 0.207 af
30.33% Pervious = 0.639 acAverage Runoff Depth = 1.18"
69.67% Impervious = 1.468 ac

Summary for Subcatchment PS-1: PROPOSED SUBCATCHMENT 1: TO POND

Runoff 4.31 cfs @ 12.13 hrs, Volume= 0.194 af, Depth> 1.31" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1 year Rainfall=2.40"

 Area	(ac)	CN	Desc	ription		
1.	361	98	Pave	d parking,	HSG B	
 0.	411	61	>75%	6 Grass co	over, Good,	, HSG B
 1.	772	89	Weig	hted Aver	age	
0.	411		23.19	9% Pervio	us Area	
1.	361		76.8 ⁻	1% Imperv	vious Area	
 Tc (min)	Lengt (fee	th S	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0						Direct Entry, Min TC

Subcatchment PS-1: PROPOSED SUBCATCHMENT 1: TO POND



Summary for Subcatchment PS-2: UNDETAINED

Runoff = 0.28 cfs @ 12.15 hrs, Volume= 0.013 af, Depth> 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1 year Rainfall=2.40"

6.0						Direct Entry, Min Tc
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
Тс	Lengt	h :	Slope	Velocity	Capacity	Description
0.			2110			
0.1	107		31.94	4% Imperv	vious Area	
0.2	228		68.0	6% Pervio	us Area	
0.3	335	73	Weig	hted Aver	age	
0.1	107	98	Pave	ed parking,	, HSG B	
0.2	228	61	>75%	6 Grass co	over, Good,	, HSG B
Area (ac)	CN	Desc	cription		

Subcatchment PS-2: UNDETAINED



Summary for Reach PR: TOTAL OUTFALL

Inflow Ar	rea =	2.107 ac, 69.67% Impervious, Inflo	ow Depth > 0.77"	for 1 year event
Inflow	=	0.39 cfs @ 12.17 hrs, Volume=	0.136 af	
Outflow	=	0.39 cfs @ 12.17 hrs, Volume=	0.136 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Reach PR: TOTAL OUTFALL

Summary for Pond 1P: STORM WATER POND

Inflow Area =	1.772 ac, 76.81% Impervious, Inflow I	Depth > 1.31" for 1 year event
Inflow =	4.31 cfs @ 12.13 hrs, Volume=	0.194 af
Outflow =	0.27 cfs @ 13.30 hrs, Volume=	0.122 af, Atten= 94%, Lag= 70.2 min
Primary =	0.27 cfs @ 13.30 hrs, Volume=	0.122 af
Secondary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 830.31' @ 13.30 hrs Surf.Area= 7,206 sf Storage= 5,443 cf

Plug-Flow detention time= 200.7 min calculated for 0.122 af (63% of inflow) Center-of-Mass det. time= 142.4 min (921.6 - 779.3)

Volume	Invert	Avail.Sto	rage Storage	e Description					
#1	829.50	35,68	B6 cf Custor	n Stage Data (Pi	rismatic)Listed below (Recalc)				
Elevatio (fee	n S t)	urf.Area (sɑ-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)					
829.5	50	6,263	0	0					
830.0	0	6,838	3,275	3,275					
831.0	0	8,030	7,434	10,709					
832.0	0	18,066	13,048	23,757					
832.5	0	29,647	11,928	35,686					
Device	Routing	Invert	Outlet Device	es					
#1 #2 #3 #4 #5	Primary Device 1 Device 1 Device 1 Secondary	829.50' 829.50' 829.80' 830.80' 831.50'	 10.0" Round Culvert L= 19.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 829.50' / 829.00' S= 0.0263 '/' Cc= 0.900 n= 0.011, Flow Area= 0.55 sf 1.0" Vert. Orifice/Grate C= 0.600 4.0" Vert. Orifice/Grate C= 0.600 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads 10.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 						
Primary 1=Cu -2= -3= -4=	Primary OutFlow Max=0.27 cfs @ 13.30 hrs HW=830.31' (Free Discharge) 1=Culvert (Passes 0.27 cfs of 1.66 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.02 cfs @ 4.22 fps) 3=Orifice/Grate (Orifice Controls 0.25 cfs @ 2.82 fps) 4=Orifice/Grate (Controls 0.00 cfs)								
Seconda 1−5=Bro	econdary OutFlow Max=0.00 cfs @ 5.00 hrs HW=829.50' (Free Discharge) —5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)								



Pond 1P: STORM WATER POND

CJE2151R1	MSE 24-hr 3 2 year Rainfall=2.70"
Prepared by {enter your company name here}	Printed 9/20/2021
HydroCAD® 10.00-16 s/n 03450 © 2015 HydroCAD Software Solutions	LLC Page 9

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PS-1: PROPOSED	Runoff Area=1.772 ac 76.81% Impervious Runoff Depth>1.57" Tc=6.0 min CN=89 Runoff=5.10 cfs 0.231 af
Subcatchment PS-2: UNDETAINED	Runoff Area=0.335 ac 31.94% Impervious Runoff Depth>0.64" Tc=6.0 min CN=73 Runoff=0.39 cfs 0.018 af
Reach PR: TOTAL OUTFALL	Inflow=0.57 cfs 0.172 af Outflow=0.57 cfs 0.172 af
Pond 1P: STORM WATER POND Primary=0.32 cfs 0	Peak Elev=830.46' Storage=6,529 cf Inflow=5.10 cfs 0.231 af .154 af Secondary=0.00 cfs 0.000 af Outflow=0.32 cfs 0.154 af

Total Runoff Area = 2.107 acRunoff Volume = 0.249 afAverage Runoff Depth = 1.42"30.33% Pervious = 0.639 ac69.67% Impervious = 1.468 ac

Runoff = 5.10 cfs @ 12.13 hrs, Volume= 0.231 af, Depth> 1.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2 year Rainfall=2.70"

 Area (ac)	CN	Desc	ription		
1.3	361	98	Pave	d parking,	HSG B	
 0.4	411	61	>75%	6 Grass co	over, Good,	, HSG B
1.7	772	89	Weig	hted Aver	age	
0.4	411		23.19	9% Pervio	us Area	
1.3	361		76.8 ⁻	1% Imperv	vious Area	
Tc (min)	Lengt	h ያ	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
 6.0	(-,	(1411)	()	()	Direct Entry, Min TC





Summary for Subcatchment PS-2: UNDETAINED

Runoff = 0.39 cfs @ 12.14 hrs, Volume= 0.018 af, Depth> 0.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2 year Rainfall=2.70"

 Area	(ac)	CN	Desc	ription		
0.	228	61	>75%	6 Grass co	over, Good,	, HSG B
 0.	107	98	Pave	ed parking,	HSG B	
0.	335	73	Weig	hted Aver	age	
0.	228		68.0	6% Pervio	us Area	
0.	107		31.94	4% Imperv	vious Area	
_					_	
Тс	Lengt	h	Slope	Velocity	Capacity	Description
 (min)	(fee	<u>t)</u>	(ft/ft)	(ft/sec)	(cfs)	
6.0						Direct Entry, Min Tc
						-

Subcatchment PS-2: UNDETAINED



Summary for Reach PR: TOTAL OUTFALL

Inflow A	Area =	2.1	07 ac,	69.67% Impe	ervious,	Inflow Dep	th > 0.9	98" for 2 y	ear event
Inflow	=	0.5	7 cfs @	12.16 hrs,	Volume	= 0	.172 af		
Outflow	/ =	0.5	7 cfs @	12.16 hrs,	Volume	= 0	.172 af,	Atten= 0%,	Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Reach PR: TOTAL OUTFALL

Summary for Pond 1P: STORM WATER POND

Inflow Area =	1.772 ac, 76.81% Impervious, Inflow De	epth > 1.57" for 2 year event
Inflow =	5.10 cfs @ 12.13 hrs, Volume=	0.231 af
Outflow =	0.32 cfs @ 13.28 hrs, Volume=	0.154 af, Atten= 94%, Lag= 68.7 min
Primary =	0.32 cfs @ 13.28 hrs, Volume=	0.154 af
Secondary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 830.46' @ 13.28 hrs Surf.Area= 7,383 sf Storage= 6,529 cf

Plug-Flow detention time= 206.1 min calculated for 0.154 af (67% of inflow) Center-of-Mass det. time= 150.2 min (926.5 - 776.3)

Volume	Invert	Avail.Stor	rage Storage	Description		
#1	829.50'	35,68	36 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)	
Elevatior (feet	n Su	rf.Area (sq-ft)	Inc.Store (cubic-feet)	Inc.Store Cum.Store (cubic-feet) (cubic-feet)		
829.50 830.00 831.00 832.00 832.50)))))	6,263 6,838 8,030 18,066 29,647	0 3,275 7,434 13,048 11,928	0 3,275 10,709 23,757 35,686		
Device	Routing	Invert	Outlet Device	S		
#1 #2 #3 #4 #5	Primary Device 1 Device 1 Device 1 Secondary	829.50' 829.50' 829.80' 830.80' 831.50'	10.0" Round L= 19.0' CPF Inlet / Outlet In n= 0.011, Flo 1.0" Vert. Ori 4.0" Vert. Ori 24.0" Horiz. O Limited to wei 10.0' long x Head (feet) 0 Coef. (English	Culvert P, square edge I nvert= 829.50' / w Area= 0.55 st fice/Grate C= fice/Grate C= fice/Grate C= 0rifice/Grate C= 0rifice/Grate C= 0rifice/Grate C= 0.10' breadth 0.20 0.40 0.60 n) 2.49 2.56 2.	neadwall, Ke= 0.500 829.00' S= 0.0263 '/' Cc= 0.900 0.600 0.600 C= 0.600 ads road-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 70 2.69 2.68 2.69 2.67 2.64	
Primary OutFlow Max=0.32 cfs @ 13.28 hrs HW=830.46' (Free Discharge) 1=Culvert (Passes 0.32 cfs of 1.93 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.03 cfs @ 4.61 fps) 3=Orifice/Grate (Orifice Controls 0.29 cfs @ 3.37 fps) 4=Orifice/Grate (Controls 0.00 cfs)						
Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=829.50' (Free Discharge)						



Pond 1P: STORM WATER POND

CJE2151R1	MSE 24-hr 3	10 year Rain	nfall=3.81"
Prepared by {enter your company name here}		Printed	9/20/2021
HydroCAD® 10.00-16 s/n 03450 © 2015 HydroCAD Software Solutions	s LLC		Page 15

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PS-1: PROPOSED	Runoff Area=1.772 ac 76.81% Impervious Runoff Depth>2.55" Tc=6.0 min CN=89 Runoff=8.09 cfs 0.377 af
Subcatchment PS-2: UNDETAINED	Runoff Area=0.335 ac 31.94% Impervious Runoff Depth>1.32" Tc=6.0 min CN=73 Runoff=0.83 cfs 0.037 af
Reach PR: TOTAL OUTFALL	Inflow=1.29 cfs 0.308 af Outflow=1.29 cfs 0.308 af
Pond 1P: STORM WATER POND Primary=1.14 cfs 0	Peak Elev=830.91' Storage=9,955 cf Inflow=8.09 cfs 0.377 af .271 af Secondary=0.00 cfs 0.000 af Outflow=1.14 cfs 0.271 af

Total Runoff Area = 2.107 acRunoff Volume = 0.413 afAverage Runoff Depth = 2.35"30.33% Pervious = 0.639 ac69.67% Impervious = 1.468 ac

Runoff = 8.09 cfs @ 12.13 hrs, Volume= 0.377 af, Depth> 2.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10 year Rainfall=3.81"

Area (ac)	CN	Description				
1.361	98	Paved parking, HSG B				
0.411	61	>75% Grass cover, Good, HSG B				
1.772	89	Weighted Average				
0.411 23.19% Pervious Area						
1.361		76.81% Impervious Area				
Tc Leng (min) (fee	∣th ∋t)	Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)				
6.0 Direct Entry, Min TC						
Subcatchment PS-1: PROPOSED SUBCATCHMENT 1: TO POND						



Summary for Subcatchment PS-2: UNDETAINED

Runoff = 0.83 cfs @ 12.14 hrs, Volume= 0.037 af, Depth> 1.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 10 year Rainfall=3.81"

 Area (ac)	CN	Desc	cription				
0.228	61	>75%	75% Grass cover, Good, HSG B				
 0.107	98	Pave	Paved parking, HSG B				
0.335	73	Weig	ghted Aver	age			
0.228		68.0	6% Pervio	us Area			
0.107		31.9	4% Imperv	vious Area			
 Tc Le (min) (ngth feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.0					Direct Entry, Min Tc		

Subcatchment PS-2: UNDETAINED



Summary for Reach PR: TOTAL OUTFALL

Inflow A	rea =	2.107 ac, 69.67% Impervious, Infl	ow Depth > 1.75"	for 10 year event
Inflow	=	1.29 cfs @ 12.51 hrs, Volume=	0.308 af	
Outflow	=	1.29 cfs @_ 12.51 hrs, Volume=	0.308 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Reach PR: TOTAL OUTFALL

Summary for Pond 1P: STORM WATER POND

Inflow Area =	1.772 ac, 76.81% Impervious, Inflow De	epth > 2.55" for 10 year event
Inflow =	8.09 cfs @ 12.13 hrs, Volume=	0.377 af
Outflow =	1.14 cfs @ 12.54 hrs, Volume=	0.271 af, Atten= 86%, Lag= 24.5 min
Primary =	1.14 cfs @ 12.54 hrs, Volume=	0.271 af
Secondary =	0.00 cfs $@$ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 830.91' @ 12.54 hrs Surf.Area= 7,917 sf Storage= 9,955 cf

Plug-Flow detention time= 193.0 min calculated for 0.271 af (72% of inflow) Center-of-Mass det. time= 141.7 min (909.8 - 768.1)

Volume	Invert	Avail.Stor	rage Storage	Description			
#1	829.50'	35,68	36 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)		
Elevatior (feet	າ Sເ)	urf.Area (sq-ft)	Inc.Store Cum.Store (cubic-feet) (cubic-feet)				
829.50)	6,263	0				
830.00)	6,838	3,275	3,275			
831.00)	8,030	7,434	10,709			
832.00)	18,066	13,048	23,757			
832.50)	29,647	11,928	35,686			
Device	Routing	Invert	Outlet Device	S			
#1 #2 #3 #4 #5	Primary Device 1 Device 1 Device 1 Secondary	829.50' 829.50' 829.80' 830.80' 831.50'	10.0" Round L= 19.0' CPF Inlet / Outlet In n= 0.011, Flo 1.0" Vert. Ori 4.0" Vert. Ori 24.0" Horiz. O Limited to wei 10.0' long x Head (feet) 0 Coef. (English	Culvert P, square edge nvert= 829.50' / w Area= 0.55 s fice/Grate C= fice/Grate C= Drifice/Grate (r flow at low hea 10.0' breadth B .20 0.40 0.60 n) 2.49 2.56 2.	headwall, Ke= 0.500 829.00' S= 0.0263 '/' Cc= 0.900 f 0.600 C= 0.600 ads Broad-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 .70 2.69 2.68 2.69 2.67 2.64		
Primary OutFlow Max=1.14 cfs @ 12.54 hrs HW=830.91' (Free Discharge) 1=Culvert (Passes 1.14 cfs of 2.61 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.03 cfs @ 5.62 fps) -3=Orifice/Grate (Orifice Controls 0.41 cfs @ 4.66 fps) 4=Orifice/Grate (Weir Controls 0.70 cfs @ 1.06 fps)							
Seconda	Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=829.50' (Free Discharge) —5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)						



Pond 1P: STORM WATER POND

CJE2151R1 Prenared by Jenter your company name b	MSE 24-hr 3 100 year Rainfall=6.18" Printed 9/20/2021
HydroCAD® 10.00-16 s/n 03450 © 2015 HydroC	CAD Software Solutions LLC Page 21
Time span=5.00-2 Runoff by SCS TR-2 Reach routing by Stor-Ind+Tra	20.00 hrs, dt=0.05 hrs, 301 points 20 method, UH=SCS, Weighted-CN ns method - Pond routing by Stor-Ind method
Subcatchment PS-1: PROPOSED	Runoff Area=1.772 ac 76.81% Impervious Runoff Depth>4.75" Tc=6.0 min CN=89 Runoff=14.46 cfs 0.701 af
Subcatchment PS-2: UNDETAINED	Runoff Area=0.335 ac 31.94% Impervious Runoff Depth>3.10" Tc=6.0 min CN=73 Runoff=1.94 cfs 0.087 af
Reach PR: TOTAL OUTFALL	Inflow=4.95 cfs 0.656 af Outflow=4.95 cfs 0.656 af
Pond 1P: STORM WATER POND Primary=3.31 cfs 0	Peak Elev=831.50' Storage=15,991 cf Inflow=14.46 cfs 0.701 af 569 af Secondary=0.00 cfs 0.000 af Outflow=3.31 cfs 0.569 af

Total Runoff Area = 2.107 ac Runoff Volume = 0.788 af Average Runoff Depth = 4.49" 30.33% Pervious = 0.639 ac 69.67% Impervious = 1.468 ac

Summary for Subcatchment PS-1: PROPOSED SUBCATCHMENT 1: TO POND

Runoff 14.46 cfs @ 12.13 hrs, Volume= 0.701 af, Depth> 4.75" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100 year Rainfall=6.18"

 Area	(ac)	CN	Desc	cription				
 1.	361	98	Pave	ed parking,	HSG B			
 0.4	411	61	>75%	75% Grass cover, Good, HSG B				
1.	772	89	Weig	hted Aver	age			
0.4	411		23.1	9% Pervio	us Area			
1.	361		76.8	1% Imperv	vious Area			
_								
Tc	Lengt	h i	Slope	Velocity	Capacity	Description		
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)			
6.0						Direct Entry, Min TC		
						•		

Subcatchment PS-1: PROPOSED SUBCATCHMENT 1: TO POND



Summary for Subcatchment PS-2: UNDETAINED

Runoff = 1.94 cfs @ 12.13 hrs, Volume= 0.087 af, Depth> 3.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100 year Rainfall=6.18"



Summary for Reach PR: TOTAL OUTFALL

Inflow Are	ea =	2.107 ac,	69.67% Impervious,	Inflow Depth > 3.	73" for 100 year event
Inflow	=	4.95 cfs @	12.15 hrs, Volume	= 0.656 af	
Outflow	=	4.95 cfs @	12.15 hrs, Volume	= 0.656 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Reach PR: TOTAL OUTFALL

Summary for Pond 1P: STORM WATER POND

Inflow Area =	1.772 ac, 76.81% Impervious, Inflo	ow Depth > 4.75" for 100 year event
Inflow =	14.46 cfs @ 12.13 hrs, Volume=	0.701 af
Outflow =	3.31 cfs @ 12.37 hrs, Volume=	0.569 af, Atten= 77%, Lag= 14.8 min
Primary =	3.31 cfs @ 12.37 hrs, Volume=	0.569 af
Secondary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 831.50' @ 12.37 hrs Surf.Area= 13,058 sf Storage= 15,991 cf

Plug-Flow detention time= 127.4 min calculated for 0.567 af (81% of inflow) Center-of-Mass det. time= 85.5 min (842.8 - 757.3)

Volume	Invert	Avail.Sto	rage Storage	Description		
#1	829.50'	35,68	86 cf Custon	n Stage Data (P	rismatic)Listed below (Recalc)	
Elevatio (fee	n Si t)	urf.Area (sg-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
829.5	<u>.</u> 0	6 263	0	0		
830.0	0	6 838	3 275	3 275		
831.0	0	8.030	7.434	10,709		
832.0	0	18,066	13,048	23,757		
832.5	0	29,647	11,928	35,686		
Device	Routing	Invert	Outlet Device	es		
#1 #2 #3 #4 #5	Primary Device 1 Device 1 Device 1 Secondary	829.50' 829.50' 829.80' 830.80' 831.50'	 10.0" Round Culvert L= 19.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 829.50' / 829.00' S= 0.0263 '/' Cc= 0.900 n= 0.011, Flow Area= 0.55 sf 1.0" Vert. Orifice/Grate C= 0.600 4.0" Vert. Orifice/Grate C= 0.600 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads 10.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64			
Primary OutFlow Max=3.30 cfs @ 12.37 hrs HW=831.50' (Free Discharge) 1=Culvert (Inlet Controls 3.30 cfs @ 6.06 fps) 2=Orifice/Grate (Passes < 0.04 cfs potential flow) 3=Orifice/Grate (Passes < 0.52 cfs potential flow) 4=Orifice/Grate (Passes < 12.03 cfs potential flow) \$econdary OutFlow Max=0.00 cfs @ 5.00 hrs HW=829.50' (Free Discharge)						
5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)						



Pond 1P: STORM WATER POND

WinSLAMM Schematic Diagram



Data file name: Z:\WinSLAMM\CJE2151R1.mdb WinSLAMM Version 10.2.0 Rain file name: C:\WinSLAMM Files\Rain Files\WisReg - Milwaukee WI 1969.RAN Particulate Solids Concentration file name: C:\WinSLAMM Files\v10.1 WI AVG01.pscx Runoff Coefficient file name: C:\WinSLAMM Files\WI SL06 Dec06.rsvx Residential Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Institutional Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Commercial Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Industrial Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Other Urban Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Freeway Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance: False Pollutant Relative Concentration file name: C:\WinSLAMM Files\WI GEO03.ppdx Source Area PSD and Peak to Average Flow Ratio File: C:\WinSLAMM Files\NURP Source Area PSD Files.csv Cost Data file name: Seed for random number generator: -42 Study period starting date: 01/05/69 Study period ending date: 12/31/69 Start of Winter Season: 12/06 End of Winter Season: 03/28 Date: 09-20-2021 Time: 11:10:52 Site information:

LU# 1 - Commercial: Subcatchment 1- Area to Pond Total area (ac): 1.772 13 - Paved Parking 1: 1.361 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz 45 - Large Landscaped Areas 1: 0.411 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

LU# 2 - Commercial: Subcatchment 2 - Undetained Total area (ac): 0.335 13 - Paved Parking 1: 0.107 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz 45 - Large Landscaped Areas 1: 0.228 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz

Control Practice 1: Wet Detention Pond CP# 1 (DS) - Stormwater Pond Particle Size Distribution file name: Not needed - calculated by program Initial stage elevation (ft): 3.5 Peak to Average Flow Ratio: 3.8 Maximum flow allowed into pond (cfs): No maximum value entered **Outlet Characteristics:** Outlet type: Orifice 1 1. Orifice diameter (ft): 0.08 2. Number of orifices: 1 3. Invert elevation above datum (ft): 3.5 Outlet type: Orifice 2 1. Orifice diameter (ft): 0.33 2. Number of orifices: 1 3. Invert elevation above datum (ft): 3.8 Outlet type: Broad Crested Weir 1. Weir crest length (ft): 5

- 2. Weir crest width (ft): 10
- 3. Height from datum to bottom of weir opening: 5.5

Outlet type: Vertical Stand Pipe

1. Stand pipe diameter (ft): 2

2. Stand pipe height above datum (ft): 4.8 Pond stage and surface area

Entry	Stage	Pond Area	Natural Seepage	Other Outflow
Number	(ft)	(acres)	(in/hr)	(cfs)
0	0.00	0.0000	0.00	0.00
1	0.01	0.0320	0.00	0.00
2	1.00	0.0440	0.00	0.00
3	2.00	0.0580	0.00	0.00
4	2.50	0.0640	0.00	0.00
5	3.00	0.1020	0.00	0.00
6	3.50	0.1440	0.00	0.00
7	4.00	0.1570	0.00	0.00
8	5.00	0.1840	0.00	0.00
9	6.00	0.4150	0.00	0.00
10	6.50	0.6810	0.00	0.00

SLAMM for Windows Version 10.2.0 (c) Copyright Robert Pitt and John Voorhees 2012 All Rights Reserved

Data file name: Z:\WinSLAMM\CJE2151R1.mdb Data file description: Rain file name: C:\WinSLAMM Files\Rain Files\WisReg - Milwaukee WI 1969.RAN Particulate Solids Concentration file name: C:\WinSLAMM Files\v10.1 WI AVG01.pscx Runoff Coefficient file name: C:\WinSLAMM Files\WI SL06 Dec06.rsvx Residential Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Institutional Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Commercial Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Industrial Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Other Urban Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Freeway Street Delivery file name: C:\WinSLAMM Files\WI Com Inst Indust Dec06.std Pollutant Relative Concentration file name: C:\WinSLAMM Files\WI GEO03.ppdx Start of Winter Season: 12/06 End of Winter Season: 03/28 Model Run Start Date: 01/05/69 Model Run End Date: 12/31/69 Date of run: 09-20-2021 Time of run: 11:08:43 Total Area Modeled (acres): 2.107 Years in Model Run: 0.99

Runoff	Percent P	articulate	Particulate	Percent	
Volume	Runoff	Solids	Solids P	Particulate	
(cu ft)	Volume	Conc.	Yield	Solids	
Re	duction	(mg/L)	(lbs) Re	duction	
Total of all Land Uses without Controls:	116	131 -	132.8	962.8	-
Outfall Total with Controls:	115865	0.23%	25.45	184.1	80.88%
Annualized Total After Outfall Controls:	117	474		186.6	

1220 S. Prairie Avenue Storm Water Management Practice Maintenance Agreement

Owner:

as "Owner" of the subject property, in accordance with the City of Waukesha agrees to install and maintain storm water management practice(s) on the subject property in accordance with approved plans and Storm Water Management Plan conditions. The owner further agrees to the terms stated in this document to ensure that the storm water management practice(s) continues serving the intended functions in perpetuity. This Agreement shall run with the Property and be binding upon all heirs, successors and assigns.

Minimum Maintenance Requirements:

To ensure the proper function of the storm water management facilities described above, the following activities must be completed:

- 1. All outlet pipes must be checked monthly to ensure there is no blockage from floating debris or ice. Any blockage must be removed immediately.
- 2. Grass buffer areas shall be preserved and maintained to allow free flowing of sheetflow surface runoff in accordance with approved grading plans. No grading or filling is allowed that may interrupt flows in any way.
- 3. Inlets and outlets must be checked after heavy rains (minimum of annually) for signs of erosion. Any eroding areas must be repaired immediately to prevent premature sediment build-up in the downstream basin. Erosion matting is recommended for repairing grassed areas.
- 4. NO trees are to be planted or allowed to grow on the earthen berms. Tree root systems can reduce soil compaction and cause berm failure. The berms must be inspected annually and any woody vegetation removed.
- 5. If floating algae or weed growth becomes a nuisance (decay odors, etc.), it must be removed from the basin and deposited where it cannot drain back into the basin. Removal of the vegetation from the water reduces regrowth the following season (by harvesting the nutrients).
- 6. When sediment in the basin has accumulated to an elevation of two foot below the outlet elevation, it must be removed. All removed sediment must be placed in an appropriate upland disposal site and stabilized (grass cover) to prevent sediment from washing back into the basin. Failure to remove sediment will cause resuspension of previously trapped sediments and increase downstream deposition.
- 7. No grading or filling of the basin or berm other than for sediment removal is allowed, unless otherwise approved by the City of Waukesha.
- 8. Periodic mowing of the grass swales will encourage rigorous grass cover and allow better inspections for erosion. Waiting until after August 1 will avoid disturbing nesting wildlife. Mowing around the basin may attract nuisance populations of geese to the property and is not necessary or recommended.
- 9. Any other repair or maintenance needed to ensure the continued function of the storm water practices or as ordered by the City of Waukesha.